Peninsula

February 18, 2026

201 North Main Street
Hudson, Ohio 44236

To the Architectural and Historic Board of Review,

We are submitting an updated package for the repair and renovation of the building at 201 North Main Street.

The owner is attempting to prevent further deterioration of the building, both structurally and cosmetically. At this time, the final use of the building is undecided, but we will submit that information to both the City of Hudson and Summit County when the use and interior fit out plans have been completed.

The drawings have been amended to include reponses to your comments from October of 2023 (see attached letter dated 10/25/2023).

Sincerely,

Matthew Schwarz

Matthew Schwarz
Principal, COO

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NORTH MAIN RETAIL

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CONTACT: JORDAN SNEDEKER

DRAWING INDEX

ABBREVIATIONS

ABV	ABOVE
A/C	AIR CONDITIONING
AFF	ABOVE FINISHED FLOOR
ALT	ALTERNATE
AHJ	AUTHORITY HAVING JURISDICTION
ALUM	ALUMINUM
APPROX.	APPROXIMATELY
ARCH	ARCHITECTURAL
ASPH	ASPHALT
BD	BOARD
BLDG	BUILDING
BRG	BEARING
BOTT	BOTTOM
BTW	BETWEEN
CF	CUBIC FEET
CIP	CAST IN PLACE
CJ	CONTROL JOINT
CLG	CEILING
CLR	CLEAR
CMU	CONCRETE MASONRY UNIT
CONC	CONCRETE
CO	CLEAN OUT
CONT	CONTINUOUS
DBL	DOUBLE
DEPT	DEPARTMENT
DIA	DIAMETER
DIM	DIMENSION
DN	DOWN
DR	DOOR
DS	DOWNSPOUT
DTL	DETAIL
DWG	DRAWING
EA	EACH
ELEC	ELECTRICAL
EQ	EQUAL
EXH	EXHAUST
EXIST	EXISTING
EXP	EXPOSED
EXT	EXTERIOR
FD	FLOOR DRAIN
FDN	FOUNDATION
FIN.	FINISHED
FLR	FLOOR
FT	FOOT
FTG	FOOTING
FUR	FURRING
GALV	GALVANIZED
GA	GAUGE
GC	GENERAL CONTRACTOR
GYP. BD.	GYPSUM BOARD
GYP	GYPSUM

HC	HOLLOW CORE
HDWR.	HARDWARE
HM	HOLLOW METAL
HVAC	HEATING, VENTILATION, AND
AIR COND.	
HT	HEIGHT
INSUL	INSULATION
JST	JOIST
LAM	LAMINATED
LF	LINEAR FOOT
MAS	MASONRY
MATL	MATERIAL
MAX.	MAXIMUM
MECH	MECHANICAL
MFG.	MANUFACTURER
MIN.	MINIMUM
MISC	MISCELLANEOUS
MO	MASONRY OPENING
MTD	MOUNTED
MTL	METAL
NOM	NOMINAL
NTS	NOT TO SCALE
O/	OVER
O.C.	ON CENTER
OPN	OPENING
PREFAB	PREFABRICATED
PLYWD	PLYWOOD
P. LAM	PLASTIC LAMINATE
PR	PAIR
PSI	POUNDS PER SQUARE INCH
REF	REFERENCE
RM	ROOM
RO	ROUGH OPENING
REQ	REQUIRED
SC	SOLID CORE
SECT	SECTION
SIM.	SIMILAR
STRUC	STRUCTURAL
TYP	TYPICAL
UNO	UNLESS NOTED OTHERWISE
W/	WITH
WWF	WELDED WIRE FABRIC

SYMBOLS

3 / A4.00	DETAIL
1 / A2.00	EXTERIOR ELEVATION
A / A4.00 — A / A4.00	BUILDING SECTION
1, 2, 3, 4 / A5.00	INTERIOR ELEVATION
2 / A3.00	WALL SECTION
J	CENTERLINE AND GRID
3	WALL TYPE
101A	DOOR TAG
3	WINDOW TAG
3	CODED NOTE
0'-0"	SPOT ELEVATION

PROJECT INFORMATION

PARTIAL DEMOLITION AND INTERIOR RENOVATION OF AN HISTORIC TWO STORY COMMERCIAL BUILDING. THE BUILDING DATES TO EARLY 1840s AND REQUIRES A NEW FOUNDATION AND FRAMING. IN ORDER TO ACCOMMODATE THE STRUCTURAL REPAIRS WITHOUT DEMOLISHING THE BUILDING, THE HISTORIC HOUSE WILL BE LIFTED AND MOVED OFF THE FOUNDATION TO ALLOW A NEW FOUNDATION TO BE INSTALLED.

THIS DRAWING SET IS SPECIFIC TO THE EFFORT RELATED TO THE FOUNDATION AND FRAMING. A SEPARATE SET OF DRAWINGS WILL BE SUBMITTED FOR THE REMAINING ARCHITECTURAL, SITE UTILITIES, AND MEP PORTIONS OF PROJECT SCOPE.

CITY: HUDSON
COUNTY: SUMMIT

PARCEL NUMBER: 3201447

ZONING: 5-VILLAGE CORE DISTRICT

SETBACKS:
FRONT YARD: MIN. OF 75% OF FRONT WALL IS 0' TO EDGE OF SIDEWALK
REAR YARD: 10'
SIDE YARD: 0'

MAX. HEIGHT: 45'

SQUARE FOOTAGES

BASEMENT	852 SF
FIRST FLOOR FINISHED	**968 SF**
EXTERIOR COVERED AREAS	101 SF
SECOND FLOOR FINISHED	**520 SF**
TOTAL FINISHED SQUARE FOOTAGE	**1,488 SF**

MATERIALS LEGEND

DIMENSIONAL LUMBER	GRAVEL
PLYWOOD	CONCRETE
FINISH WOOD	CONCRETE BLOCK
GYPSUM BOARD	STEEL
BRICK	RIGID INSULATION
EARTH	SPRAY FOAM INSULATION
STONE VENEER	MINERAL WOOL INSULATION
BLOCKING	METAL DECK

PROJECT GENERAL NOTES

4

CONTRACTOR SHALL PROVIDE ALL MATERIALS AND WORKMANSHIP FOR ALL CONSTRUCTION REQUIRED HEREIN AND SHALL BE IN ACCORDANCE WITH THE:

COMMERCIAL BUILDING CODE: 2024 OF OHIO (IBC 2021 + AMENDMENTS)
EXISTING BUILDING CODE: 2024 OF OHIO (IEBC 2021 + AMENDMENTS)
MECHANICAL CODE: 2024 OF OHIO (IMC 2021 + AMENDMENTS)
ELECTRICAL CODE: 2023 OF OHIO (NFPA 70, 2023 + AMENDMENTS)
PLUMBING CODE: 2024 OF OHIO (IPC 2021 + AMENDMENTS)
ENERGY CONSERVATION CODE: 2021 OF OHIO (IECC 2021 + AMENDMENTS)
ACCESSIBILITY CODE: 2017 OF OHIO (A117.1.2017 + AMENDMENTS)
FUEL AND GAS CODE: 2021 OF OHIO (IFGC 2021 + AMENDMENTS)
FIRE CODE: 2017 OF OHIO

THE CONTRACTOR WILL FURNISH ALL LABOR, MATERIAL, EQUIPMENT, PERMITS, TAXES, AND INSURANCE NECESSARY TO COMPLETE THE WORK INDICATED AND/ OR IMPLIED IN THE CONSTRUCTION DOCUMENTS UNLESS NOTED OTHERWISE AND WILL COORDINATE THE WORK RESPONSIBILITIES OF ALL SUBCONTRACTORS. ALL LABOR AND MATERIALS TO CARRY OUT FULLY THE INTENTIONS OF THE PLANS AND SPECIFICATIONS ARE PART OF THE CONTRACT, WHETHER OR NOT SPECIFICALLY DOCUMENTED.

ALL WORK WILL CONFORM TO THE CURRENT OHIO BUILDING, MECHANICAL & PLUMBING CODES, AS WELL AS THE CURRENT NATIONAL BOARD OF FIRE UNDERWRITERS AND ALL OTHER APPLICABLE CITY CODES, LOCAL LAWS, AND AUTHORITIES HAVING JURISDICTION. CODE STANDARDS AND PUBLICATIONS OF PRIVATE AND PUBLIC BODIES MENTIONED WITHIN THE SPECIFICATIONS OR ON THE DRAWINGS, WILL BE CONSIDERED TO BE THOSE IN FORCE AT THE TIME OF THE CONTRACT AWARD.

