

# **Stormwater Management Report**

CCC - Hudson, OH

750 W Streetsboro St, Hudson, OH 44236

Date Prepared: July 7th, 2025

Revised:

# On behalf of:

# Christ Community Chapel

# **Contact:**

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Date: 07/07/2025

# **Engineer of Record:**

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#### Introduction

This report covers the methodology and calculations used in the design of the stormwater management system for the proposed turf field and restroom at the existing Chris Community Chapel campus in Hudson, Ohio.

The stormwater management system is designed in accordance with Chapter 1419 of the City of Hudson's Codified Ordinances. Section 5 of Chapter 1419 has requirements for stormwater quality, stormwater quantity, and major flood path.

- The major flood path is met by directing all runoff towards the proposed detention basin.
- The stormwater quality requirements can be met by adding the water quality volume to the volume of the proposed detention basin and meeting the minimum drawdown time of 48 hours.
- The stormwater quantity requirements for the field's construction and restroom area can be met by
  the proposed detention basin. According to the City of Hudson Stormwater Code, the 25-year peak
  rate of runoff in the post-developed condition shall not exceed the 2-year peak rate of runoff in the
  existing condition. The 100-year post developed condition must also be reduced to the 10-yr peak
  rate of runoff in the existing condition.

Storm routings for this project were performed using HydroCAD. Time of Concentration was determined by using the TR-55 method, within HydroCAD.

The onsite soils were obtained from USDA NRCS Web Soil Survey and can be found in Appendix D.

The storm pipe network was designed using Hydraflow Stormsewers Extension for Autodesk Civil 3D. Section 4 of Chapter 4 of the City of Hudson's Codified Ordinances requires that the pipes be sized so that the HGL does not exceed the crown of the pipe for the 10-year storm. Refer to **Appendix C** for the Storm Pipe Calculations and **Appendix E3** for the associated Tributary Drainage area Map.

#### **Existing Conditions**

The site is a church on 30.07 acres that has frontage along West Streetsboro Street to the north and Terex Road to the west. The site consists of a chapel, parking lot, open field, and stormwater management area. Residential properties border the site to the east and to the south. Existing runoff flows to the stormwater management area via a combination of overland flow and an existing storm sewer system.

The existing runoff consists of one (1) major existing drainage areas as listed below:

 EDA-WEST - This drainage area drains to the north, towards an existing stormwater management area. The Soil Survey indicates this site to have Caneadea Silt Loam, Ellsworth-Urban Land Complex, Geeburg Silt Loam, Sebring Silt Loam, each soil being Hydric Group 'D' type soil. For hydrologic soil group 'D' soils we assumed CN values of 98 for impervious areas and 84 for grass areas in good conditions.



Peak runoff rates from the existing conditions of the site are listed in the following table:

	Existing Conditions Peak Runoff Rates									
Drainage	1-year	2-year	5-year	10-year	25-year	50-year	100-year			
Area	Storm	Storm	Storm	Storm	Storm	Storm	Storm			
EDA-WEST	5.42 CFS	7.38 CFS	10.22 CFS	12.67 CFS	16.25 CFS	19.29 CFS	22.52 CFS			

Refer to **Appendix A** for the Existing Conditions Calculations. The Existing Conditions Drainage Area Map can be found in **Appendix E1**.

### **Proposed Conditions**

The proposed development of the site will consist of the construction of a 389 SF restroom building, a 6,834 SF office addition, a 44,500 SF turf soccer field, associated site improvements and a stormwater management system. Due to the location of the proposed office addition, an existing parking area to the south of the proposed field addition is being routed to a proposed detention basin to compensate for the limits of disturbance associated with the office addition. See Proposed Drainage Plan in Appendix E2. The office addition will be routed to the existing detention basin. The stormwater management system consists of an extended detention basin, a gravel area with a 6" underdrain, an outlet control structure, and an emergency spillway. The extended detention basin in conjunction with the outlet control structure has been designed to address the water quality and water quantity requirements. The outflow from the proposed detention basin will be routed through the outlet structure and directed into the existing storm water management system on site.

The proposed improvements will create one (1) major drainage area and one (1) detention node

- DA-WEST This drainage area drains to stormwater management basin. This includes the existing parking lot area that is being routed to the basin, in place of the proposed office addition area.
- POND Proposed stormwater management basin, which discharges into the existing storm water management system on site.

For hydrologic soil group 'D' soils we assumed CN values of 98 for impervious areas and 84 for grass areas in good conditions.

#### **Stormwater Quality**

To satisfy the water quality requirements, Ohio's water quality BMP Compliance Worksheet and Water Quality Calculator were used. The water quality volume was calculated and was incorporated into the detention pond design. Calculations included the proposed field LOD, as well as the parking lot to the south of this area. The parking lot to the south of the proposed field LOD was added to this calculation to compensate for the office addition LOD, which is not included because it is being routed to the existing detention basin. A water quality orifice was included to satisfy the requirements, as well as a window on the outlet control structure at the water quality elevation. The orifice has been designed to meet the required minimum drawdown time of 48 hours.



Therefore, the stormwater quality requirements have been satisfied. Refer to **Appendix B1** for Stormwater Quality Calculations.

### **Stormwater Quantity**

The resulting proposed conditions peak runoff rates are listed in the following table:

	Proposed Conditions Peak Runoff Rates											
Drainage 1-year 2-year 5-year 10-year 25-year 50-year 100-												
Area	Storm	Storm	Storm	Storm	Storm	Storm	Storm					
PDA-WEST	7.62 CFS	9.89 CFS	13.22 CFS	16.06 CFS	20.16 CFS	23.62 CFS	27.29 CFS					

The discharge characteristics for the proposed stormwater management basin are listed in the following table:

Basin									
Storm	Peak	Water surface							
3(01111	Discharge	Elevation							
1-year	2.02 CFS	1010.64 FT							
2-year	3.21 CFS	1010.86 FT							
5-year	4.52 CFS	1011.24 FT							
10-year	5.38 CFS	1011.57FT							
25-year	7.30 CFS	1011.98 FT							
50-year	9.86 CFS	1012.21 FT							
100-year	11.65 CFS	1012.45 FT							

The runoff of the 25-year storm event has been designed to be lower than the existing runoff of the 2-year storm event, as has the 100-year event to the existing 10-year storm event based upon the requirements set forth by the City of Hudson's codified ordinances. Refer to **Appendix B** for storm calculations.

A summary of the existing conditions peak runoff rates, the allowable peak runoff rates and the proposed conditions peak runoff rates are listed in the following table:

	Runoff Reduction Summary										
Storm Existing Allowable Proposed											
1-year	5.42 CFS	7.38 CFS	2.02 CFS								
2-year	7.38 CFS	7.38 CFS	3.21 CFS								
5-year	10.22 CFS	7.38 CFS	4.52 CFS								
10-year	12.67 CFS	7.38 CFS	5.38 CFS								
25-year	16.25 CFS	7.38 CFS	7.30 CFS								
50-year	19.29 CFS	12.67 CFS	9.86 CFS								
100-year	22.52 CFS	12.67 CFS	11.65 CFS								

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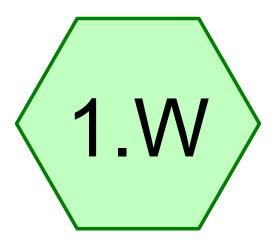
Refer to **Appendix B** for the Stormwater Quantity Calculations. The Proposed Conditions Drainage Area Map can be found in **Appendix E2**.

### **Summary**

The proposed stormwater management system has been successfully designed to manage the increased runoff from associated improvements of the project. The stormwater management system has been designed in accordance with the appropriate regulations, as demonstrated in the previous tables and accompanying calculations.



# APPENDIX A: EXISTING CONDITIONS CALCULATIONS



# **EDA-WEST**









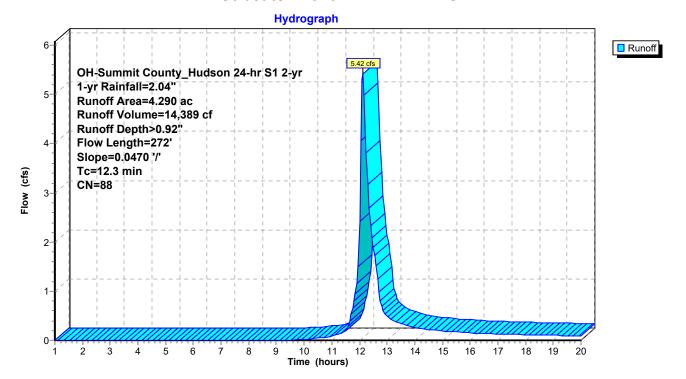
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# **Summary for Subcatchment 1.W: EDA-WEST**

Runoff = 5.42 cfs @ 12.13 hrs, Volume= 14,389 cf, Depth> 0.92"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-20.00 hrs, dt= 0.05 hrs OH-Summit County Hudson 24-hr S1 2-yr 1-yr Rainfall=2.04"

	Area	(ac) C	N Des	cription		
_	3.	075 8	34 50-7	5% Grass	cover, Fair	HSG D
_	1.	215	98 Pave	ed parking	, HSG D	
	4.	290 8	38 Weig	ghted Aver	age	
	3.	075	71.6	8% Pervio	us Area	
	1.	215	28.3	2% Imperv	/ious Area	
_	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
	11.0	150	0.0470	0.23		Sheet Flow,
	1.3	122	0.0470	1.52		Grass: Short n= 0.150 P2= 2.44"  Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
	12.3	272	Total			



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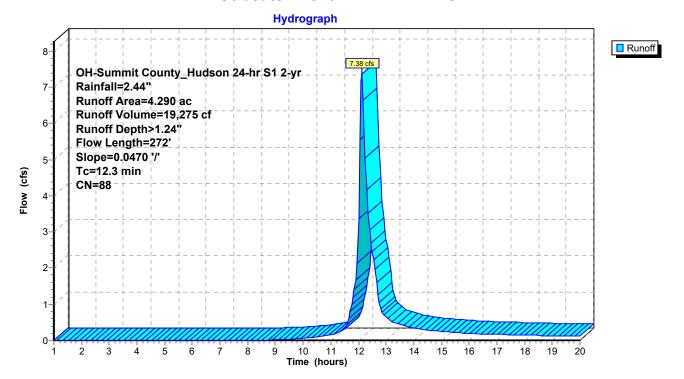
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# **Summary for Subcatchment 1.W: EDA-WEST**

Runoff = 7.38 cfs @ 12.12 hrs, Volume= 19,275 cf, Depth> 1.24"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-20.00 hrs, dt= 0.05 hrs OH-Summit County Hudson 24-hr S1 2-yr Rainfall=2.44"

_	Area	(ac) C	N Des	cription		
	3.	075 8	34 50-7	5% Grass	cover, Fair	r, HSG D
_	1.	215	98 Pave	ed parking	, HSG D	
	4.	290 8	38 Weig	ghted Aver	age	
	3.	075	71.6	8% Pervio	us Area	
	1.	215	28.3	2% Imperv	/ious Area	
	_					
	Tc	Length	Slope	Velocity	Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	11.0	150	0.0470	0.23		Sheet Flow,
						Grass: Short n= 0.150 P2= 2.44"
	1.3	122	0.0470	1.52		Shallow Concentrated Flow,
_						Short Grass Pasture Kv= 7.0 fps
	12 3	272	Total			



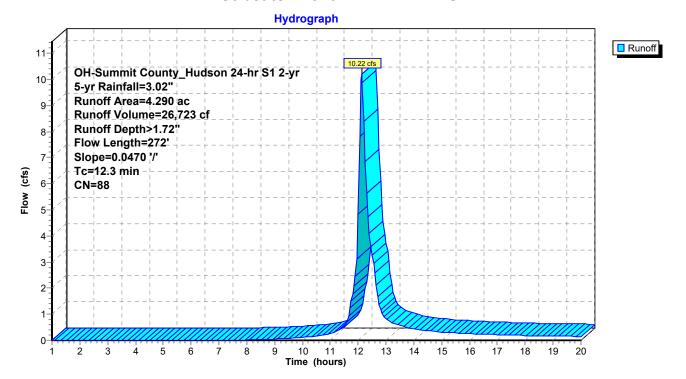
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# **Summary for Subcatchment 1.W: EDA-WEST**

Runoff = 10.22 cfs @ 12.12 hrs, Volume= 26,723 cf, Depth> 1.72"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-20.00 hrs, dt= 0.05 hrs OH-Summit County Hudson 24-hr S1 2-yr 5-yr Rainfall=3.02"

_	Area	(ac) C	N Des	cription		
	3.	075	84 50-7	5% Grass	cover, Fair	HSG D
_	1.	215	98 Pave	ed parking	, HSG D	
	4.	290	88 Wei	ghted Aver	age	
	3.	075	71.6	8% Pervio	us Area	
	1.	215	28.3	2% Imperv	∕ious Area	
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
	11.0	150	0.0470	0.23		Sheet Flow,
	1.3	122	0.0470	1.52		Grass: Short n= 0.150 P2= 2.44" <b>Shallow Concentrated Flow,</b> Short Grass Pasture Kv= 7.0 fps
	12.3	272	Total			