THE CONTRACTOR WILL BE RESPONSIBLE FOR ALL CONTROLLED INSPECTIONS AND ANY TECHNICAL TESTING REQUIRED FOR CONTROLLED INSPECTIONS AS STIPULATED BY ALL APPLICABLE CODES.

ALL MANUFACTURED ARTICLES, MATERIALS, AND EQUIPMENT WILL BE NEW AND FREE OF DEFECTS AND WILL BE SUPPLIED, INSTALLED, CONNECTED, ERECTED, USED, CLEANED, AND CONDITIONED AS DIRECTED BY THE RESPECTIVE MANUFACTURERS, UNLESS SPECIFIED OTHERWISE.

THE CONTRACTOR WILL NOTIFY THE ARCHITECT OF ANY ERRORS, OMISSIONS, CONFLICTS, OR AMBIGUITIES IN AND BETWEEN THE DRAWINGS AND THE SPECIFICATIONS PRIOR TO PROCEEDING WITH THE WORK. IF SUCH NOTICE IS NOT FURNISHED TO THE ARCHITECT, THE CONTRACTOR WILL BE DEEMED TO HAVE INSPECTED THE DRAWINGS AND SPECIFICATIONS AND TO HAVE FOUND THEM IN PROPER FORM FOR EXECUTION.

THE CONTRACTOR REPRESENTS THAT HE HAS HAD ADEQUATE ACCESS TO THE JOB SITE AND BUILDING AREA IN WHICH THE WORK IS TO BE PERFORMED, THAT HE HAS SATISFIED HIMSELF AT TO THE NATURE AND LOCATION OF WORK, INCLUDING ANY OBSTRUCTIONS, SCOPE OF WORK, ACTUAL LEVELS, THE EQUIPMENT AND FACILITIES NEEDED PRELIMINARY TO AND DURING THE EXECUTION OF THE WORK AND ALL OTHER MATTERS, WHICH CAN IN ANY WAY AFFECT THE WORK OR THE COST THEREOF UNDER THIS CONTRACT, AND THAT HE HAS STUDIED THE CONTRACT DOCUMENTS AND ALL OTHER DOCUMENTS PERTAINING TO THE INSTALLATION OF OTHER TRADES WHICH MAY INFLUENCE HIS WORK.

THE CONTRACTOR WILL ASSUME FULL RESPONSIBILITY, INCLUDING RESPONSIBILITY FOR ALL RELATED COSTS FOR ANY AND ALL WORK DONE WITHOUT THE APPROVAL OF THE ARCHITECT IF SUCH WORK IS IN CONFLICT WITH THE CONTRACT, DRAWINGS, OR SPECIFICATIONS.

THE CONTRACTOR WILL BE RESPONSIBLE FOR THE SAFE WORKING CONDITIONS AT THE SITE. THE ARCHITECT AND OWNER WILL NOT BE DEEMED TO HAVE ANY RESPONSIBILITY OR LIABILITY IN CONNECTION HEREWITH.

CONSTRUCTION OPERATIONS WILL NOT INVOLVE INTERRUPTION OF HEATING, WATER, ELECTRICAL, OR OTHER SERVICES TO ANY PORTION OF THE BUILDING OUTSIDE THE LIMITS OF THE CONSTRUCTION SITE.

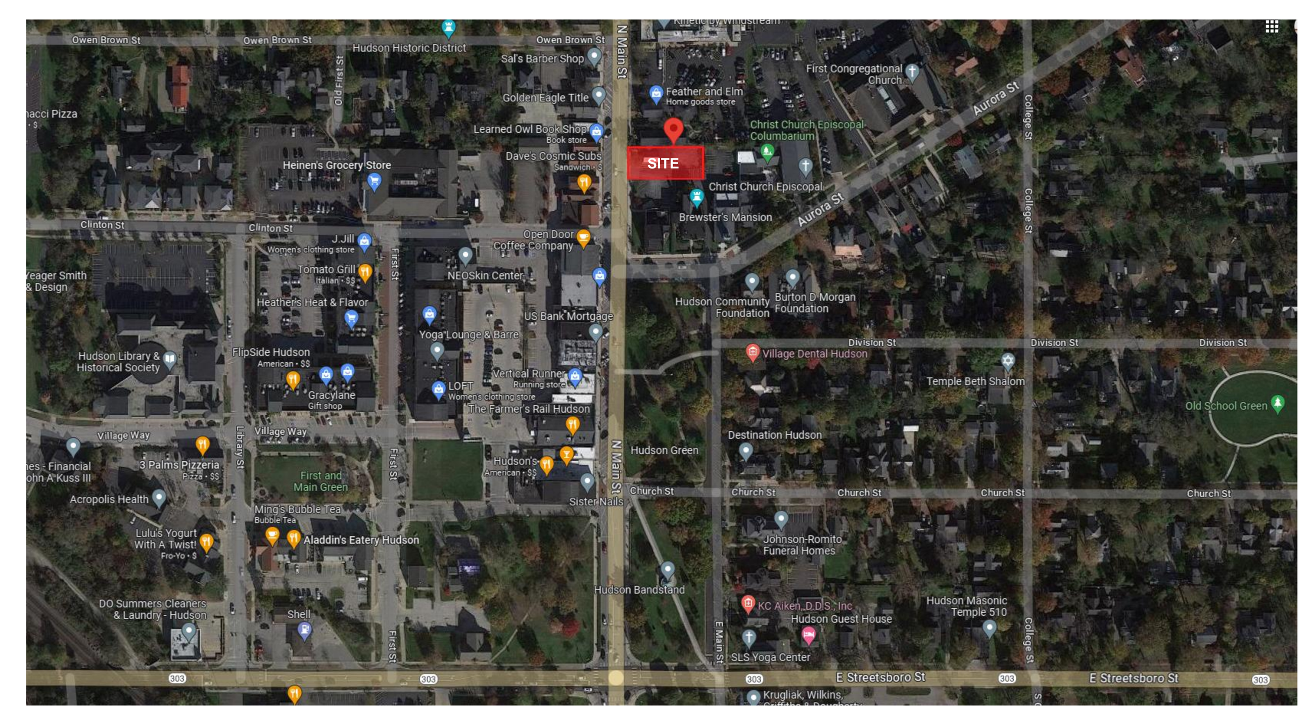
THE CONTRACTOR WILL BE RESPONSIBLE FOR CORRECTING ANY DEFICIENCIES CUASED BY DEFECTIVE OR ILL TIMED WORK AT NO ADDITIONAL COST TO THE OWNER.

NO SUBSTITUTIONS ARE PERMITTED EXCEPT WHERE THE TERM "APPROVED EQUAL" APPEARS. ALL SUBSTITUTIONS MUST BE APPROVED IN WRITING BY THE ARCHITECT. THE CONTRACTOR IS TO SUBMIT SAMPLES OR CATALOG CUTS OF ALL VISIBLE MATERIALS AND EQUIPMENT FOR THE ARCHITECT'S APPROVAL PRIOR TO INSTALLATION.

CONTRACTOR TO MAINTAIN FULL SET OF PLANS AND INSTALLATION INSTRUCTIONS ON SITE.

PERFORM VISUAL INSPECTION OF ENVELOPE AND INSULATION TO MEET 2021 IECC, 402.5.1.5 "BUILDING ENVELOPE PERFORMANCE VERIFICATION" ITEM 2

SITE LOCATION MAP



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JOSEPH MATAVA
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REGISTERED ARCHITECT

JOSEPH MATAVA
0614145 | 12/31/23

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NORTH MAIN RETAIL
201 NORTH MAIN STREET, HUDSON, OHIO

N

PROJECT #: 2317

ISSUE	ID	DATE
BID PACKAGE #1		08/23/23
BULLETIN #1	1	11/22/23
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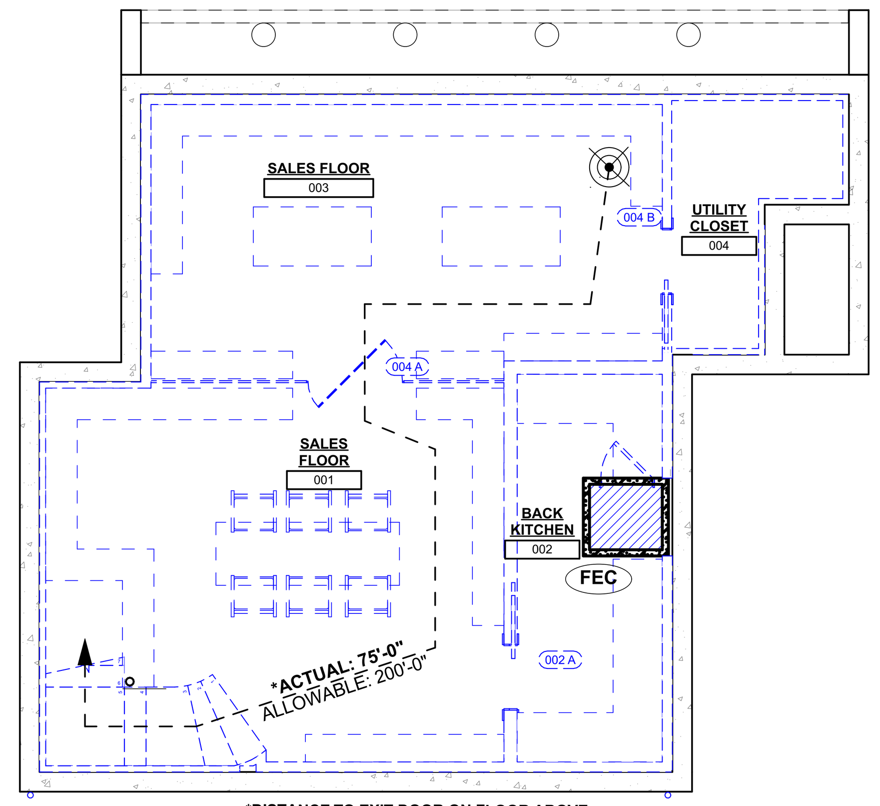
COVER SHEET

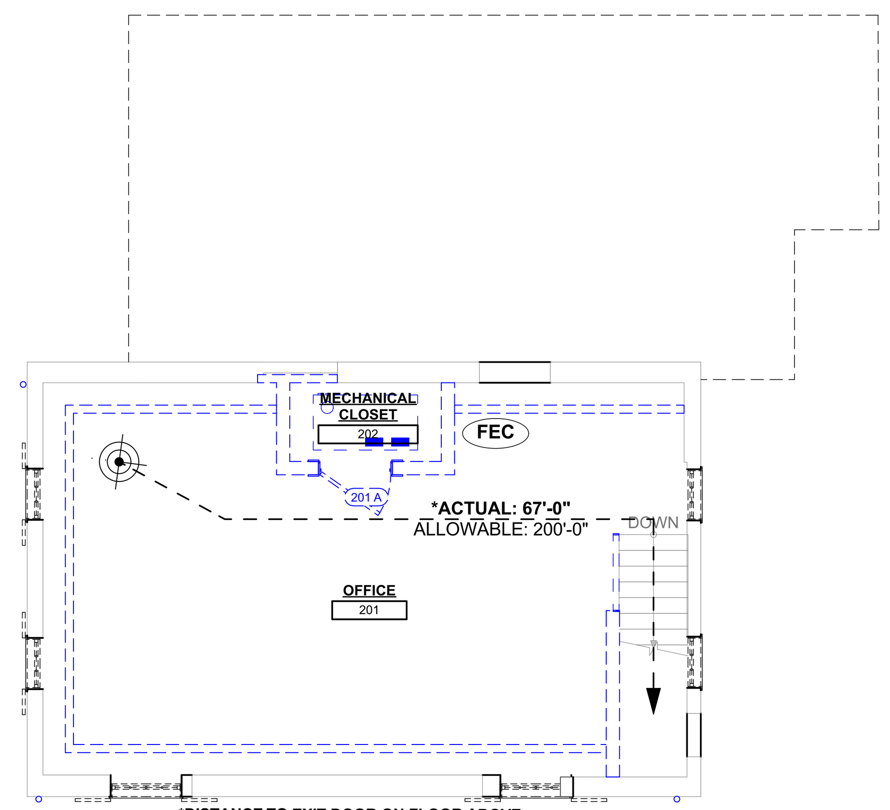
G1.00



*DISTANCE TO EXIT DOOR ON FLOOR ABOVE.

1 BASEMENT LIFE SAFETY PLAN

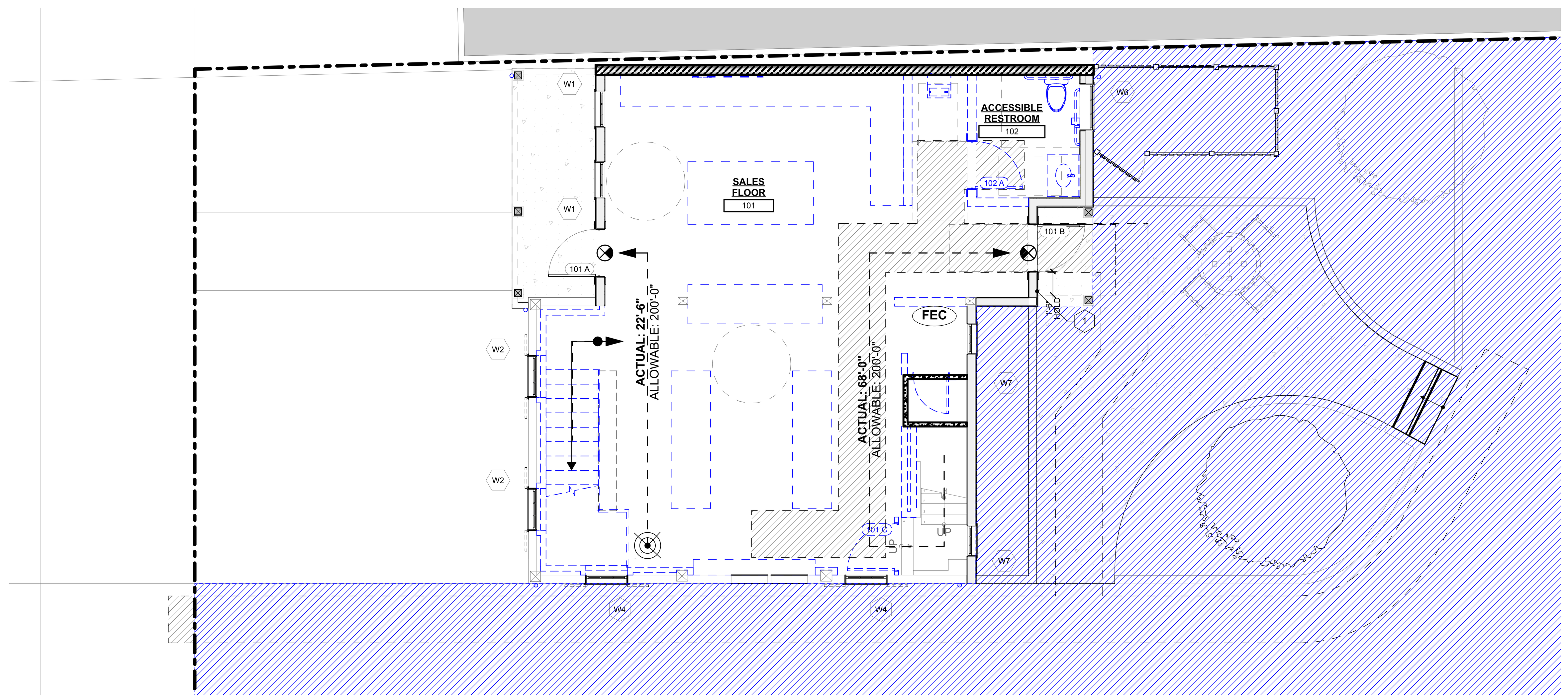
SCALE: 3/16" = 1'-0"



*DISTANCE TO EXIT DOOR ON FLOOR ABOVE.