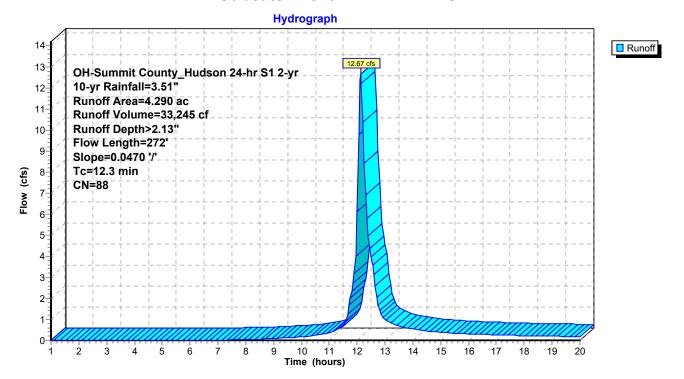
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# **Summary for Subcatchment 1.W: EDA-WEST**

Runoff = 12.67 cfs @ 12.12 hrs, Volume= 33,245 cf, Depth> 2.13"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-20.00 hrs, dt= 0.05 hrs OH-Summit County Hudson 24-hr S1 2-yr 10-yr Rainfall=3.51"

_	Area	(ac) C	N Des	cription		
	3.	075 8	34 50-7	5% Grass	cover, Fair	, HSG D
_	1.	215	98 Pave	ed parking	, HSG D	
	4.	290 8	38 Weig	ghted Aver	age	
	3.	075	71.6	8% Pervio	us Area	
	1.	215	28.3	2% Imper	/ious Area	
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
	11.0	150	0.0470	0.23		Sheet Flow,
	1.3	122	0.0470	1.52		Grass: Short n= 0.150 P2= 2.44" <b>Shallow Concentrated Flow,</b> Short Grass Pasture Kv= 7.0 fps
	12.3	272	Total			



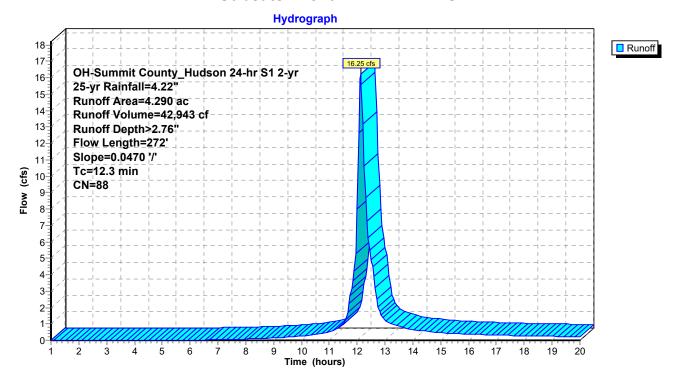
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# **Summary for Subcatchment 1.W: EDA-WEST**

Runoff = 16.25 cfs @ 12.12 hrs, Volume= 42,943 cf, Depth> 2.76"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-20.00 hrs, dt= 0.05 hrs OH-Summit County Hudson 24-hr S1 2-yr 25-yr Rainfall=4.22"

_	Area	(ac) C	N Des	cription		
	3.	075 8	34 50-7	5% Grass	cover, Fair	, HSG D
_	1.	215	98 Pave	ed parking	, HSG D	
	4.	290 8	38 Weig	ghted Aver	age	
	3.	075	71.6	8% Pervio	us Area	
	1.	215	28.3	2% Imper	/ious Area	
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
	11.0	150	0.0470	0.23		Sheet Flow,
	1.3	122	0.0470	1.52		Grass: Short n= 0.150 P2= 2.44" <b>Shallow Concentrated Flow,</b> Short Grass Pasture Kv= 7.0 fps
	12.3	272	Total			



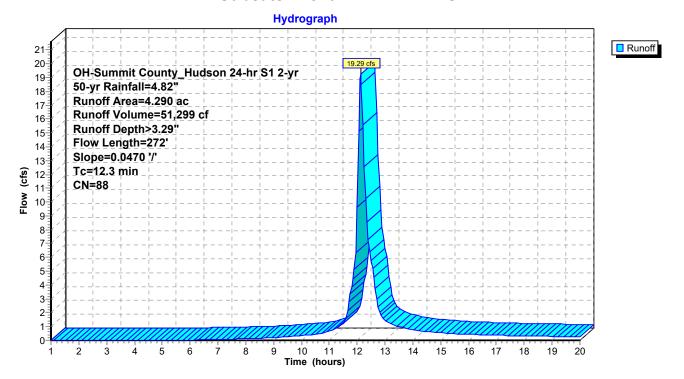
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# **Summary for Subcatchment 1.W: EDA-WEST**

Runoff = 19.29 cfs @ 12.12 hrs, Volume= 51,299 cf, Depth> 3.29"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-20.00 hrs, dt= 0.05 hrs OH-Summit County Hudson 24-hr S1 2-yr 50-yr Rainfall=4.82"

	Area	(ac) C	N Des	cription		
	3.	075 8	34 50-7	5% Grass	cover, Fair	HSG D
_	1.	215	98 Pave	ed parking	, HSG D	
	4.	290 8	38 Weig	ghted Aver	age	
	3.	075	71.6	8% Pervio	us Area	
	1.	215	28.3	2% Imperv	∕ious Area	
	Тс	Length	Slope	Velocity	Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	11.0	150	0.0470	0.23		Sheet Flow,
						Grass: Short n= 0.150 P2= 2.44"
	1.3	122	0.0470	1.52		Shallow Concentrated Flow,
_						Short Grass Pasture Kv= 7.0 fps
	12.3	272	Total		·	



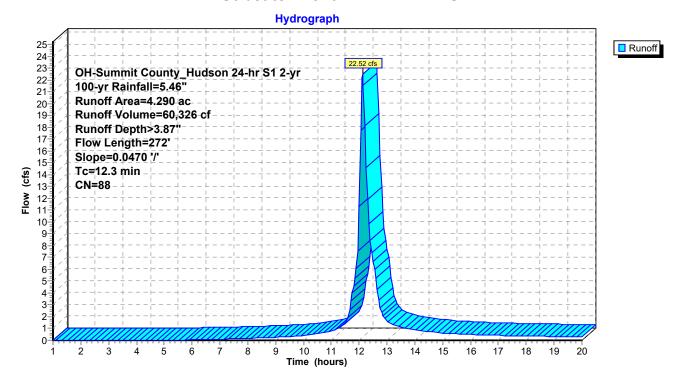
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# **Summary for Subcatchment 1.W: EDA-WEST**

Runoff = 22.52 cfs @ 12.12 hrs, Volume= 60,326 cf, Depth> 3.87"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-20.00 hrs, dt= 0.05 hrs OH-Summit County Hudson 24-hr S1 2-yr 100-yr Rainfall=5.46"

_	Area	(ac) C	N Des	cription		
	3.	075	84 50-7	5% Grass	cover, Fair	HSG D
_	1.	215	98 Pave	ed parking	, HSG D	
	4.	290	88 Wei	ghted Aver	age	
	3.	075	71.6	8% Pervio	us Area	
	1.	215	28.3	2% Imperv	∕ious Area	
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
	11.0	150	0.0470	0.23		Sheet Flow,
	1.3	122	0.0470	1.52		Grass: Short n= 0.150 P2= 2.44" <b>Shallow Concentrated Flow,</b> Short Grass Pasture Kv= 7.0 fps
	12.3	272	Total	·		



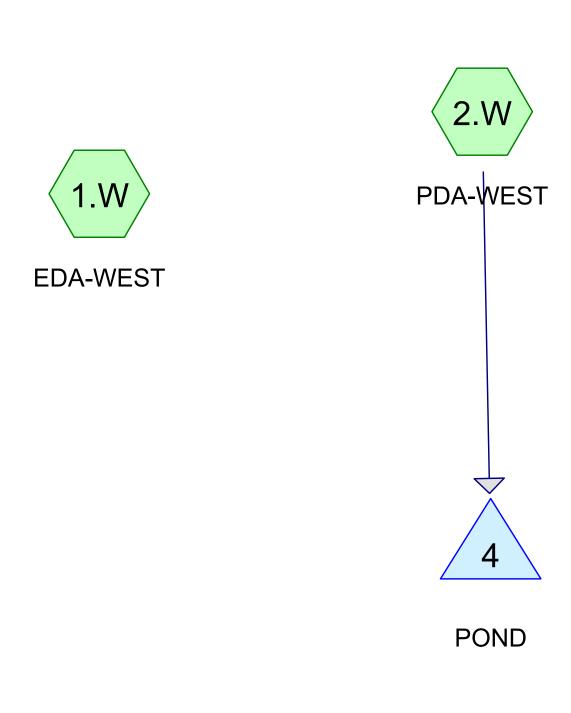
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# **Events for Subcatchment 1.W: EDA-WEST**

Event	Rainfall	Runoff	Volume	Depth
	(inches)	(cfs)	(cubic-feet)	(inches)
1-yr	2.04	5.42	14,389	0.92
2-yr	2.44	7.38	19,275	1.24
5-yr	3.02	10.22	26,723	1.72
10-yr	3.51	12.67	33,245	2.13
25-yr	4.22	16.25	42,943	2.76
50-yr	4.82	19.29	51,299	3.29
100-yr	5.46	22.52	60,326	3.87



# APPENDIX B: PROPOSED CONDITIONS CALCULATIONS











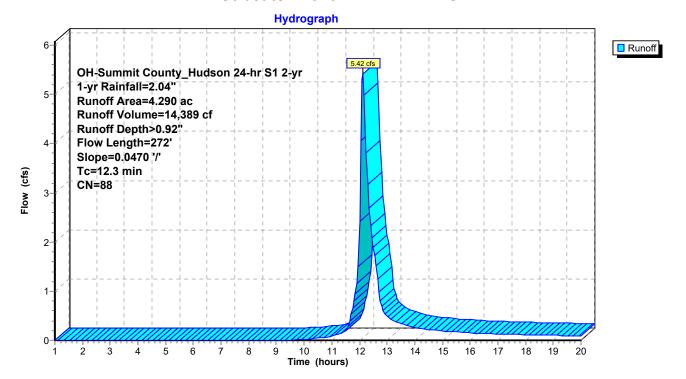
Page 2

# **Summary for Subcatchment 1.W: EDA-WEST**

Runoff = 5.42 cfs @ 12.13 hrs, Volume= 14,389 cf, Depth> 0.92"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-20.00 hrs, dt= 0.05 hrs OH-Summit County Hudson 24-hr S1 2-yr 1-yr Rainfall=2.04"

	Area	(ac) C	N Des	Description					
_	3.	075 8	34 50-7	5% Grass	cover, Fair	HSG D			
_	1.	1.215 98 Paved parking, HSG D							
	4.290 88 Weighted Average								
	3.075 71.68% Pervious Area								
	1.	215	28.3	2% Imperv	/ious Area				
_	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
	11.0	150	0.0470	0.23		Sheet Flow,			
	1.3	122	0.0470	1.52		Grass: Short n= 0.150 P2= 2.44"  Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps			
	12.3	272	Total						



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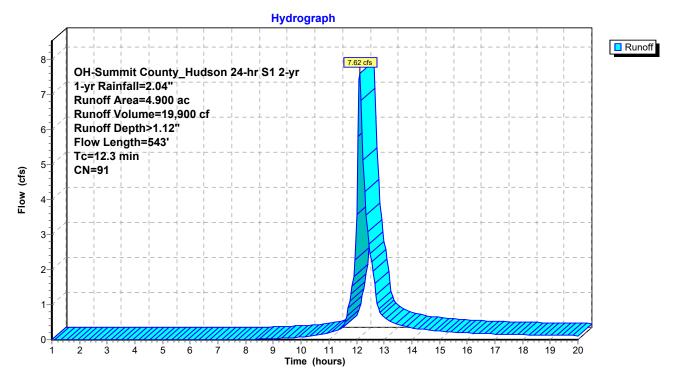
# **Summary for Subcatchment 2.W: PDA-WEST**

7.62 cfs @ 12.12 hrs, Volume= 19,900 cf, Depth> 1.12" Runoff

Routed to Pond 4: POND

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-20.00 hrs, dt= 0.05 hrs OH-Summit County Hudson 24-hr S1 2-yr 1-yr Rainfall=2.04"

	Area	(ac) C	N Des	cription				
*	1.	020	95 Perr	neable Tui	rf Field, HS	G D		
	2.	080	84 50-7	5% Grass	cover, Fair	, HSG D		
	1.	800	98 Pave	ed parking	, HSG D			
	4.900 91 Weighted Average			ghted Aver	age			
	3.100		•	63.27% Pervious Area				
	1.	800	36.7	3% Imper	vious Area			
				•				
	Tc	Length	Slope	Velocity	Capacity	Description		
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	·		
	9.9	43	0.0050	0.07	`	Sheet Flow,		
						Grass: Short n= 0.150 P2= 2.44"		
	1.9	300	0.0050	2.63	0.52	Pipe Channel,		
						6.00" Round Area= 0.2 sf Perim= 1.6' r= 0.13'		
						n= 0.010		
	0.5	200	0.0100	6.84	8.40	Pipe Channel,		
						15.00" Round Area= 1.2 sf Perim= 3.9' r= 0.31'		
						n= 0.010		
	12.3	543	Total					



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## **Summary for Pond 4: POND**

Inflow Area = 213,444 sf, 36.73% Impervious, Inflow Depth > 1.12" for 1-yr event

Inflow 7.62 cfs @ 12.12 hrs, Volume= 19,900 cf

2.02 cfs @ 12.59 hrs, Volume= 2.02 cfs @ 12.59 hrs, Volume= Outflow 12,175 cf, Atten= 73%, Lag= 28.0 min

Primary 12,175 cf

Routed to nonexistent node 5L

Secondary = 0.00 cfs @ 1.00 hrs, Volume= 0 cf

Routed to nonexistent node 5L

Routing by Stor-Ind method, Time Span= 1.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 1,010.64' @ 12.59 hrs Surf.Area= 8,152 sf Storage= 10,510 cf

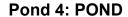
Plug-Flow detention time= 132.6 min calculated for 12,175 cf (61% of inflow)

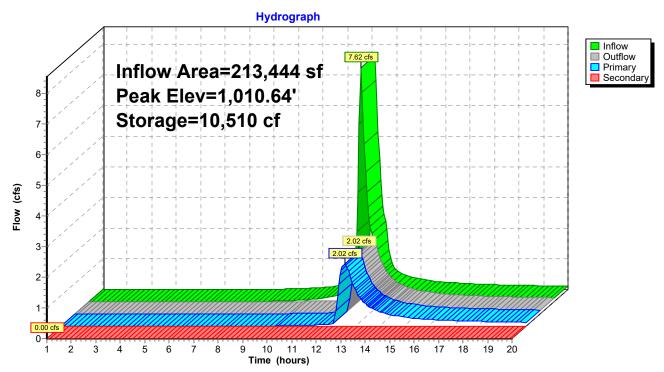
Center-of-Mass det. time= 68.3 min (852.2 - 783.9)

Volume	Invert	Avai	l.Stora	ge Storage Descr	iption			
#1	#1 1,008.00' 44,483 cf		cf Custom Stage	e Data (Prismatic)Lis	ited below (Recalc)			
<b>-</b> 1		C A	V/ . ! . l .	la contra de la contra del la contra de la contra de la contra del la contra del la contra de la contra de la contra del la contra de	O Ot			
		Voids		Cum.Store				
(feet	,	(sq-ft)	(%)		(cubic-feet)			
1,008.0		2,848	0.0		0			
1,009.0		2,848	40.0		1,139			
1,010.0		6,536	100.0		5,831			
1,011.0		9,073	100.0		13,636			
1,012.0		11,663	100.0	•	24,004			
1,013.0	0	14,311	100.0	12,987	36,991			
1,013.5	0	15,658	100.0	7,492	44,483			
Device	Routing	ln	vert	Outlet Devices				
#1	Device 4	1,008	.00'	1.44" Vert. Water C	Quality Orifice C= 0	.600		
				Limited to weir flow at low heads				
#2	Secondary	1,013	.00'	10.0' long + 3.0 '/' SideZ x 4.0' breadth Spillway				
				Head (feet) 0.20 0.	.40 0.60 0.80 1.00	1.20 1.40 1.60 1.80 2.00		
				2.50 3.00 3.50 4.00 4.50 5.00 5.50				
				Coef. (English) 2.38	8 2.54 2.69 2.68 2.	67 2.67 2.65 2.66 2.66		
				2.68 2.72 2.73 2.76 2.79 2.88 3.07 3.32				
#3	Device 4	1,012	.80'	27.50" x 27.50" Ho	<b>riz. Rim</b> C= 0.600			
				Limited to weir flow	at low heads			
#4	Primary	1,008	.00'	18.00" Vert. Outlet	C= 0.600 Limited	to weir flow at low heads		
#5	Device 4	1,010	.25'	30.00" W x 5.00" H	<b>Vert. WQV Window</b>	C= 0.600		
				Limited to weir flow	at low heads			
#6	Device 4	1,011	.80'	48.00" W x 4.00" H Vert. Orifice/Grate C= 0.600				
				Limited to weir flow	at low heads			

Primary OutFlow Max=2.02 cfs @ 12.59 hrs HW=1,010.64' (Free Discharge)
4=Outlet (Passes 2.02 cfs of 11.69 cfs potential flow)
1=Water Quality Orifice (Orifice Controls 0.09 cfs @ 7.73 fps)
3=Rim (Controls 0.00 cfs)
5=WQV Window (Orifice Controls 1.93 cfs @ 2.00 fps)
6=Orifice/Grate (Controls 0.00 cfs)

Secondary OutFlow Max=0.00 cfs @ 1.00 hrs HW=1,008.00' (Free Discharge) 2=Spillway (Controls 0.00 cfs)





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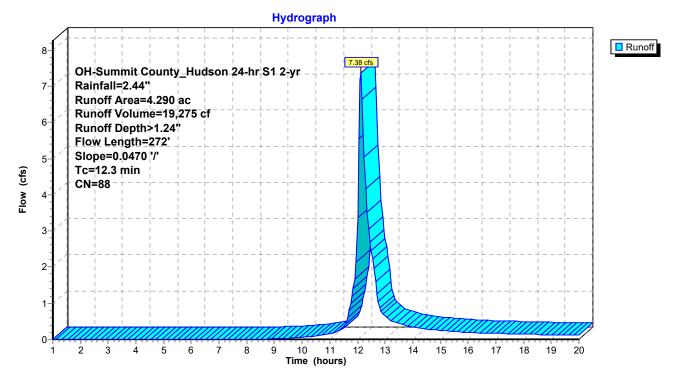
Page 7

# **Summary for Subcatchment 1.W: EDA-WEST**

Runoff = 7.38 cfs @ 12.12 hrs, Volume= 19,275 cf, Depth> 1.24"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-20.00 hrs, dt= 0.05 hrs OH-Summit County Hudson 24-hr S1 2-yr Rainfall=2.44"

_	Area	(ac) C	N Des	Description						
	3.	075	84 50-7	5% Grass	cover, Fair	HSG D				
_	1.	215	98 Pave	ed parking	, HSG D					
	4.290 88 Weighted Average									
	3.075 71.68% Pervious Area									
	1.	215	28.3	2% Imperv	∕ious Area					
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description				
	11.0	150	0.0470	0.23		Sheet Flow,				
	1.3	122	0.0470	1.52		Grass: Short n= 0.150 P2= 2.44" <b>Shallow Concentrated Flow,</b> Short Grass Pasture Kv= 7.0 fps				
	12.3	272	Total	·						



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# **Summary for Subcatchment 2.W: PDA-WEST**

Runoff = 9.89 cfs @ 12.12 hrs, Volume= 25,919 cf, Depth> 1.46"

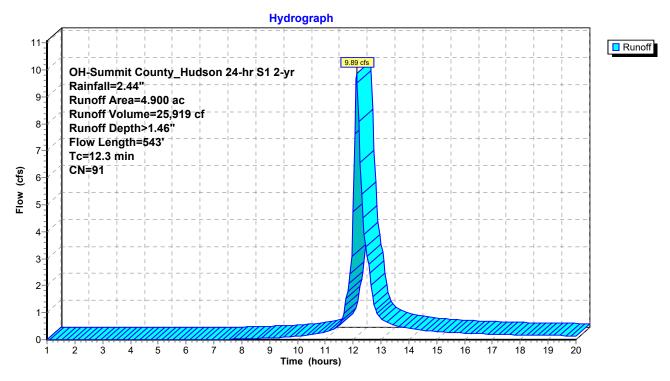
Routed to Pond 4: POND

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-20.00 hrs, dt= 0.05 hrs OH-Summit County Hudson 24-hr S1 2-yr Rainfall=2.44"

	Area	(ac) C	N Des	cription				
*	1.	020	95 Perr	neable Tui	rf Field, HS	G D		
	2.	080	84 50-7	5% Grass	cover, Fair	, HSG D		
	1.	800	98 Pave	ed parking	, HSG D			
	4.900 91 Weighted Average			ghted Aver	age			
	3.100		•	63.27% Pervious Area				
	1.	800	36.7	3% Imper	vious Area			
				•				
	Tc	Length	Slope	Velocity	Capacity	Description		
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	·		
	9.9	43	0.0050	0.07	`	Sheet Flow,		
						Grass: Short n= 0.150 P2= 2.44"		
	1.9	300	0.0050	2.63	0.52	Pipe Channel,		
						6.00" Round Area= 0.2 sf Perim= 1.6' r= 0.13'		
						n= 0.010		
	0.5	200	0.0100	6.84	8.40	Pipe Channel,		
						15.00" Round Area= 1.2 sf Perim= 3.9' r= 0.31'		
						n= 0.010		
	12.3	543	Total					

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## **Summary for Pond 4: POND**

Inflow Area = 213,444 sf, 36.73% Impervious, Inflow Depth > 1.46" for 2-yr event

Inflow 9.89 cfs @ 12.12 hrs, Volume= 25,919 cf

3.21 cfs @ 12.48 hrs, Volume= 3.21 cfs @ 12.48 hrs, Volume= Outflow 18,094 cf, Atten= 68%, Lag= 21.7 min

Primary 18,094 cf

Routed to nonexistent node 5L

Secondary = 0.00 cfs @ 1.00 hrs, Volume= 0 cf

Routed to nonexistent node 5L

Routing by Stor-Ind method, Time Span= 1.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 1,010.86' @ 12.48 hrs Surf.Area= 8,706 sf Storage= 12,348 cf

Plug-Flow detention time= 112.5 min calculated for 18,047 cf (70% of inflow)

Center-of-Mass det. time= 54.8 min (833.4 - 778.7)

Volume	Invert	Avail	.Stora	ge Storage Descr	iption			
#1	1,008.00'	۷	14,483	cf Custom Stage	e Data (Prismatic)Li	sted below (Recalc)		
Elevatio	n Su	rf.Area	Voids	Inc.Store	Cum.Store			
		(%)	(cubic-feet)	(cubic-feet)				
1,008.0	/	2,848	0.0		0			
1,009.0		2,848	40.0		1,139			
1,010.0		6,536	100.0		5,831			
1,011.0		9,073	100.0	•	13,636			
1,012.0		11,663	100.0	10,368	24,004			
	· · · · · · · · · · · · · · · · · · ·		100.0	•	36,991			
1,013.5	0	15,658	100.0	7,492	44,483			
Device	Routing	ln۱	vert C	Outlet Devices				
#1	Device 4	1,008	.00' <b>1</b>	.44" Vert. Water C	Quality Orifice C= 0	0.600		
			_	Limited to weir flow at low heads				
#2	Secondary	1,013		10.0' long + 3.0 '/' SideZ x 4.0' breadth Spillway				
				Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00				
				2.50 3.00 3.50 4.0				
						2.67 2.67 2.65 2.66 2.66		
#3	Device 4	1,012		2.68 2.72 2.73 2.76 2.79 2.88 3.07 3.32 <b>27.50" x 27.50" Horiz. Rim</b> C= 0.600				
#3	Device 4	1,012		imited to weir flow				
#4	Primary	1,008				to weir flow at low heads		
#5	Device 4	1,010		8.00" Vert. Outlet C= 0.600 Limited to weir flow at low heads 80.00" W x 5.00" H Vert. WQV Window C= 0.600				
		.,		Limited to weir flow at low heads				
#6	Device 4	1,011	.80' <b>4</b>	8.00" W x 4.00" H	Vert. Orifice/Grate	C= 0.600		
				imited to weir flow	at low heads			

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Primary OutFlow Max=3.21 cfs @ 12.48 hrs HW=1,010.85' (Free Discharge)

4=Outlet (Passes 3.21 cfs of 12.34 cfs potential flow)

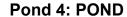
1=Water Quality Orifice (Orifice Controls 0.09 cfs @ 8.05 fps)

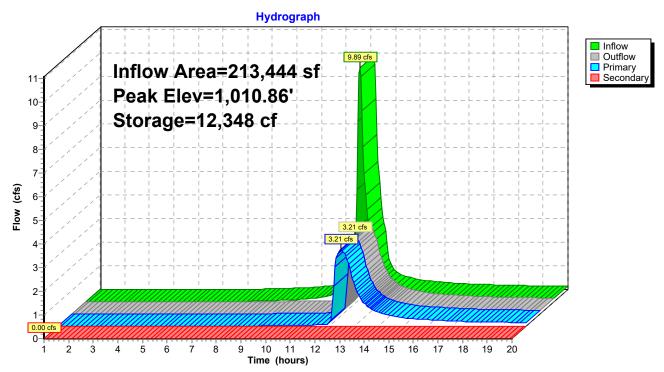
3=Rim (Controls 0.00 cfs)

5=WQV Window (Orifice Controls 3.12 cfs @ 2.99 fps)

6=Orifice/Grate (Controls 0.00 cfs)

Secondary OutFlow Max=0.00 cfs @ 1.00 hrs HW=1,008.00' (Free Discharge) 2=Spillway (Controls 0.00 cfs)





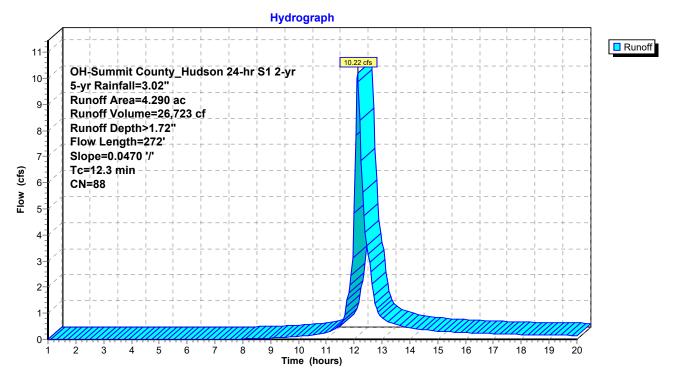
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# **Summary for Subcatchment 1.W: EDA-WEST**