2 SECOND FLOOR LIFE SAFETY PLAN

SCALE: 3/16" = 1'-0"



3 FIRST FLOOR LIFE SAFETY PLAN

SCALE: 3/16" = 1'-0"

CODE INFORMATION

APPLICABLE CODES

BUILDING:	2017 OHIO BUILDING CODE (IBC 2015)
MECHANICAL:	2017 OHIO MECHANICAL CODE (IMC 2015)
PLUMBING:	2017 OHIO PLUMBING CODE (IPC 2015)
ELECTRICAL:	2017 NATIONAL ELECTRICAL CODE (NFPA 70-17)
FIRE:	2017 OHIO FIRE CODE (IFC 2015)
ENERGY:	2012 INTERNATIONAL ENERGY CONSERVATION CODE (IECC-12) OR ASHRAE 90.1 2010
ACCESSIBILITY:	STANDARDS FOR ACCESSIBLE DESIGN (ICC A117.1-09 ACCESSIBLE AND USABLE BUILDINGS AND FACILITIES)
FIRE:	2017 OHIO FIRE CODE (IFC 15)
SPRINKLERS:	2016 INSTALLATION OF SPRINKLER SYSTEMS (NFPA 13-16)
FUEL GAS CODE:	2015 INTERNATIONAL FUEL GAS CODE (IFCG-15)

PROJECT SUMMARY:

OBC CHAPTER 3 | USE & CLASSIFICATION

SECTION 304 — B, BUSINESS
M, MERCANTILE

OBC CHAPTER 5 | GENERAL BUILDING HEIGHTS & AREAS

ALLOWABLE HEIGHT AND STORIES:
TABLES 504.3 AND 504.4:
B: 2 STORIES, 60'-0"

ACTUAL:
HEIGHT: 2 STORY, 30'-0"

AREA:
TABLE 506.2: (NS, NONSPRINKLERED)
B & M: 9,000 SF

ACTUAL:
TOTAL AREA: 1,488 SF

OBC CHAPTER 6 | TYPES OF CONSTRUCTION

SECTION 602 CONSTRUCTION TYPE: TYPE V-B

TYPE VB CONSTRUCTION IS THAT TYPE OF CONTRUCTION IN WHICH THE STRUCTURAL ELEMENTS, EXTERIOR WALLS, AND INTERIOR WALLS ARE OF ANY MATERIALS PERMITTED BY THIS CODE.

3 **OBC CHAPTER 7 | FIRE AND SMOKE PROTECTION FEATURES**

SECTION 705.2.3 PROJECTION PROTECTION

PROJECTIONS EXTENDING TO WITHIN 5' OF THE LINE USED TO DETERMINE THE FIRE SEPARATION DISTANCE SHALL BE FIRE-RETARDANT-TREATED WOOD.

OBC CHAPTER 9 | FIRE PROTECTION SYSTEMS

THIS BUILDING IS NOT SUPPRESSED AND NOT REQUIRED PER 903.2.7, ALL EXCEPTIONS MET.

OBC CHAPTER 10 | MEANS OF EGRESS

BASEMENT	852 SF @ 60 GSF =	14 OCCUPANTS
FIRST FLOOR	968 SF @ 60 GSF =	16 OCCUPANTS
SECOND FLOOR	520 SF @ 100 GSF =	5 OCCUPANTS
		35 OCCUPANTS

OTHER EGRESS COMPONENTS: 0.2 INCHES PER OCCUPANT

SECTION 1017.2: EXIT ACCESS TRAVEL DISTANCE FOR OCCUPANCY, NOT SUPPRESSED IS 200'-0".

ITEMS SHOWN IN BLUE DASHED LINES ARE FOR REFERENCE ONLY AND NOT TO BE REVIEWED BY PLANS EXAMINER AT THIS TIME.

LIFE SAFETY PLAN CODED NOTES

1 LOCATION OF RECESSED MOUNT KNOX BOX. REFER TO EXTERIOR ELEVATIONS FOR LOCATION INFORMATION AND MANUFACTURER SUPPLIED PRODUCT INFORMATION FOR ADDITIONAL DETAILS.

LIFE SAFETY WALL LEGEND

FIRE EXTINGUISHER: VERIFY LOCATIONS OF FIRE EXTINGUISHERS WITH LOCAL FIRE DEPARTMENT OFFICIAL AND REFER TO SPECIFICATIONS FOR TYPE INFORMATION AND ADDITIONAL DETAILS.

LIFE SAFETY PLAN SYMBOLS

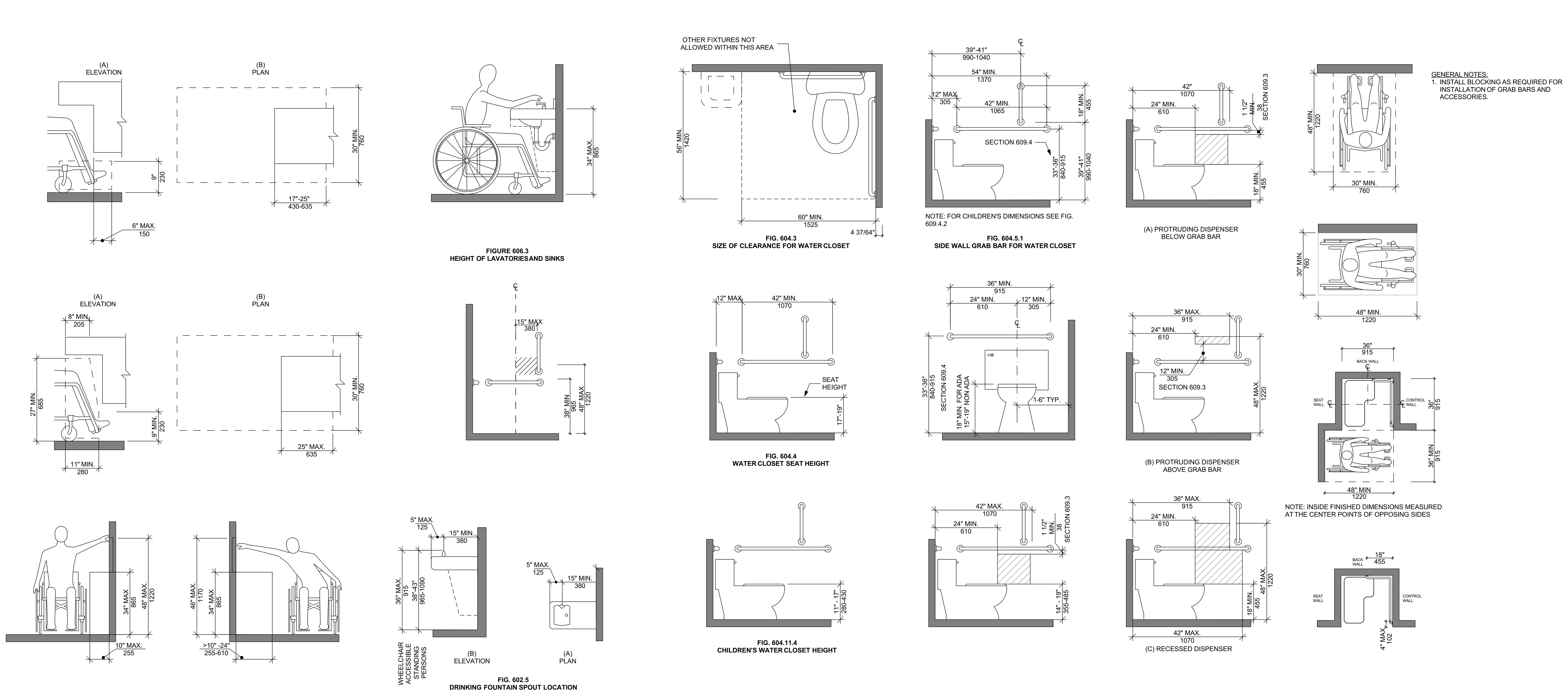
EXIT

W = ACTUAL CLEAR WIDTH OF LIMITING COMPONENT (INCHES).
X = OCCUPANTS USING EXIT
Y = EXIT CAPACITY
Z = EGRESS WIDTH PER OCCUPANT (INCHES/OCCUPANT)

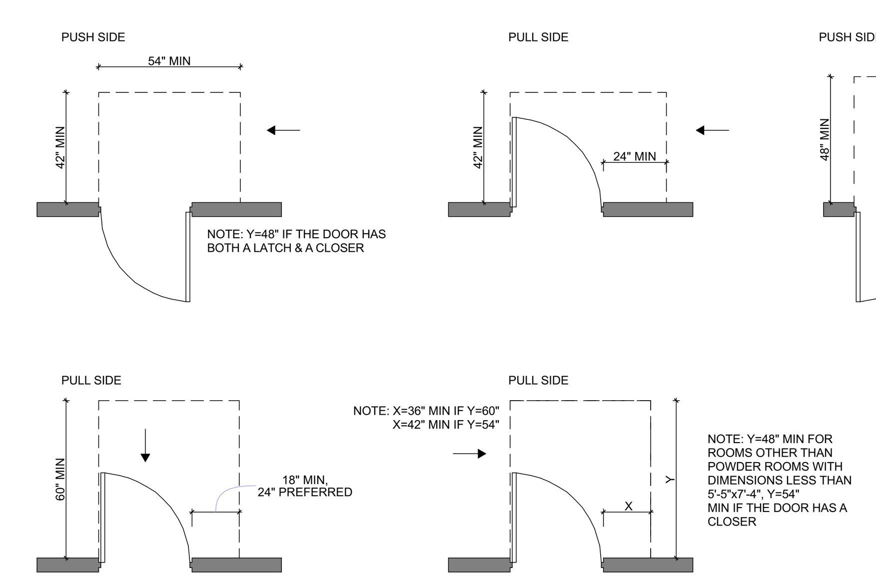
MOST REMOTE POINT, TO DETERMINE EXIT ACCES TRAVEL DISTANCE

FEC PORTABLE FIRE EXTINGUISHER CABINET
FA FIRE ALARM PULL STATION
F FIRE ALARM VISUAL/STROBE
F FIRE ALARM AUDIO AND VISUAL
FAP FIRE ALARM REMOTE ANNUNCIATOR
HATCH INDICATES ACCESSIBLE PATH OF TRAVEL
HATCH INDICATES FOR REFERENCE ONLY
HATCH INDICATES 1 HOUR RATED WALL
HATCH INDICATES 2 HOUR RATED WALL

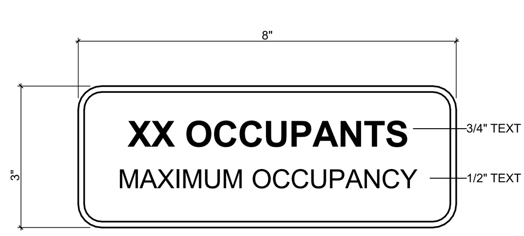
TYPICAL ADA DETAILS (ICC A117.1-2009)



MANEUVERING CLEARANCES AT MANUAL SWING DOORS (ADA 404.2.4.1)



OCCUPANCY SIGNAGE



NOTE: 'XX' TO BE REPLACED WITH INDICATED OCCUPANCY CAPACITY PER OCCUPANCY SCHEDULE ON G1.01. WHERE OCCUPANCY NUMBERS ARE TO BE POSTED IN ROOMS ADJACENT AND OPEN TO ANOTHER ROOM, COMBINED OCCUPANCY FOR CONNECTED SPACES MAY BE POSTED AT ALL REQUIRED LOCATIONS. INDICATED NUMBER NOT TO EXCEED SUM OF INIDVIDUAL ROOMS INDICATED PER OCCUPANCY SCHEDULE.

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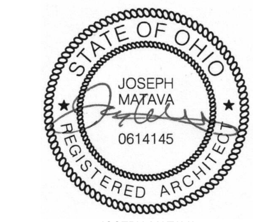

JOSEPH MATAVA
0614145 | 12/31/23

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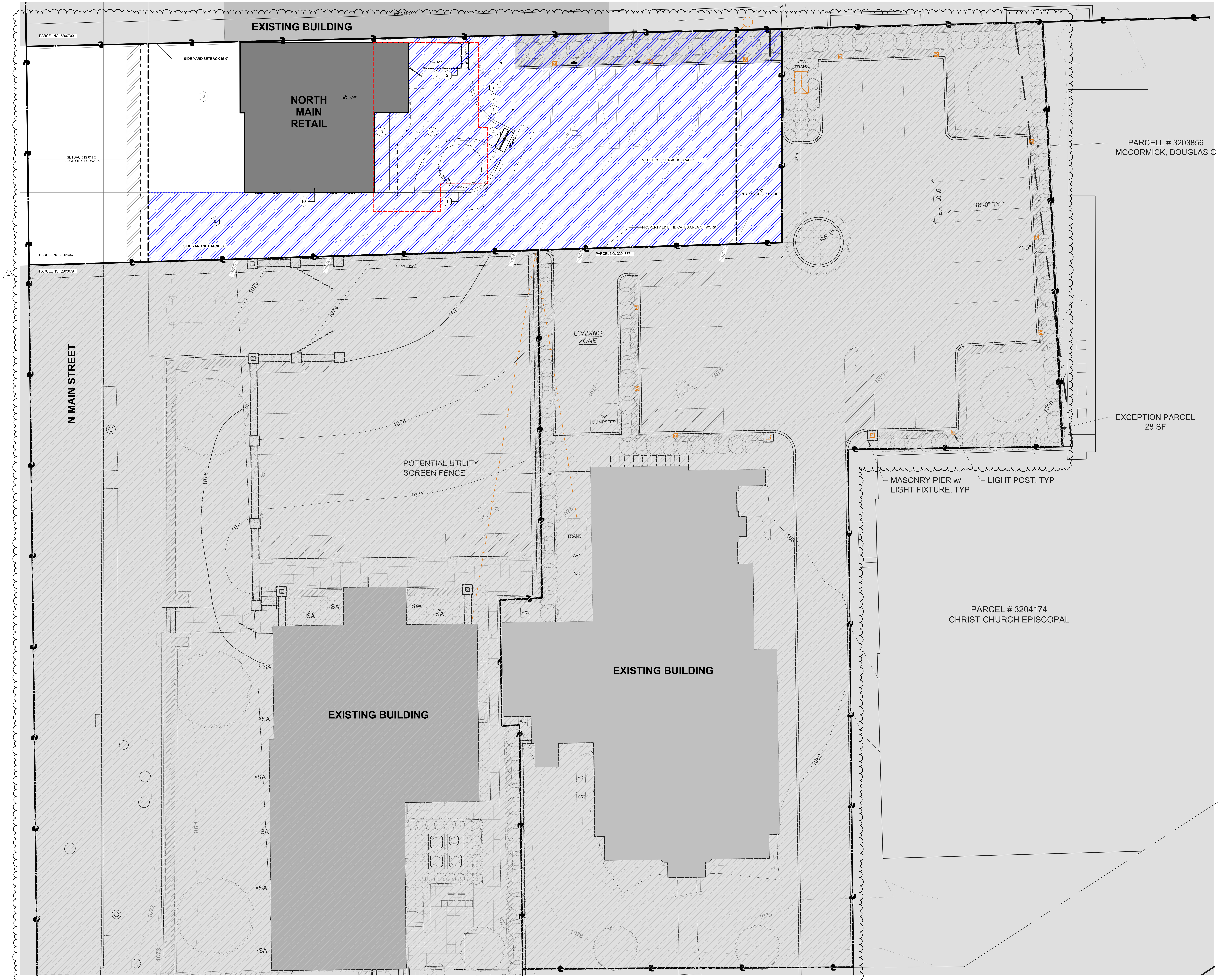
LIFE SAFETY PLAN & NOTES

G1.01



1 SITE PLAN
SCALE: 1/8" = 1'-0"

N

GENERAL NOTES

REFER TO CIVIL DRAWINGS FOR ADDITIONAL SITE INFORMATION, SUCH AS STREET GRADES, UTILITY LOCATIONS, AND FINISH GRADES.

REQUIRED PARKING SPACES: 6 SPACES
(RETAIL BUSINESS - 1 SPACE FOR EACH 250 SF)

PROPOSED PARKING SPACES:
ACCESSIBLE SPACES - 2 SPACES
TOTAL OFF STREET PARKING SPACES - 5 SPACES

1207.12 IN ZONE DISTICT 5, VILLAGE CORE, ANY ON-STREET PARKING LOCATED WITHIN 300 FT OF THE SBJECT SITE THAT CAN BE ACCESSED BY SIDEWALKS AND CROSS WALKS WITHIN THAT DISTANCE SHALL BE COUNTED TOWARD THE OFF-STREET PARKING REQUIREMENT OF THE SUBJECT USE.