Runoff = 10.22 cfs @ 12.12 hrs, Volume= 26,723 cf, Depth> 1.72"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-20.00 hrs, dt= 0.05 hrs OH-Summit County Hudson 24-hr S1 2-yr 5-yr Rainfall=3.02"

_	Area	(ac) C	N Des	Description						
	3.	075	84 50-7	5% Grass	cover, Fair	HSG D				
_	1.	215	98 Pave	ed parking	, HSG D					
	4.290 88 Weighted Average									
	3.075 71.68% Pervious Area									
	1.	215	28.3	2% Imperv	∕ious Area					
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description				
	11.0	150	0.0470	0.23		Sheet Flow,				
	1.3	122	0.0470	1.52		Grass: Short n= 0.150 P2= 2.44" <b>Shallow Concentrated Flow,</b> Short Grass Pasture Kv= 7.0 fps				
	12.3	272	Total	·						



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# **Summary for Subcatchment 2.W: PDA-WEST**

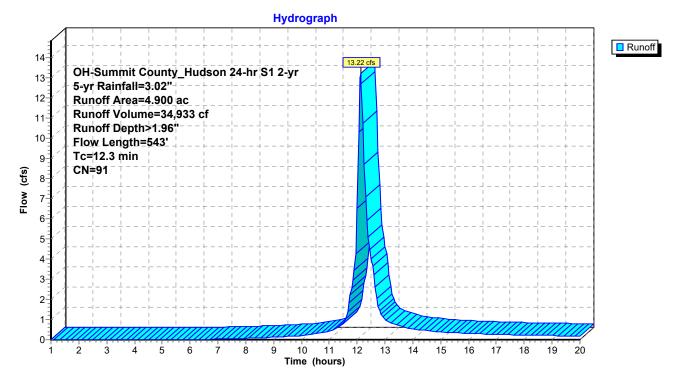
Runoff = 13.22 cfs @ 12.12 hrs, Volume= 34,933 cf, Depth> 1.96"

Routed to Pond 4: POND

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-20.00 hrs, dt= 0.05 hrs OH-Summit County Hudson 24-hr S1 2-yr 5-yr Rainfall=3.02"

	Area	(ac) (	CN Des	cription				
*	1.	020	95 Perr	neable Tui	rf Field, HS	G D		
	2.	080	84 50-7	'5% Grass	cover, Fair	, HSG D		
	1.	800	98 Pav	ed parking	, HSG D			
4.900 91 Weighted Average								
		100		63.27% Pervious Area				
	_	800	36.7	3% Imper	vious Area			
1.000 00.1070 Importious 7 ii du								
	Tc	Length	Slope	Velocity	Capacity	Description		
	(min)	(feet)	•	(ft/sec)	(cfs)	•		
_	9.9	43		0.07	, ,	Sheet Flow,		
	0.0	.0	0.0000	0.01		Grass: Short n= 0.150 P2= 2.44"		
	1.9	300	0.0050	2.63	0.52			
		000	0.0000	2.00	0.02	6.00" Round Area= 0.2 sf Perim= 1.6' r= 0.13'		
						n= 0.010		
	0.5	200	0.0100	6.84	8.40	Pipe Channel,		
	0.0	200	0.0100	0.01	0.10	15.00" Round Area= 1.2 sf Perim= 3.9' r= 0.31'		
						n= 0.010		
_	12.3	543	Total					

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## **Summary for Pond 4: POND**

Inflow Area = 213,444 sf, 36.73% Impervious, Inflow Depth > 1.96" for 5-yr event

Inflow 13.22 cfs @ 12.12 hrs, Volume= 34,933 cf

4.52 cfs @ 12.45 hrs, Volume= 4.52 cfs @ 12.45 hrs, Volume= Outflow 26,980 cf, Atten= 66%, Lag= 19.8 min

Primary 26,980 cf

Routed to nonexistent node 5L

Secondary = 0.00 cfs @ 1.00 hrs, Volume= 0 cf

Routed to nonexistent node 5L

Routing by Stor-Ind method, Time Span= 1.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 1,011.24' @ 12.45 hrs Surf.Area= 9,692 sf Storage= 15,879 cf

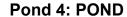
Plug-Flow detention time= 100.3 min calculated for 26,909 cf (77% of inflow)

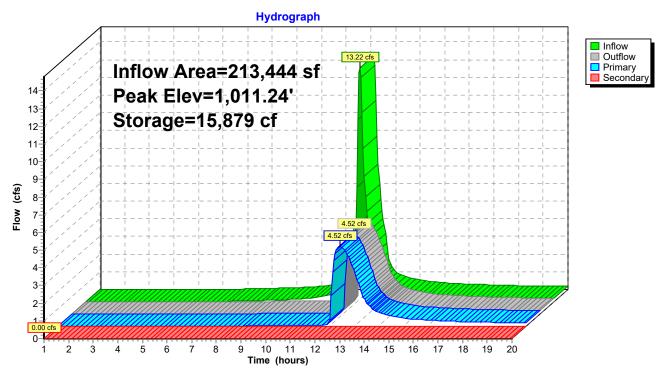
Center-of-Mass det. time= 48.9 min (821.4 - 772.6)

Volume	Invert	Avai	l.Stora	ge Storage Descr	iption			
#1	#1 1,008.00' 44,483 cf		cf Custom Stage	e Data (Prismatic)Lis	sted below (Recalc)			
	0			. 01	0 01			
		Voids		Cum.Store				
(feet	,	(sq-ft)	(%)		(cubic-feet)			
1,008.0		2,848	0.0		0			
1,009.0		2,848	40.0	,	1,139			
1,010.0		6,536	100.0	•	5,831			
1,011.0		9,073	100.0	•	13,636			
1,012.0		11,663	100.0	•	24,004			
1,013.0	0	14,311	100.0	12,987	36,991			
1,013.5	0	15,658	100.0	7,492	44,483			
Device	Routing	ln	vert	Outlet Devices				
#1	Device 4	1,008	.00'	1.44" Vert. Water C	Quality Orifice C= 0	.600		
		•		Limited to weir flow at low heads				
#2	Secondary	1,013	.00'	10.0' long + 3.0 '/' SideZ x 4.0' breadth Spillway				
	•			Head (feet) 0.20 0.	40 0.60 0.80 1.00	1.20 1.40 1.60 1.80 2.00		
				2.50	0 4.50 5.00 5.50			
				Coef. (English) 2.38	3 2.54 2.69 2.68 2.	.67 2.67 2.65 2.66 2.66		
				2.68 2.72 2.73 2.76 2.79 2.88 3.07 3.32				
#3	Device 4	1,012	.80'	27.50" x 27.50" Ho	riz. Rim C= 0.600			
		•		Limited to weir flow	at low heads			
#4	Primary	1,008	.00'	18.00" Vert. Outlet	C= 0.600 Limited	to weir flow at low heads		
#5	Device 4	1,010	.25'	30.00" W x 5.00" H	Vert. WQV Window	C= 0.600		
		,		Limited to weir flow	at low heads			
#6	Device 4	1,011	.80'	48.00" W x 4.00" H	Vert. Orifice/Grate	C= 0.600		
		•		Limited to weir flow	at low heads			

Primary OutFlow Max=4.52 cfs @ 12.45 hrs HW=1,011.24' (Free Discharge)
4=Outlet (Passes 4.52 cfs of 13.42 cfs potential flow)
1=Water Quality Orifice (Orifice Controls 0.10 cfs @ 8.59 fps)
3=Rim (Controls 0.00 cfs)
5=WQV Window (Orifice Controls 4.42 cfs @ 4.24 fps)
6=Orifice/Grate (Controls 0.00 cfs)

Secondary OutFlow Max=0.00 cfs @ 1.00 hrs HW=1,008.00' (Free Discharge) 2=Spillway (Controls 0.00 cfs)





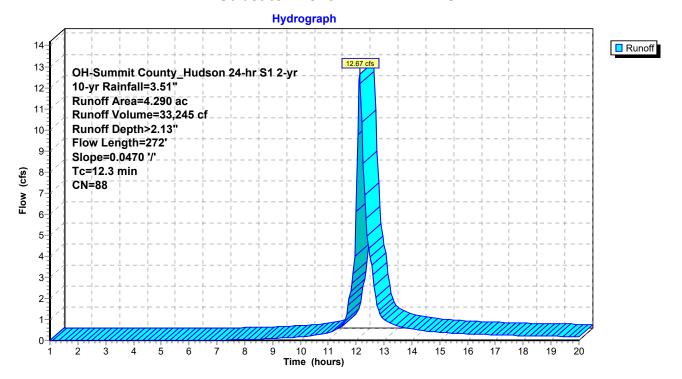
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# **Summary for Subcatchment 1.W: EDA-WEST**

Runoff = 12.67 cfs @ 12.12 hrs, Volume= 33,245 cf, Depth> 2.13"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-20.00 hrs, dt= 0.05 hrs OH-Summit County Hudson 24-hr S1 2-yr 10-yr Rainfall=3.51"

_	Area	(ac) C	N Des	Description						
	3.075 84 50-75% Grass cover, Fair, HSG D									
_	1.215 98 Paved parking, HSG D									
	4.290 88 Weighted Average									
	3.075 71.68% Pervious Area									
	1.	215	28.3	2% Imper	/ious Area					
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description				
	11.0	150	0.0470	0.23		Sheet Flow,				
	1.3	122	0.0470	1.52		Grass: Short n= 0.150 P2= 2.44" <b>Shallow Concentrated Flow,</b> Short Grass Pasture Kv= 7.0 fps				
	12.3	272	Total							



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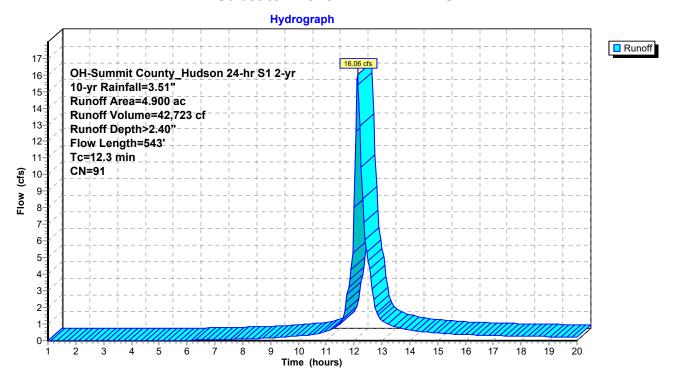
# **Summary for Subcatchment 2.W: PDA-WEST**

Runoff = 16.06 cfs @ 12.12 hrs, Volume= 42,723 cf, Depth> 2.40"

Routed to Pond 4: POND

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-20.00 hrs, dt= 0.05 hrs OH-Summit County Hudson 24-hr S1 2-yr 10-yr Rainfall=3.51"

	Area	(ac) C	N Des	cription				
*	1.	020	95 Perr	neable Tui	rf Field, HS	G D		
	2.	080	84 50-7	5% Grass	cover, Fair	, HSG D		
	1.	800	98 Pave	ed parking	, HSG D			
	4.900 91 Weighted Average			ghted Aver	age			
	3.100		•	63.27% Pervious Area				
	1.	800	36.7	3% Imper	vious Area			
				•				
	Tc	Length	Slope	Velocity	Capacity	Description		
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	·		
	9.9	43	0.0050	0.07	`	Sheet Flow,		
						Grass: Short n= 0.150 P2= 2.44"		
	1.9	300	0.0050	2.63	0.52	Pipe Channel,		
						6.00" Round Area= 0.2 sf Perim= 1.6' r= 0.13'		
						n= 0.010		
	0.5	200	0.0100	6.84	8.40	Pipe Channel,		
						15.00" Round Area= 1.2 sf Perim= 3.9' r= 0.31'		
						n= 0.010		
	12.3	543	Total					



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### **Summary for Pond 4: POND**

Inflow Area = 213,444 sf, 36.73% Impervious, Inflow Depth > 2.40" for 10-yr event

Inflow 16.06 cfs @ 12.12 hrs, Volume= 42,723 cf

5.38 cfs @ 12.45 hrs, Volume= 5.38 cfs @ 12.45 hrs, Volume= Outflow 34,696 cf, Atten= 66%, Lag= 19.9 min

Primary 34,696 cf

Routed to nonexistent node 5L

Secondary = 0.00 cfs @ 1.00 hrs, Volume= 0 cf

Routed to nonexistent node 5L

Routing by Stor-Ind method, Time Span= 1.00-20.00 hrs, dt= 0.05 hrs

Peak Elev= 1,011.57' @ 12.45 hrs Surf.Area= 10,550 sf Storage= 19,233 cf

Plug-Flow detention time= 95.6 min calculated for 34,605 cf (81% of inflow)

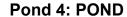
Center-of-Mass det. time= 48.7 min (817.1 - 768.3)