SITE PLAN KEYNOTES

1 DRY LAID SANDSTONE RETAINING WALL
2 PRIVACY FENCE FOR HVAC CONDESNER, GENERATOR, AND TRASH
3 CONCRETE PATIO
4 STONE STAIR TREAD
5 LANDSCAPE BED
6 CRUSHED GRAVEL
7 EXISTING TREE TO REMAIN
8 EXISTING CONCRETE WALK TO REMAIN
9 EXISTING DRIVEWAY TO BE DEMOLISHED AND REPAVED
10 EXISTING HISTORIC BUILDING TO BE TEMPORARILY RELOCATED FOR EXCAVATION AND INSTALLATION OF NEW FOUNDATION AND FRAMING. BUILDING IS TO BE REINSTALLED IN EXACT LOCATION OF HISTORIC BUILDING, VERIFY FOOTPRINT IN FIELD

SITE PLAN LEGEND

HATCH INDICATES AREA NOT INCLUDED IN PROJECT SCOPE
PROPERTY LINE
SETBACK LINE
FOOT PRINT OF EXISTING BUILDING TO BE DEMOLISHED
HATCH INDICATES ACCESSIBLITY ROUTE
NEW ACCESSIBLE PARKING SPACE POLE MOUNTED SIGN
HATCH INDICATES FUTURE DESIGN AND FOR REFERENCE ONLY
4
HATCH INDICATES OTHER PARCELS OWNED BY OWNER AND SERVES AS FUTURE MASTER PLAN. FOR REFERENCE ONLY.

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STATE OF OHIO JOSEPH MATAVA 0614145 REGISTERED ARCHITECT
JOSEPH MATAVA
0614145 | 12/31/23

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ARCHITECTURAL SITE PLAN

A1.00

OHIO Utilities Protection SERVICE
811 or 1-800-362-2764 Call Before You Dig

GENERAL STRUCTURAL NOTES (GSN)

STRUCTURAL DESIGN CRITERIA

CODE:	OHIO BLDG CODE, ADOPTED NOVEMBER 1, 2017 (REFERENCE 2015 IBC & ASCE 7-10)
BUILDING CONSTRUCTION TYPE:	
USAGE:	
LIVE LOAD DATA:	
ROOF LIVE LOAD:	20 PSF MIN.
FLOOR LIVE LOAD:	100 PSF
SECOND FLOOR OFFICE	50 PSF
STAIR LOADS:	100 PSF
GUARDRAIL LOADS:	50 PLF ANY DIRECTION 200 LBS ANY DIRECTION NOT APPLIED SIMULTANEOUSLY
ROOF SNOW LOAD DATA:	
FLAT-ROOF SNOW LOAD, P_f:	20 PSF
SNOW EXPOSURE FACTOR, C_e:	0.9
SNOW IMPORTANCE FACTOR, I_s:	1.0
THERMAL FACTOR, C_t:	1.0
WIND DESIGN DATA:	
ULTIMATE WIND SPEED, V_{ult}:	115 MPH
DESIGN WIND VELOCITY, V_{asd}:	90 mph
RISK CATEGORY:	II
EXPOSURE:	B
INTERNAL PRESSURE COEFFICIENT, GC_{pi}:	+/- 0.18
DESIGN PRESSURE:	20 PSF
EARTHQUAKE DESIGN DATA:	
RISK CATEGORY:	II
SEISMIC IMPORTANCE FACTOR, I_e:	1.0
SPECTRAL RESPONSE ACCELERATION PARAMETERS:	S_s = 0.13 S_1 = 0.052
SITE CLASS:	D
DESIGN SPECTRAL RESPONSE ACCELERATION PARAMETERS:	S_{DS} = 0.11 S_{D1} = 0.074
SEISMIC DESIGN CATEGORY:	B
BASIC SEISMIC FORCE-RESISTING DESIGN BASE SHEAR:	0.8 KIPS
RESPONSE MODIFICATION COEFFICIENT, C_s:	0.213
RESPONSE MODIFICATION COEFFICIENT, R:	6.5
ANALYSIS PROCEDURE:	EQUIVALENT LATERAL FORCE
DESIGN BASIS:	ALLOWABLE STRESS DESIGN (ASD) FOR ALL MEMBERS EXCEPT CONCRETE ULTIMATE STRENGTH DESIGN (USD) FOR REINFORCED CONCRETE MEMBERS

GENERAL

1. THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS PRIOR TO STARTING CONSTRUCTION. THE ENGINEER SHALL BE NOTIFIED OF ANY DISCREPANCIES OR INCONSISTENCIES. IN CASE OF CONFLICT, MORE COSTLY REQUIREMENTS GOVERN FOR BIDDING. SUBMIT CLARIFICATION REQUEST PRIOR TO PROCEEDING WITH WORK.
2. ALL DRAWINGS ARE CONSIDERED TO BE A PART OF THE CONTRACT DOCUMENTS. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE REVIEW AND COORDINATION OF ALL DRAWINGS AND SPECIFICATIONS PRIOR TO THE START OF CONSTRUCTION. ANY DISCREPANCIES THAT OCCUR SHALL BE BROUGHT TO THE ATTENTION OF THE ENGINEER PRIOR TO START OF CONSTRUCTION SO THAT A CLARIFICATION CAN BE ISSUED. ANY WORK PERFORMED IN CONFLICT WITH THE CONTRACT DOCUMENTS OR ANY CODE REQUIREMENTS SHALL BE CORRECTED BY THE CONTRACTOR AT HIS OWN EXPENSE.
3. NOTES AND DETAILS ON DRAWINGS SHALL TAKE PRECEDENCE OVER GENERAL NOTES AND TYPICAL DETAILS. WHERE NO DETAILS ARE GIVEN, CONSTRUCTION SHALL BE AS SHOWN FOR SIMILAR WORK. UNLESS NOTED OTHERWISE, DETAILS IN STRUCTURAL DRAWINGS ARE TYPICAL AS INDICATED BY CUTS, REFERENCES, OR TITLES.
4. ALL WORK SHALL CONFORM TO THE MINIMUM STANDARDS OF THE FOLLOWING CODES: OHIO BLDG CODE AND LATEST REVISIONS REFERRED TO HERE AS "THE CODE", AND ANY OTHER REGULATING AGENCIES WHICH HAVE AUTHORITY OVER ANY PORTION OF THE WORK.
5. SEE ARCHITECTURAL DRAWINGS FOR THE FOLLOWING: SIZE AND LOCATION OF ALL DOOR AND WINDOW OPENINGS, EXCEPT AS NOTED. SIZE AND LOCATION OF ALL INTERIOR AND EXTERIOR NON-BEARING PARTITIONS. SIZE AND LOCATION OF ALL CONCRETE CURBS, EQUIPMENT PADS, PITS, FLOOR DRAINS, SLOPES, DEPRESSED AREAS, CHANGE IN LEVEL, CHAMFERS, GROOVES, INSERTS, ETC. SIZE AND LOCATION OF ALL FLOOR AND ROOF OPENINGS EXCEPT AS SHOWN. FLOOR AND ROOF FINISHES. DIMENSIONS NOT SHOWN ON STRUCTURAL DRAWINGS.
6. SEE MECHANICAL, PLUMBING, AND ELECTRICAL DRAWINGS FOR THE FOLLOWING: PIPE RUNS, SLEEVES, HANGERS, TRENCHES, WALL AND SLAB OPENINGS, ETC., EXCEPT AS SHOWN OR NOTED. ELECTRICAL CONDUIT RUNS, BOXES, OUTLETS IN WALLS AND SLABS. CONCRETE INSERTS FOR ELECTRICAL, MECHANICAL, OR PLUMBING FIXTURES. SIZE AND LOCATION OF MACHINE OR EQUIPMENT BASES, ANCHOR BOLTS FOR MOTOR MOUNTS.
7. THE CONTRACT STRUCTURAL DRAWINGS AND SPECIFICATIONS REPRESENT THE FINISHED STRUCTURE. THEY DO NOT INDICATE THE METHOD OF CONSTRUCTION. THE CONTRACTOR SHALL PROVIDE ALL MEASURES NECESSARY TO PROTECT THE STRUCTURE DURING CONSTRUCTION. SUCH MEASURES SHALL INCLUDE BUT NOT BE LIMITED TO BRACING, SHORING FOR LOADS DUE TO CONSTRUCTION EQUIPMENT, ETC. OBSERVATION VISITS TO THE SITE BY THE STRUCTURAL ENGINEER SHALL NOT INCLUDE INSPECTION OF THE ABOVE ITEMS.
8. ASTM SPECIFICATIONS ON THE DRAWINGS SHALL BE OF THE LATEST REVISION.
9. CONTRACTOR SHALL INVESTIGATE SITE DURING CLEARING AND EARTHWORK OPERATIONS FOR FILLED EXCAVATIONS OR BURIED STRUCTURES, SUCH AS CESSPOOLS, CISTERNS, FOUNDATIONS, ETC. IF ANY SUCH STRUCTURES ARE FOUND, ENGINEER SHALL BE NOTIFIED IMMEDIATELY.
10. CONSTRUCTION MATERIAL SHALL BE SPREAD OUT IF PLACED ON FRAMED ROOF. LOAD SHALL NOT EXCEED THE DESIGN LIVE LOAD PER SQUARE FOOT. PROVIDE ADEQUATE SHORING AND/OR BRACING WHERE STRUCTURE HAS NOT ATTAINED DESIGN STRENGTH.
11. UNLESS NOTED OTHERWISE, EXPANSION BOLTS IN CONCRETE SHALL BE 1/2" DIAMETER X 3 1/2" EMBEDMENT HILTI KWIK BOLTS III (ICBO #4627) OR APPROVED ALTERNATE WITH ALLOWABLE VALUES EQUAL TO OR EXCEEDING THOSE FOR HILTI, PER CURRENT ICBO RESEARCH REPORT. UNLESS NOTED OTHERWISE, ALL EPOXY ANCHORS SHALL BE 1/2" DIAMETER WITH 4 1/4" EMBEDMENT HILTI HIT SYSTEM (ICBO 4016) OR APPROVED ALTERNATE WITH ALLOWABLE VALUES EQUAL TO OR EXCEEDING THOSE FOR HILTI, PER CURRENT ICBO RESEARCH REPORT. INSTALL EXPANSION AND EPOXY ANCHORS PER MANUFACTURER'S RECOMMENDATIONS.
12. GROUT OTHER THAN FOR MASONRY CELLS SHALL BE NON-SHRINK, NON-METALLIC, MEETING ASTM C-827, C-191, AND C-109. MIXED AND INSTALLED PER MANUFACTURER'S SPECIFICATIONS. MINIMUM COMPRESSIVE STRENGTH 5,000 PSI IN TWO DAYS.