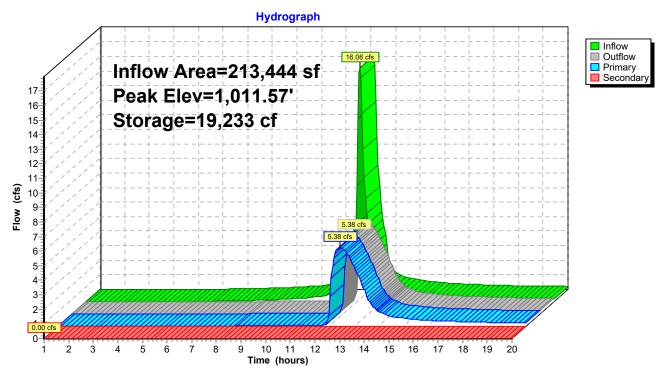
Volume	Invert	Avail	.Stora	ge Storage Descr	iption	
#1	1,008.00'	۷	14,483	cf Custom Stage	e Data (Prismatic)Li	sted below (Recalc)
Elevatio	n Su	rf.Area	Voids	Inc.Store	Cum.Store	
(fee		(sq-ft)	(%)	(cubic-feet)	(cubic-feet)	
1,008.0	/	2,848	0.0		0	
1,009.0		2,848	40.0		1,139	
1,010.0		6,536	100.0		5,831	
1,011.0		9,073	100.0	•	13,636	
1,012.0		11,663	100.0	10,368	24,004	
1,013.0		14,311	100.0	•	36,991	
1,013.5	0	15,658	100.0	7,492	44,483	
Device	Routing	ln۱	vert C	Outlet Devices		
#1	Device 4	1,008	.00' <b>1</b>	.44" Vert. Water C	Quality Orifice C= 0	0.600
			_	imited to weir flow		
#2	Secondary	1,013			SideZ x 4.0' bread	
						1.20 1.40 1.60 1.80 2.00
				2.50 3.00 3.50 4.0		
					3 2.54 2.69 2.68 2 6 2.79 2.88 3.07 3	2.67 2.67 2.65 2.66 2.66
#3	Device 4	1,012	· ·	2.7.50" x 27.50" Ho		3.32
#3	Device 4	1,012		imited to weir flow		
#4	Primary	1,008				to weir flow at low heads
#5	Device 4	1,010			Vert. WQV Window	
		.,		imited to weir flow		
#6	Device 4	1,011	.80' <b>4</b>	8.00" W x 4.00" H	Vert. Orifice/Grate	C= 0.600
			L	imited to weir flow	at low heads	

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Primary OutFlow Max=5.38 cfs @ 12.45 hrs HW=1,011.57' (Free Discharge)
4=Outlet (Passes 5.38 cfs of 14.29 cfs potential flow)
1=Water Quality Orifice (Orifice Controls 0.10 cfs @ 9.02 fps)
3=Rim (Controls 0.00 cfs)
5=WQV Window (Orifice Controls 5.28 cfs @ 5.07 fps)
6=Orifice/Grate (Controls 0.00 cfs)

Secondary OutFlow Max=0.00 cfs @ 1.00 hrs HW=1,008.00' (Free Discharge) 2=Spillway (Controls 0.00 cfs)





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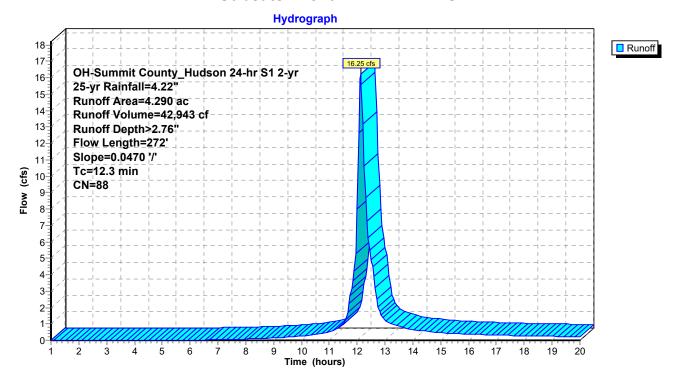
### **Summary for Subcatchment 1.W: EDA-WEST**

Runoff = 16.25 cfs @ 12.12 hrs, Volume= 42,943 cf, Depth> 2.76"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-20.00 hrs, dt= 0.05 hrs OH-Summit County Hudson 24-hr S1 2-yr 25-yr Rainfall=4.22"

_	Area	(ac) C	N Des	cription					
	3.075 84 50-75% Grass cover, Fair, HSG D								
_	1.215 98 Paved parking, HSG D								
	4.290 88 Weighted Average								
	3.	075	71.6	8% Pervio	us Area				
	1.	215	28.3	2% Imper	/ious Area				
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
	11.0	150	0.0470	0.23		Sheet Flow,			
	1.3	122	0.0470	1.52		Grass: Short n= 0.150 P2= 2.44" <b>Shallow Concentrated Flow,</b> Short Grass Pasture Kv= 7.0 fps			
	12.3	272	Total						

### **Subcatchment 1.W: EDA-WEST**



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### **Summary for Subcatchment 2.W: PDA-WEST**

Runoff = 20.16 cfs @ 12.12 hrs, Volume= 54,196 cf, Depth> 3.05"

Routed to Pond 4: POND

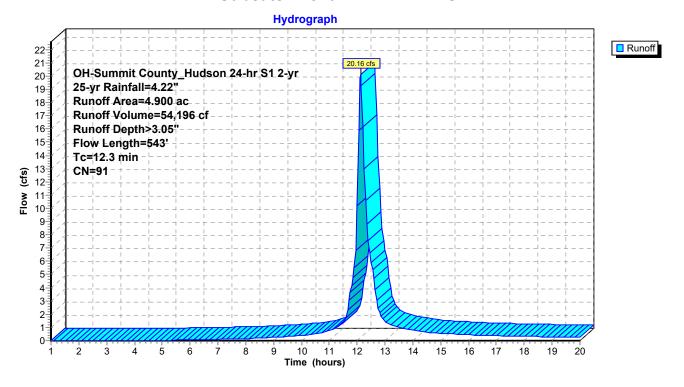
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-20.00 hrs, dt= 0.05 hrs OH-Summit County Hudson 24-hr S1 2-yr 25-yr Rainfall=4.22"

	Area	(ac) C	N Des	cription				
*	1.	020	95 Perr	neable Tui	rf Field, HS	G D		
	2.	080	84 50-7	5% Grass	cover, Fair	, HSG D		
	1.	800	98 Pave	ed parking	, HSG D			
	4.	900	91 Wei	ghted Aver	age			
	3.	100	•	63.27% Pervious Area				
	1.	800	36.7	3% Imper	vious Area			
				•				
	Tc	Length	Slope	Velocity	Capacity	Description		
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	·		
	9.9	43	0.0050	0.07	`	Sheet Flow,		
						Grass: Short n= 0.150 P2= 2.44"		
	1.9	300	0.0050	2.63	0.52	Pipe Channel,		
						6.00" Round Area= 0.2 sf Perim= 1.6' r= 0.13'		
						n= 0.010		
	0.5	200	0.0100	6.84	8.40	Pipe Channel,		
						15.00" Round Area= 1.2 sf Perim= 3.9' r= 0.31'		
						n= 0.010		
	12.3	543	Total					

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### **Subcatchment 2.W: PDA-WEST**



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### **Summary for Pond 4: POND**

Inflow Area = 213,444 sf, 36.73% Impervious, Inflow Depth > 3.05" for 25-yr event

Inflow 20.16 cfs @ 12.12 hrs, Volume= 54,196 cf

7.30 cfs @ 12.42 hrs, Volume= 7.30 cfs @ 12.42 hrs, Volume= Outflow 46,066 cf, Atten= 64%, Lag= 17.9 min

Primary 46,066 cf

Routed to nonexistent node 5L

Secondary = 0.00 cfs @ 1.00 hrs, Volume= 0 cf

Routed to nonexistent node 5L

Routing by Stor-Ind method, Time Span= 1.00-20.00 hrs, dt= 0.05 hrs

Peak Elev= 1,011.98' @ 12.42 hrs Surf.Area= 11,618 sf Storage= 23,800 cf

Plug-Flow detention time= 91.0 min calculated for 45,945 cf (85% of inflow)

Center-of-Mass det. time= 49.6 min (812.8 - 763.3)

Volume	Invert	Ava	il.Stor	age	Storage Descrip	tion	
#1	1,008.00'		44,48	3 cf	Custom Stage I	<b>Data (Prismatic)</b> List	ed below (Recalc)
Elevatio	n Sı	ırf.Area	Voic	ls	Inc.Store	Cum.Store	
(feet		(sq-ft)	(%		(cubic-feet)	(cubic-feet)	
1,008.0	,	2,848	0.		0	0	
1,000.0		2,848	40.	-	1,139	1,139	
1,010.0		6,536	100		4,692	5,831	
1,010.0		9,073	100		7,805	13,636	
1,012.0		11,663	100		10,368	24,004	
1,013.0		14,311			12,987	36,991	
1,013.5		15,658			7,492	44,483	
1,010.0	•	10,000	100	•	7,102	11,100	
Device	Routing	In	vert	Outl	et Devices		
#1	Device 4	1,008	3.00'	1.44	" Vert. Water Qu	ality Orifice C= 0.0	500
		,			ted to weir flow at	•	
#2	Secondary	1,013	3.00'	10.0	' long + 3.0 '/' Si	deZ x 4.0' breadth	Spillway
	•			Hea	d (feet) 0.20 0.40	0.60 0.80 1.00 1	.20 1.40 1.60 1.80 2.00
				2.50	3.00 3.50 4.00	4.50 5.00 5.50	
				Coe	f. (English) 2.38	2.54 2.69 2.68 2.6	7 2.67 2.65 2.66 2.66
						2.79 2.88 3.07 3.	
#3	Device 4	1,012	2.80'	27.5	0" x 27.50" Horiz	<b> Rim</b> C= 0.600	
		,		Limi	ted to weir flow at	low heads	
#4	Primary	1,008	3.00'	18.0	0" Vert. Outlet	C= 0.600 Limited to	weir flow at low heads
#5	Device 4	1,010	).25'	30.0	0" W x 5.00" H V	ert. WQV Window	C= 0.600
				Limi	ted to weir flow at	low heads	
#6	Device 4	1,011	1.80'	48.0	0" W x 4.00" H V	ert. Orifice/Grate	C= 0.600
				Limi	ted to weir flow at	low heads	

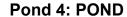
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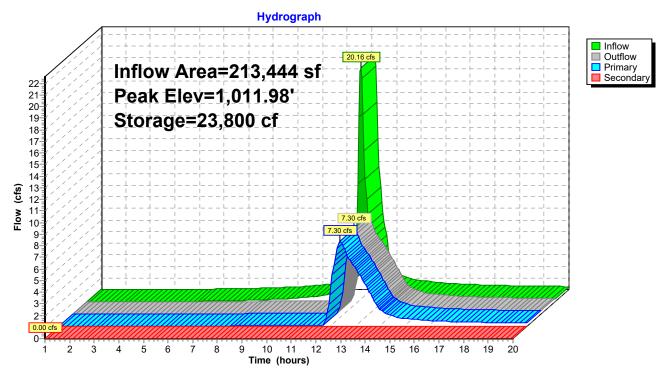
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Primary OutFlow Max=7.28 cfs @ 12.42 hrs HW=1,011.98' (Free Discharge)
4=Outlet (Passes 7.28 cfs of 15.29 cfs potential flow)
1=Water Quality Orifice (Orifice Controls 0.11 cfs @ 9.53 fps)
3=Rim (Controls 0.00 cfs)

**-5=WQV Window** (Orifice Controls 6.18 cfs @ 5.94 fps) **-6=Orifice/Grate** (Orifice Controls 0.98 cfs @ 1.36 fps)

Secondary OutFlow Max=0.00 cfs @ 1.00 hrs HW=1,008.00' (Free Discharge) —2=Spillway (Controls 0.00 cfs)





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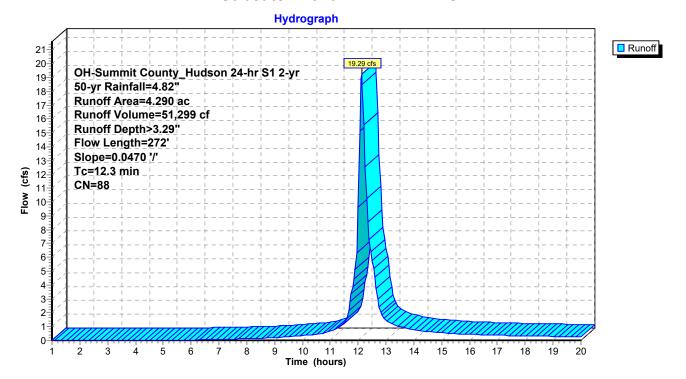
### Summary for Subcatchment 1.W: EDA-WEST

Runoff = 19.29 cfs @ 12.12 hrs, Volume= 51,299 cf, Depth> 3.29"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-20.00 hrs, dt= 0.05 hrs OH-Summit County Hudson 24-hr S1 2-yr 50-yr Rainfall=4.82"

_	Area	(ac) C	N Des	cription					
	3.075 84 50-75% Grass cover, Fair, HSG D								
_	1.215 98 Paved parking, HSG D								
	4.290 88 Weighted Average								
	3.	075	71.6	8% Pervio	us Area				
	1.	215	28.3	2% Imper	/ious Area				
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
	11.0	150	0.0470	0.23		Sheet Flow,			
	1.3	122	0.0470	1.52		Grass: Short n= 0.150 P2= 2.44" <b>Shallow Concentrated Flow,</b> Short Grass Pasture Kv= 7.0 fps			
	12.3	272	Total						

### **Subcatchment 1.W: EDA-WEST**



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### **Summary for Subcatchment 2.W: PDA-WEST**

Runoff = 23.62 cfs @ 12.12 hrs, Volume= 64,006 cf, Depth> 3.60"

Routed to Pond 4: POND

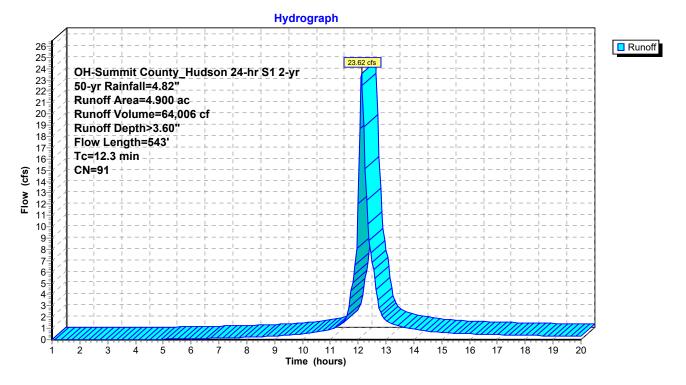
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-20.00 hrs, dt= 0.05 hrs OH-Summit County Hudson 24-hr S1 2-yr 50-yr Rainfall=4.82"

	Area	(ac) (	CN Des	cription		
*	1.	020	95 Perr	neable Tui	rf Field, HS	G D
	2.	080	84 50-7	'5% Grass	cover, Fair	, HSG D
	1.	800	98 Pav	ed parking	, HSG D	
_	4.	900	91 Wei	ghted Aver	age	
		100		7% Pervio	•	
	_	800	36.7	3% Imper	vious Area	
	Tc	Length	Slope	Velocity	Capacity	Description
	(min)	(feet)	•	(ft/sec)	(cfs)	•
_	9.9	43		0.07	, ,	Sheet Flow,
	0.0	.0	0.0000	0.01		Grass: Short n= 0.150 P2= 2.44"
	1.9	300	0.0050	2.63	0.52	
		000	0.0000	2.00	0.02	6.00" Round Area= 0.2 sf Perim= 1.6' r= 0.13'
						n= 0.010
	0.5	200	0.0100	6.84	8.40	Pipe Channel,
	0.0	200	0.0100	0.01	0.10	15.00" Round Area= 1.2 sf Perim= 3.9' r= 0.31'
						n= 0.010
_	12.3	543	Total			

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### **Subcatchment 2.W: PDA-WEST**



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### **Summary for Pond 4: POND**

Inflow Area = 213,444 sf, 36.73% Impervious, Inflow Depth > 3.60" for 50-yr event

Inflow 23.62 cfs @ 12.12 hrs, Volume= 64,006 cf

9.86 cfs @ 12.37 hrs, Volume= 9.86 cfs @ 12.37 hrs, Volume= Outflow 55,791 cf, Atten= 58%, Lag= 14.9 min

Primary 55,791 cf

Routed to nonexistent node 5L

Secondary = 0.00 cfs @ 1.00 hrs, Volume= 0 cf

Routed to nonexistent node 5L

Routing by Stor-Ind method, Time Span= 1.00-20.00 hrs, dt= 0.05 hrs

Peak Elev= 1,012.21' @ 12.37 hrs Surf.Area= 12,227 sf Storage= 26,549 cf

Plug-Flow detention time= 86.8 min calculated for 55,791 cf (87% of inflow)

Center-of-Mass det. time= 48.4 min (808.1 - 759.7)

Volume	Invert	Avai	l.Stora	ge Storage Descr	iption	
#1	1,008.00'	4	44,483	cf Custom Stag	e Data (Prismatic)Listed b	elow (Recalc)
<b>-</b>	0			. 0	0 01	
Elevatio		rf.Area	Voids		Cum.Store	
(feet	,	(sq-ft)	(%		(cubic-feet)	
1,008.0		2,848	0.0		0	
1,009.0		2,848	40.0	•	1,139	
1,010.0		6,536	100.0	•	5,831	
1,011.0	0	9,073	100.0		13,636	
1,012.0		11,663	100.0	•	24,004	
1,013.0	0	14,311	100.0	12,987	36,991	
1,013.5	0	15,658	100.0	7,492	44,483	
Device	Routing			Outlet Devices		
#1	Device 4	1,008			Quality Orifice C= 0.600	
				Limited to weir flow		
#2	Secondary	1,013			SideZ x 4.0' breadth Spi	
					.40 0.60 0.80 1.00 1.20	1.40 1.60 1.80 2.00
				2.50 3.00 3.50 4.0		
					8 2.54 2.69 2.68 2.67 2	.67 2.65 2.66 2.66
				2.68 2.72 2.73 2.7	6 2.79 2.88 3.07 3.32	
#3	Device 4	1,012	.80'	27.50" x 27.50" Ho	riz. Rim C= 0.600	
				Limited to weir flow	at low heads	
#4	Primary	1,008	.00'	18.00" Vert. Outlet	C= 0.600 Limited to we	ir flow at low heads
#5	Device 4	1,010	.25'	30.00" W x 5.00" H	Vert. WQV Window C=	0.600
				Limited to weir flow	at low heads	
#6	Device 4	1,011	.80'	48.00" W x 4.00" H	Vert. Orifice/Grate C= 0	.600
				Limited to weir flow	at low heads	

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Primary OutFlow Max=9.84 cfs @ 12.37 hrs HW=1,012.21' (Free Discharge)

4=Outlet (Passes 9.84 cfs of 15.83 cfs potential flow)

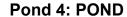
1=Water Quality Orifice (Orifice Controls 0.11 cfs @ 9.81 fps)

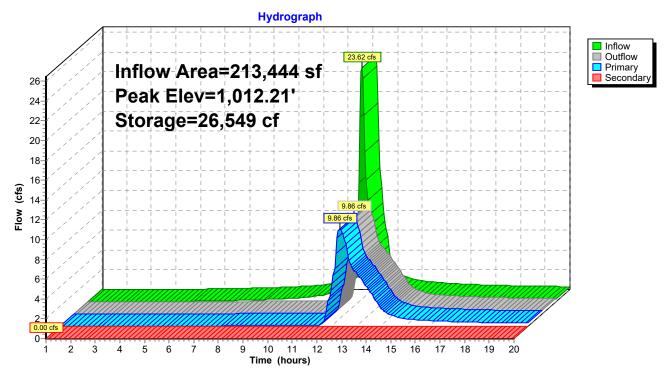
3=Rim (Controls 0.00 cfs)

5=WQV Window (Orifice Controls 6.63 cfs @ 6.37 fps)

6=Orifice/Grate (Orifice Controls 3.10 cfs @ 2.32 fps)

Secondary OutFlow Max=0.00 cfs @ 1.00 hrs HW=1,008.00' (Free Discharge) 2=Spillway (Controls 0.00 cfs)





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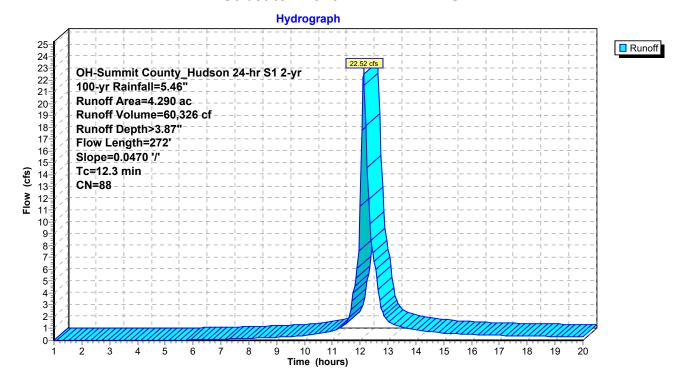
### **Summary for Subcatchment 1.W: EDA-WEST**

Runoff = 22.52 cfs @ 12.12 hrs, Volume= 60,326 cf, Depth> 3.87"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-20.00 hrs, dt= 0.05 hrs OH-Summit County Hudson 24-hr S1 2-yr 100-yr Rainfall=5.46"

_	Area	(ac) C	N Des	cription					
	3.075 84 50-75% Grass cover, Fair, HSG D								
_	1.	215	98 Pave	ed parking	, HSG D				
	4.290 88 Weighted Average								
	3.075 71.68% Pervious Area								
	1.	215	28.3	2% Imperv	∕ious Area				
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
	11.0	150	0.0470	0.23		Sheet Flow,			
	1.3	122	0.0470	1.52		Grass: Short n= 0.150 P2= 2.44" <b>Shallow Concentrated Flow,</b> Short Grass Pasture Kv= 7.0 fps			
	12.3	272	Total						

### **Subcatchment 1.W: EDA-WEST**



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### **Summary for Subcatchment 2.W: PDA-WEST**

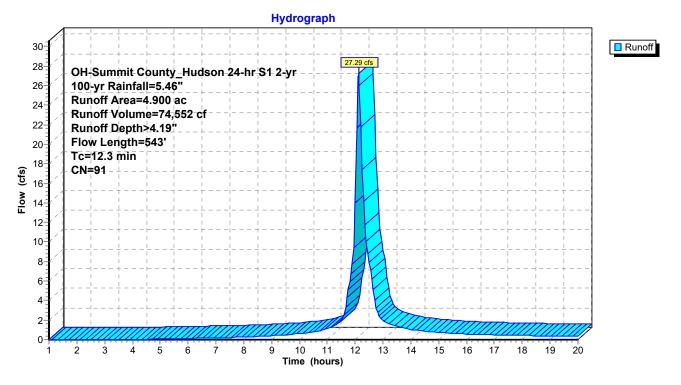
Runoff = 27.29 cfs @ 12.12 hrs, Volume= 74,552 cf, Depth> 4.19"

Routed to Pond 4: POND

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-20.00 hrs, dt= 0.05 hrs OH-Summit County Hudson 24-hr S1 2-yr 100-yr Rainfall=5.46"

	Area	(ac) C	N Des	cription		
*	1.	020 9	95 Pern	neable Tui	f Field, HS	G D
	2.	080			cover, Fair	
	1.	800		ed parking	,	,
_				ghted Aver		
		100		7% Pervio		
		800			/ious Area	
	١.	000	50.1	570 Imper	nous Arca	
	Tc	Length	Slope	Velocity	Capacity	Description
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	Boompton
_	9.9	43	0.0050	0.07	(0.0)	Sheet Flow,
	9.9	43	0.0030	0.07		Grass: Short n= 0.150 P2= 2.44"
	1.0	200	0.0050	2.62	0.52	5.5.5. 5.1.6.1 61.166 . <u>-</u>
	1.9	300	0.0050	2.63	0.52	•
						6.00" Round Area= 0.2 sf Perim= 1.6' r= 0.13'
	٥-	000	0.0400	0.04	0.40	n= 0.010
	0.5	200	0.0100	6.84	8.40	Pipe Channel,
						15.00" Round Area= 1.2 sf Perim= 3.9' r= 0.31'
_						n= 0.010
	12.3	543	Total			

### **Subcatchment 2.W: PDA-WEST**



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### **Summary for Pond 4: POND**

213,444 sf, 36.73% Impervious, Inflow Depth > 4.19" for 100-yr event Inflow Area =

Inflow 27.29 cfs @ 12.12 hrs, Volume= 74,552 cf

11.65 cfs @ 12.36 hrs, Volume= 11.65 cfs @ 12.36 hrs, Volume= Outflow 66,246 cf, Atten= 57%, Lag= 14.4 min

Primary 66,246 cf

Routed to nonexistent node 5L

Secondary = 0.00 cfs @ 1.00 hrs, Volume= 0 cf

Routed to nonexistent node 5L

Routing by Stor-Ind method, Time Span= 1.00-20.00 hrs, dt= 0.05 hrs

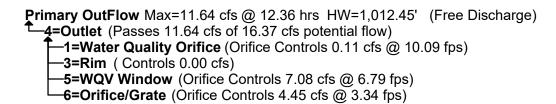
Peak Elev= 1,012.45' @ 12.36 hrs Surf.Area= 12,864 sf Storage= 29,567 cf

Plug-Flow detention time= 82.8 min calculated for 66,246 cf (89% of inflow)

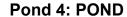
Center-of-Mass det. time= 47.8 min (804.2 - 756.4)