SPECIAL INSPECTION

THE FOLLOWING ELEMENTS OF CONSTRUCTION SHALL REQUIRE SPECIAL INSPECTION PER OBC SECTION 1701. BUILDING OWNER TO FURNISH INSPECTION UNLESS INSTRUCTED OTHERWISE BY THE CONSTRUCTION CONTRACT.

Description	Design Value
Soils	
Soil Bearing Capacity (Presumptive Table 1806.2)	1,500 PSF
Helical Pile Capacity (Underpinning)	20 KIPS
Concrete	
Compressive strength at 7 and 28 days	ASTM C-31 and C-39
Footings	3000 PSI
Slabs on Grade	4000 PSI
Concrete Density, ASTM C-138	145 PCF normal
Concrete Slump (1 per 50 CY or fraction)	4" max ASTM C-143
Percentage Entrained Air for exterior concrete	6% ± 1 ASTM C-231
Verify reinforcing sizes and placement	ASTM A-615, Grade 60
Wood Framing	
Verify lumber grades	Visual
Shear Wall	Visual
Nailing Patterns	Visual
Light Gage Metal Connectors	Visual

a. THE CONSTRUCTION INSPECTIONS LISTED ARE IN ADDITION TO THE CALLED INSPECTIONS REQUIRED BY OBC, AS AMENDED. SPECIAL INSPECTION IS NOT A SUBSTITUTE FOR INSPECTION BY LOCAL BUILDING DEPARTMENT. SPECIALLY INSPECTED WORK WHICH IS INSTALLED OR COVERED WITHOUT THE APPROVAL OF THE LOCAL BUILDING INSPECTOR IS SUBJECT TO REMOVAL OR EXPOSURE AT CONTRACTOR'S EXPENSE.
b. CONTINUOUS INSPECTION IS ALWAYS REQUIRED DURING THE PERFORMANCE OF THE WORK UNLESS OTHERWISE SPECIFIED.
c. THE SPECIAL INSPECTORS MUST BE CERTIFIED BY THE LOCAL BUILDING DEPARTMENT TO PERFORM THE TYPES OF INSPECTION SPECIFIED.
d. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO INFORM THE SPECIAL INSPECTOR OR INSPECTION AGENCY AT LEAST ONE WORKING DAY PRIOR TO PERFORMING ANY WORK THAT REQUIRES SPECIAL INSPECTION. ALL WORK PERFORMED WITHOUT REQUIRED SPECIAL INSPECTION IS SUBJECT TO REMOVAL.
e. SUBMIT WRITTEN REPORTS WITHIN TWO DAYS OF TESTING TO ENGINEER OF RECORD.
f. G.C. TO NOTIFY BUILDING DEPT. OF SPECIAL INSPECTORS NAME AND CONTACT INFORMATION.

CONSTRUCTION SUBMITTALS

1. THE STRUCTURAL SUBMITTAL REVIEW IS INTENDED TO HELP THE ENGINEER VERIFY HIS DESIGN CONCEPT. IT IS THE CONTRACTOR'S RESPONSIBILITY TO CHECK HIS OWN SUBMITTALS. THE ENGINEER WILL REVIEW THE SUBMITTALS FOR CONFORMANCE WITH CONSTRUCTION DOCUMENTS, GENERAL DIMENSIONS, MEMBERS, ELEVATIONS AND CONNECTIONS. THE CONTRACTOR IS SOLELY RESPONSIBLE FOR ALL DIMENSIONS IN SUBMITTALS AND COORDINATION WITH OTHER TRADES.
2. SHOP DRAWINGS ARE THE CONTRACTOR'S AND FABRICATOR'S WORK PRODUCT. THE CONTRACTOR AND FABRICATOR ARE SOLELY RESPONSIBLE FOR ANY ERRORS IN THEIR SHOP DRAWINGS. THE ENGINEER IS NOT ENGAGED TO PERFORM DETAIL CHECKING OF THE SHOP DRAWINGS NOR WILL BE RESPONSIBLE FOR ANY ERRORS IN OR MISSING MATERIALS FROM THE SHOP DRAWINGS.
3. FOR PRINT COPIES CONTRACTOR IS TO SUBMIT ONLY 3 SETS OF SHOP DRAWINGS TO ENGINEER FOR REVIEW. ANY ADDITIONAL SETS WILL BE RETURNED UNMARKED. FOR ELECTRONIC SUBMITTALS, CONTRACTOR WILL BE RESPONSIBLE FOR PRINTING CHARGES FOR ONE SET OF EACH SUBMITTAL. CORRECTIONS WILL BE RETURNED ELECTRONICALLY.
4. ALL SUBMITTALS ARE TO BE REVIEWED AND APPROVED BY THE GENERAL CONTRACTOR AND CHECKED BY THE FABRICATOR OR VENDOR PRIOR TO SUBMITTAL FOR REVIEW BY ENGINEER.
5. THE STRUCTURAL SUBMITTALS WILL BE RETURNED FOR RESUBMITTAL IF A CURSORY REVIEW SHOWS MAJOR ERRORS WHICH SHOULD HAVE BEEN FOUND BY THE GENERAL CONTRACTOR'S CHECKING.
THE FOLLOWING SUBMITTALS, WHEN APPLICABLE, ARE REQUIRED FOR SUBMITTAL FOR STRUCTURAL REVIEW:

ITEM	PROD. DATA	DRAWINGS	CALC'S	PE SEAL
a. SPLICED REINFORCING		X		
b. CONCRETE MIX DESIGNS	X			
c. STRUCTURAL STEEL		X		
d. WOOD TRUSSES	X	X	X	X
e. HELICAL PILES	X		X	X