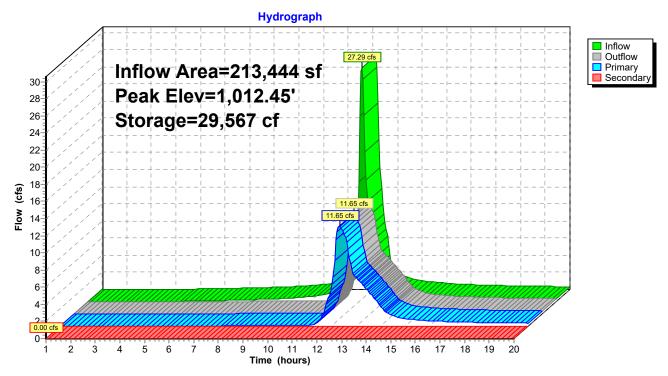
Volume	Invert	Avai	l.Stora	age Storage Desci	ription	
#1	1,008.00'	4	44,48	3 cf Custom Stag	<b>e Data (Prismatic)</b> Lis	ted below (Recalc)
Elevatio	n Su	rf.Area	Void:	s Inc.Store	Cum.Store	
fee!		(sq-ft)	(%		(cubic-feet)	
1,008.0	,	2,848	0.0	, , , , , , , , , , , , , , , , , , , ,	0	
1,009.0		2,848	40.0		1,139	
1,010.0	0	6,536	100.0		5,831	
1,011.0		9,073	100.0	•	13,636	
1,012.0		11,663	100.0	•	24,004	
1,013.0		14,311	100.0	•	36,991	
1,013.5	0	15,658	100.0	7,492	44,483	
Device	Routing	In	vert	Outlet Devices		
#1	Device 4	1,008			Quality Orifice C= 0.	600
				Limited to weir flow		
#2	Secondary	1,013	.00'		SideZ x 4.0' breadth	
				` ,		1.20 1.40 1.60 1.80 2.00
				2.50 3.00 3.50 4.0		27 0 07 0 05 0 00 0 00
				` ` ,	8 2.54 2.69 2.68 2.6 76 2.79 2.88 3.07 3.	37 2.67 2.65 2.66 2.66
#3	Device 4	1,012	אַחי	27.50" x 27.50" Ho		32
#3	Device 4	1,012		Limited to weir flow		
#4	Primary	1,008				o weir flow at low heads
#5	Device 4	1,010			Vert. WQV Window	
		,		Limited to weir flow		
#6	Device 4	1,011	.80'	48.00" W x 4.00" H	Vert. Orifice/Grate	C= 0.600
				Limited to weir flow	at low heads	

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Secondary OutFlow Max=0.00 cfs @ 1.00 hrs HW=1,008.00' (Free Discharge) 2=Spillway (Controls 0.00 cfs)





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### **Events for Subcatchment 1.W: EDA-WEST**

Event	Rainfall	Runoff	Volume	Depth
	(inches)	(cfs)	(cubic-feet)	(inches)
1-yr	2.04	5.42	14,389	0.92
2-yr	2.44	7.38	19,275	1.24
5-yr	3.02	10.22	26,723	1.72
10-yr	3.51	12.67	33,245	2.13
25-yr	4.22	16.25	42,943	2.76
50-yr	4.82	19.29	51,299	3.29
100-yr	5.46	22.52	60,326	3.87

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### **Events for Subcatchment 2.W: PDA-WEST**

Event	Rainfall	Runoff	Volume	Depth
	(inches)	(cfs)	(cubic-feet)	(inches)
1-yr	2.04	7.62	19,900	1.12
2-yr	2.44	9.89	25,919	1.46
5-yr	3.02	13.22	34,933	1.96
10-yr	3.51	16.06	42,723	2.40
25-yr	4.22	20.16	54,196	3.05
50-yr	4.82	23.62	64,006	3.60
100-yr	5.46	27.29	74,552	4.19

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### **Events for Pond 4: POND**

Event	Inflow	Outflow	Primary	Secondary	Elevation	Storage
	(cfs)	(cfs)	(cfs)	(cfs)	(feet)	(cubic-feet)
1-yr	7.62	2.02	2.02	0.00	1,010.64	10,510
2-yr	9.89	3.21	3.21	0.00	1,010.86	12,348
5-yr	13.22	4.52	4.52	0.00	1,011.24	15,879
10-yr	16.06	5.38	5.38	0.00	1,011.57	19,233
25-yr	20.16	7.30	7.30	0.00	1,011.98	23,800
50-yr	23.62	9.86	9.86	0.00	1,012.21	26,549
100-yr	27.29	11.65	11.65	0.00	1,012.45	29,567



# APPENDIX B1: STORMWATER QUALITY CALCULATIONS

## Post-Construction Water Quality Volume As Required Under Ohio NPDES Construction General Permit No. OHC000006

version 1.2 2023-5-15

acres

This spreadsheet calculates the Water Quality Volume required for both new development and redevelopment projects. Green boxes indicate user input for 1) the total area disturbed, 2) planned total impervious surface and, if redevelopment, 3) total existing impervious surface, each in acres. The user must select new or redevelopment from the dropdown menu to apply the proper equation. Use the separate BMP Compliance Spreadsheets to verify a designed practice or combination of practices meets the applicable requirements including the required Water Quality Volume calculated here. This spreadsheet does not account for factors that may affect the final practice design, including offsite run-on or sediment storage volume.

Project Details							
Project Name:	oject Name: Christ Community Chapel						
Project ID:							
Project Location:	750 W Streetsboro	St					
	Hudson, OH 44236						
Project Latitude:	41.23116	Longitude:	-81.48405				
NPDES Permit Applicant:							
Submitted By:	5/19/2025						
Date:	5/14/2025						

Required Water Quality Volume C	alculation			
Total Disturbed Area, A =	3.930	acres		
Type of Development:	Redevelop	ment		
Water Quality Volume Equation:	WQvr = 0	.90 in. * A * [(	[Rv1*0.2)+(Rv2-Rv1)] / 12	[Equation 3]
	where,	Rv = 0.05 + 0.9	9(i)	
PRE-CONSTRUCTION CONDITIONS	_	PROPOSED P	OST-CONSTRUCTION CON	<u>IDITIONS</u>
Ex. Impervious Surface = 0.100	acres	Total Im	pervious Surface Area =	1.910 acres
Ex. Impervious Fraction, i = 0.02!	5		Impervious Fraction, i =	0.486
Rv1 = 0.073	3	Volumetric R	unoff Coefficient, Rv2 =	0.487
		Δ <b>R</b> v =	569 %	

Water Quality Volume, WQv =	0.126 ac-ft	=	5,509 cu. ft.	
_				

**Message Center:** 

The minimum impervious area to treat with a practice is

### **Dry Extended Detention Basin WQv Compliance Tool**

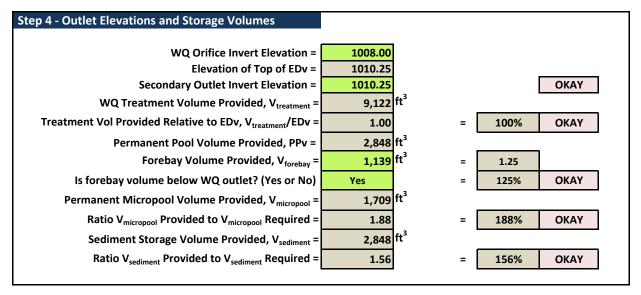
version 3.2 2020-07-07

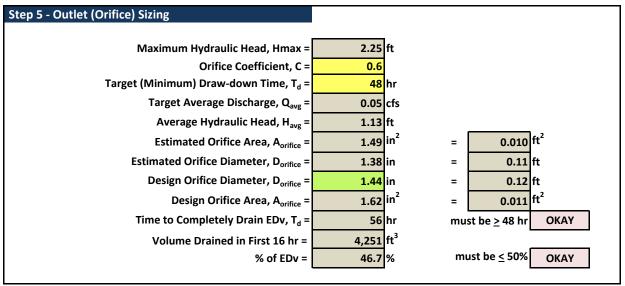
Project Summary				
Project Name:	Christ Commu	nity Chapel		
Subwatershed ID/Label:				
Submitted by:	CESO, Inc.			
Date:	7/7/2025			
		•		
Subwatershed Drainage Area, A <sub>total</sub> =	5.00	acres =	217,800	ft2
Subwatershed Impervious Area, A <sub>imp</sub> =	2.82	acres =	122,839	ft2
Imperviousness fraction, i =	0.56		56	%
Water Quality Volume, WQv =	9,108	ft <sup>3</sup> =	0.21	ac-ft

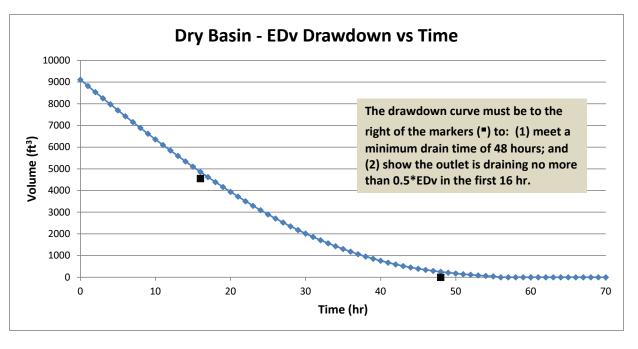
Step 1 - Soil Suitability		
Soil Series	HSG	D
Soli Series	пэс	U

Step 2 - Dry ED Basin Volume Requirements	
Extended Detention Volume, EDv =  Minimum Sediment Storage Volume, V <sub>sediment</sub> =  Minimum Forebay Volume, V <sub>forebay</sub> =  Minimum Permanent Micropool Volume, V <sub>micropool</sub> =	911 ft <sup>3</sup>

tep 3 - Basin Stage-Storage Relationship	Elevation ft	Area ft²	Incremental Volume ft <sup>3</sup>	Cumulative Volume ft <sup>3</sup>
Bottom of Permanent Micropool =	1007.00	2,848		
(include forebay area if below EDv)	1008.00	2,848	2,848	2,848
	1009.00	2,848	2,848	5,696
	1010.00	6,536	4,566	10,262
	1011.00	9,073	7,770	
	1012.00	11,663	10,341	
	1013.00	14,311	12,964	
	1013.50	15,658	7,490	48,827
l				









# APPENDIX C: STORMWATER PIPE CALCULATIONS

## **MyReport**

Line No.	Line ID	Line Length	Line Size	Line Slope	Drng Area	Total Area	Flow Rate	Capac Full	Invert Dn	Invert Up	HGL Dn	Gnd/Rim El Dn	HGL Up	Gnd/Rim El Up	Junct Type	Inlet Depth	Vel Ave	Cover Up	Тс	
		(ft)	(in)	(%)	(ac)	(ac)	(cfs)	(cfs)	(ft)	(ft)	(ft)	(ft)	(ft)	(ft)		(ft)	(ft/s)	(ft)	(min)	
1	35	33.236	18	1.50	0.52	1.32	4.03	12.88	1009.50	1010.00	1010.08	1011.71	1010.77	1014.95	Comb.	0.30	5.43	3.45	7.4	
2	104	58.165	15	1.00	0.06	0.80	2.35	6.45	1010.00	1010.58	1010.77	1014.95	1011.19 j	1014.25	Comb.	0.10	3.46	2.42	6.9	
3	24	55.426	15	1.01	0.06	0.74	2.29	6.49	1010.58	1011.14	1011.19	1014.25	1011.75 j	1014.44	Comb.	0.10	3.86	2.05	6.5	
4	25	52.202	15	1.00	0.55	0.68	2.23	6.44	1011.14	1011.66	1011.75	1014.44	1012.26 j	1015.25	Comb.	0.30	3.83	2.34	6.0	
5	26	25.304	12	0.99	0.13	0.13	0.33	3.54	1011.66	1011.91	1012.26	1015.25	1012.15	1015.75	Comb.	0.13	1.50	2.84	5.0	
6	27	29.000	24	0.52	0.10	0.57	11.61	16.27	1010.00	1010.15	1011.25	1012.71	1011.40	1013.89	Comb.	0.11	5.62	1.74	6.2	
7	28	164.000	24	0.50	0.13	0.47	11.50	15.99	1010.14	1010.96	1011.65	1013.89	1012.18	1014.88	Comb.	0.12	5.13	1.92	5.4	
8	148	95.991	24	0.50	0.34	0.34	11.24	15.99	1010.96	1011.44	1012.94	1014.88	1013.11	1014.69	Comb.	0.22	3.80	1.25	5.0	
9	150	7.012	18	12.26	0.00	0.00	10.50	36.77	1011.44	1012.30	1013.48	1014.69	1013.54	1016.41	MH		6.32	2.61	0.0	
10	153	34.919	15	9.99	0.10	0.10	0.14	20.41	1006.02	1009.51	1006.10	1017.08	1009.66	1019.50	Comb.	0.11	3.30	8.74	5.0	

Project File: stm.stm Number of lines: 10 Date: 5/9/2025

NOTES: \*\* Critical depth



# APPENDIX D: USDA NRCS Web Soil Survey



#### MAP LEGEND

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Water Features

Transportation

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Background

Spoil Area

Stony Spot

Wet Spot

Other

Rails

**US Routes** 

Major Roads

Local Roads

Very Stony Spot

Special Line Features

Streams and Canals

Interstate Highways

Aerial Photography

#### Area of Interest (AOI)

Area of Interest (AOI)

#### Soils

Soil Map Unit Polygons



Soil Map Unit Points

#### Special Point Features

Blowout

Borrow Pit

Clay Spot

Closed Depression

Gravel Pit

Gravelly Spot

Landfill

Lava Flow

Marsh or swamp

Mine or Quarry

Miscellaneous Water

Perennial Water

→ Saline Spot

Sandy Spot

Severely Eroded Spot

Sinkhole

Slide or Slip

Sodic Spot

### MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:20.000.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service Web Soil Survey URL:

Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: Summit County, Ohio Survey Area Data: Version 21, Aug 29, 2024

Soil map units are labeled (as space allows) for map scales 1:50.000 or larger.

Date(s) aerial images were photographed: Sep 12, 2020—Sep 21, 2020

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Soil Map—Summit County, Ohio CCC Hudson, OH

### **Map Unit Legend**

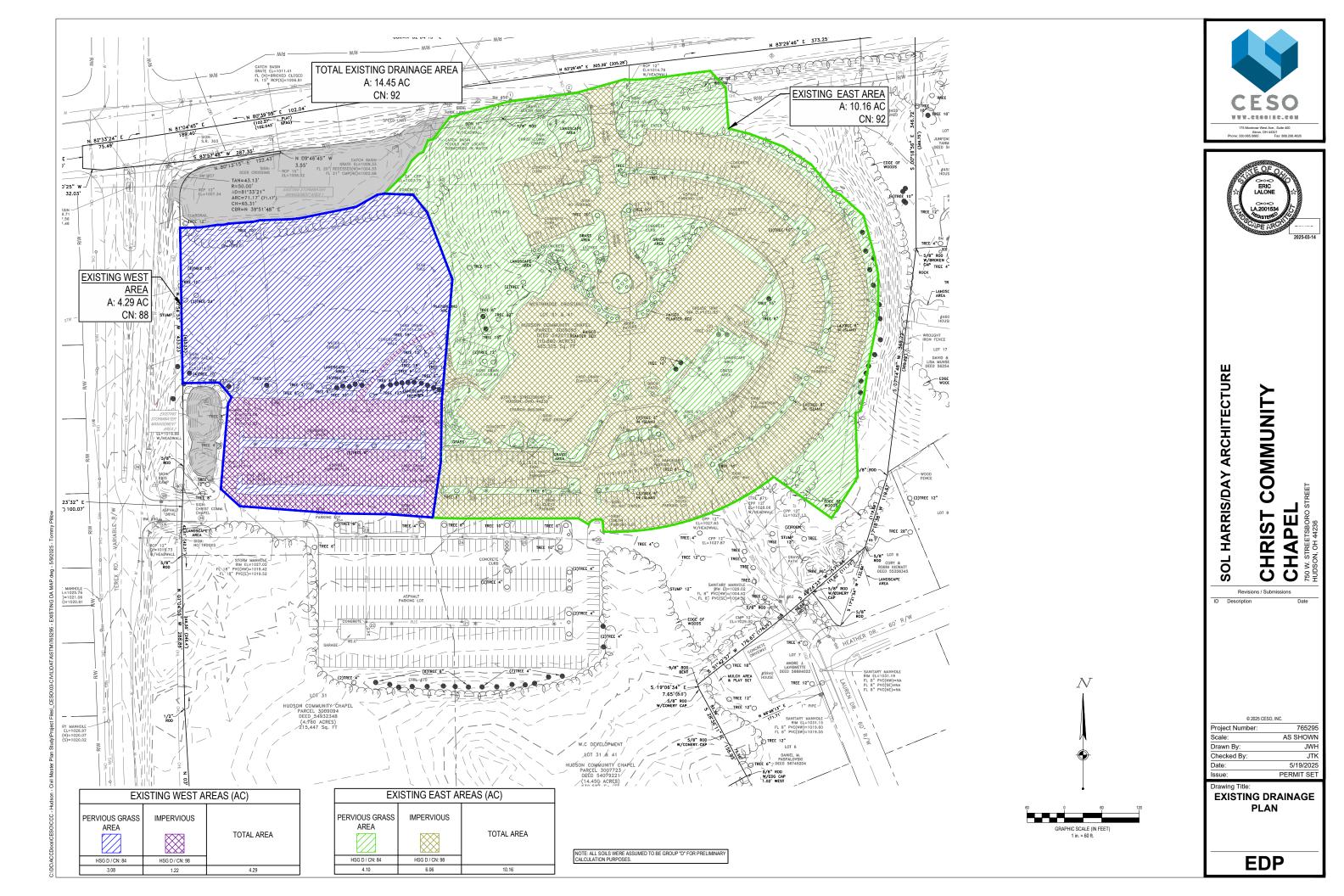
Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI	
Ca	Canadice silty clay loam	0.2	0.5%	
СсВ	Caneadea silt loam, 2 to 6 percent slopes	15.1	46.9%	
CoC2	Chili gravelly loam, 6 to 12 percent slopes, moderately eroded	2.2	6.9%	
EuC	Ellsworth-Urban land complex, 2.3 6 to 18 percent slopes			
GbC2	Geeburg silt loam, 6 to 12 percent slopes, moderately eroded	6.2	19.3%	
GbD2	Geeburg silt loam, 12 to 18 percent slopes, moderately eroded	0.5	1.6%	
Mn	Mahoning-Urban land complex, 0 to 2 percent slopes	2.9	9.0%	
Sb	Sebring silt loam, 0 to 2 percent slopes	0.6	1.9%	
WrB	Wheeling silt loam, 2 to 6 percent slopes	2.2	6.7%	
Totals for Area of Interest		32.2	100.0%	



### APPENDIX E: DRAINAGE AREA MAPS

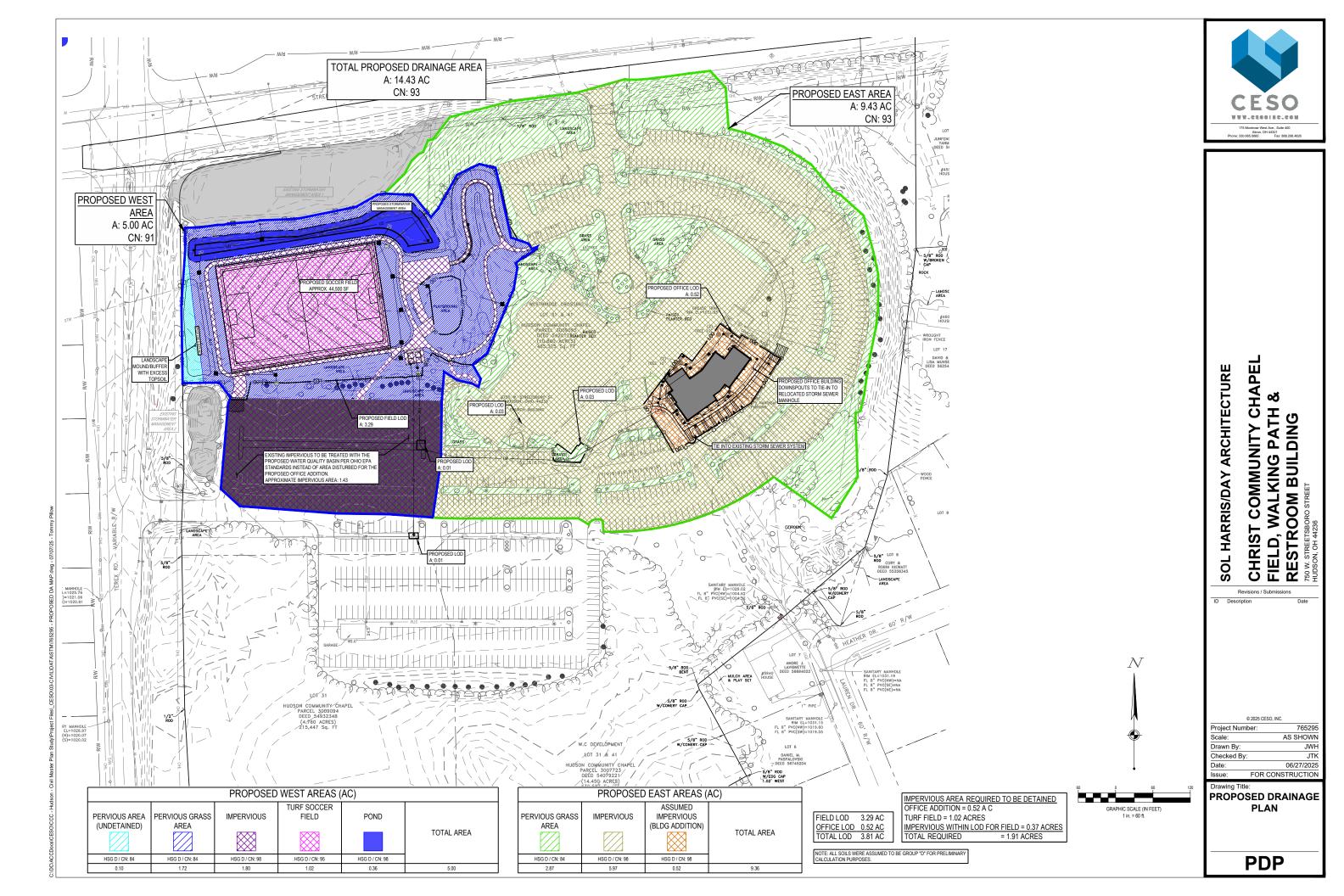


# APPENDIX E1: EXISTING CONDITIONS DRAINAGE AREA MAP



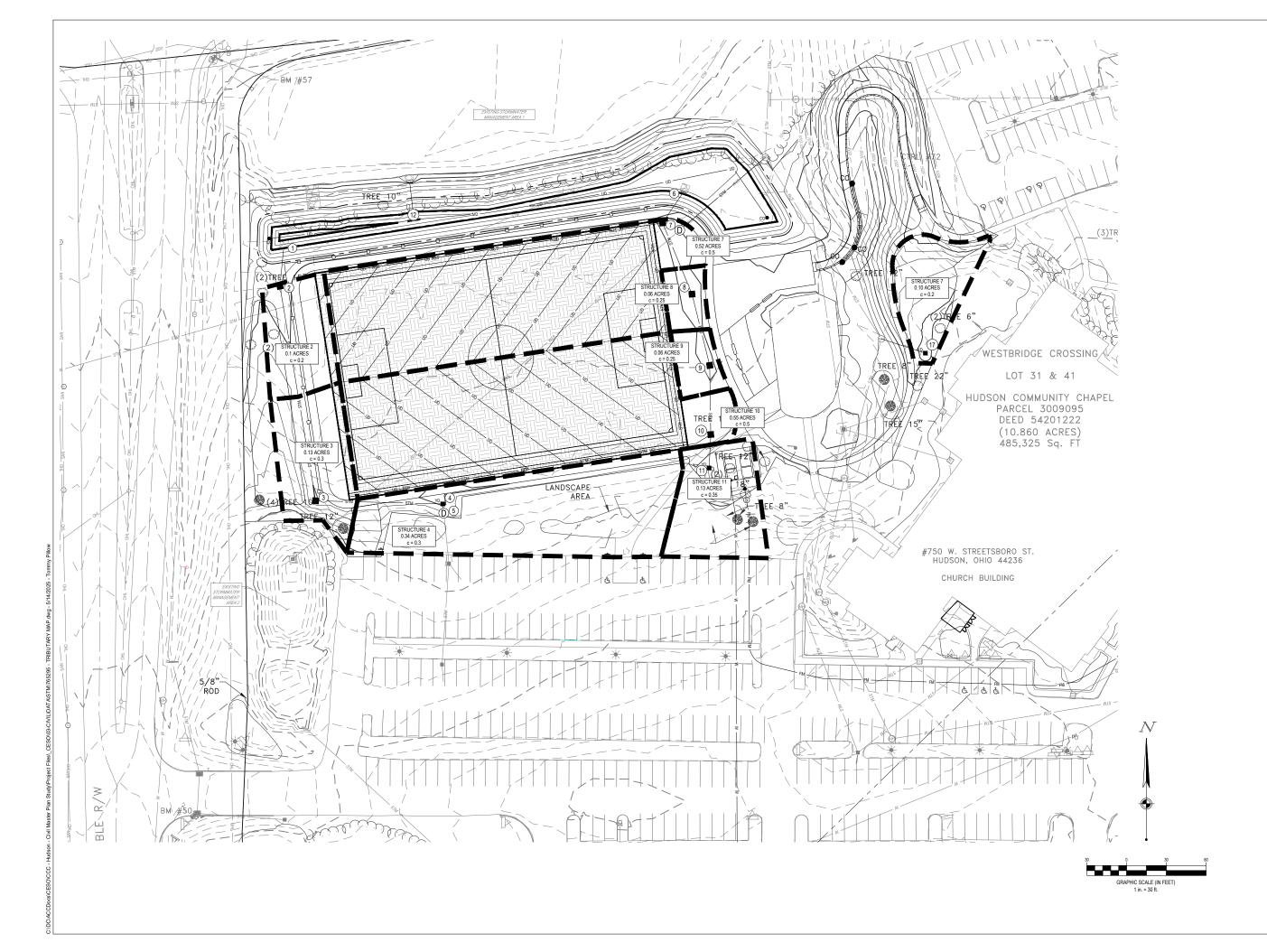


# APPENDIX E2: PROPOSED CONDITIONS DRAINAGE AREA MAP





### APPENDIX E3: TRIBUTARY DRAINAGE AREA MAP







SOL HARRIS/DAY ARCHITECTURE

CHRIST COMMUNITY
CHAPEL
750 W. STRETSBORO STREET
HUDSON, OH 44236

Project Number: 765295
Scale: AS SHOWN
Drawn By: KAN
Checked By: JMS
Date: 5/19/2025
Issue: PERMIT SET

Drawing Title:
TRIBUTARY MAP

**TRIB**