6. ANY SUBMITTAL OF A DETAIL SHEET WITH ADDED INFORMATION SHALL BE ACCOMPANIED BY LOCATION PLAN IDENTIFYING THE MEMBERS INVOLVED AND CLOUDING AROUND ADDED INFORMATION.
7. ANY ENGINEERING DESIGN PROVIDED BY OTHERS AND SUBMITTED FOR REVIEW SHALL BE BY A REGISTERED STRUCTURAL ENGINEER, AND SUBMITTAL SHALL BE SEALED BY THE ENGINEER. SAID ENGINEER MUST BE REGISTERED WITH THE STATE THE PROJECT IS LOCATED WITHIN.
8. THE CONTRACT DOCUMENTS MAY NOT BE USED BY THE DETAILER AS USE IN ERECTION OR DETAIL DRAWINGS WITH OUT PRIOR WRITTEN PERMISSION FROM THE STRUCTURAL ENGINEER.
9. SUBMITTALS ARE TO BE RECEIVED BY THE STRUCTURAL ENGINEER A MINIMUM OF 10 WORKING DAYS PRIOR TO CONSTRUCTION SCHEDULING. STRUCTURAL ENGINEER IS NOT RESPONSIBLE FOR CONSTRUCTION DELAYS DUE TO INADEQUATE SCHEDULING OF SUBMITTAL REVIEW. SUBMITTALS TO BE SUBMITTED TO ARCHITECT PRIOR TO ENGINEERS REVIEW.
10. ANY ALTERNATE PRODUCTS ARE TO BE SUBMITTED IN ADVANCE OF PRODUCT'S INSTALLATION FOR APPROVAL BY ENGINEER OF RECORD. PRODUCT MUST EQUAL OR EXCEED SPECIFICATIONS AND QUALITY OF PRODUCTS SPECIFIED BY ENGINEER OF RECORD. ENGINEER OF RECORD RESERVES THE RIGHT TO ACCEPT OR REJECT ANY PRODUCT SUBSTITUTION WITHOUT CAUSE.

FOUNDATION

1. FOOTINGS ARE DESIGNED BASED ON THE FOLLOWING INFORMATION: PRESUMPTIVE BEARING CAPACITY PER OBC TABLE 1806.2 = 1500 PSF FOOTINGS SHALL BEAR ON COMPACTED FILL OR NATIVE SOILS TESTED.
2. CONTRACTOR TO PROVIDE FOR DE-WATERING OF EXCAVATIONS FROM EITHER SURFACE WATER, GROUND WATER, OR SEEPAGE, IF REQUIRED.
3. CONTRACTOR SHALL PROVIDE FOR DESIGN AND INSTALLATION OF ALL CRIBBING, SHEATHING, AND SHORING REQUIRED AND SHALL BE SOLELY RESPONSIBLE FOR ALL EXCAVATION PROCEDURES INCLUDING LAGGING, SHORING, AND PROTECTION OF ADJACENT PROPERTY, STRUCTURES, STREETS, AND UTILITIES IN ACCORDANCE WITH ALL NATIONAL, STATE, AND LOCAL SAFETY ORDINANCES.
4. EXCAVATION FOR FOOTINGS SHALL BE APPROVED BY THE INSPECTOR OR SOILS ENGINEER PRIOR TO PLACING THE CONCRETE AND REINFORCING. CONTRACTOR TO NOTIFY THE INSPECTOR WHEN INSPECTION OF EXCAVATION IS READY. INSPECTOR TO SUBMIT A LETTER OF COMPLIANCE.
5. ALL EXCAVATIONS SHALL BE PROPERLY BACKFILLED. DO NOT PLACE BACKFILL BEHIND RETAINING WALLS BEFORE CONCRETE OR GROUT HAS ATTAINED FULL DESIGN STRENGTH.
6. FOUNDATIONS SHALL BE PLACED AND ESTIMATED ACCORDING TO DEPTHS SHOWN ON DRAWINGS. SHOULD SOIL ENCOUNTERED AT THESE DEPTHS NOT BE APPROVED BY THE INSPECTOR OR SOILS ENGINEER, FOUNDATION ELEVATIONS WILL BE ALTERED BY CHANGE ORDER.
7. FOOTING BACKFILL AND UTILITY TRENCH BACKFILL WITHIN BUILDING AREA SHALL BE MECHANICALLY COMPACTED IN LAYERS IN ACCORDANCE WITH THE SOILS REPORT AND APPROVED BY THE INSPECTOR. ALL FILLS USED TO SUPPORT FOUNDATIONS SHALL BE INSPECTED BY THE SOILS ENGINEER REPRESENTATIVE PER CODE SECTION 1704.
8. ALL ABANDONED FOOTINGS, UTILITIES, ETC. WHICH INTERFERE WITH NEW CONSTRUCTION SHALL BE REMOVED. NEW FOOTINGS SHALL EXTEND INTO UNDISTURBED SOILS.
9. SLABS ON GRADE SHALL BE SUPPORTED ON NATURAL GRADE OR COMPACTED FILL AS PER THE RECOMMENDATIONS OF THE SOILS REPORT. PROOF ROLL PRIOR TO PLACING BASE. REPLACE SOFT AREAS WITH COMPACTED FILL.
10. PLACE FILLS TO BE COMPACTED IN MAX 8" LOOSE LIFTS. COMPACT TO MINIMUM 98% OF MAXIMUM DENSITY AT +/-2% OPTIMUM MOISTURE WHEN TESTED IN ACCORDANCE WITH ASTM D-698.
11. COMPACT UNDERSLAB GRANULAR FILL TO 98% OF MAXIMUM DENSITY AT OPTIMUM MOISTURE CONTENT IN ACCORDANCE WITH ASTM D-698.
12. DO NOT BACKFILL AGAINST BASEMENT WALLS UNTIL FLOOR STRUCTURE IS COMPLETE OR WALL IS ADEQUATELY BRACED. USE STRUCTURAL PIPE BRACING. CONTRACTOR IS RESPONSIBLE FOR THE DESIGN OF BRACING.

CONCRETE

1. ALL CONCRETE CONSTRUCTION SHALL CONFORM WITH CHAPTER 19 OF THE CODE AND WITH THE PROVISIONS OF ACI 318, LATEST EDITION.
2. REINFORCED CONCRETE IS DESIGNED BY THE "ULTIMATE STRENGTH DESIGN METHOD".
3. CONCRETE MIXES SHALL BE DESIGNED BY A QUALIFIED TESTING LABORATORY AND APPROVED BY THE STRUCTURAL ENGINEER. MIX DESIGN METHODS (TEST HISTORY OR TRIAL BATCH METHOD) PER ACI SECTION 5.3 SHALL BE USED TO PROPORTION CONCRETE. SUBMIT MIX DESIGN METHOD DATA. IF 3-POINT CURVES ARE USED, GC TO CLEARLY IDENTIFY WHICH POINT ON CURVE IS USED AND MIX DESIGN ON 3-POINT CURVE.
4. SCHEDULE OF STRUCTURAL CONCRETE 28-DAY STRENGTH AND TYPES (SLUMP LISTED IS MAX):

LOCATION IN STRUCTURE	STRENGTH	W/C RATIO	SLUMP	DENSITY
• FOUNDATIONS	3000 PSI	0.55	4"	145 PCF
• FOUDNATION WALLS	4000 PSI	0.50	4"	145 PCF
• SLABS ON GRADE	4000 PSI	0.45	4"	145 PCF

CONTRACTOR AT HIS OPTION MAY INCREASE SLUMP WITH USE OF HRWR ADMIXTURE. LIMIT SLUMP INCREASE TO 2" GREATER THAN THAT ALLOWED WITHOUT HRWRA.
5. PORTLAND CEMENT SHALL CONFORM TO ASTM C-150, TYPE I OR II.
6. AGGREGATE FOR CONCRETE SHALL CONFORM TO ALL REQUIREMENTS AND TESTS OF ASTM C-33 AND PROJECT SPECIFICATIONS.
7. CONCRETE MIXING OPERATION, ETC. SHALL CONFORM TO ASTM C-94.
8. PLACEMENT OF CONCRETE SHALL CONFORM TO ACI CODE CHAPTER 5 AND PROJECT SPECIFICATIONS.
9. CLEAN AND ROUGHEN TO 1/4" AMPLITUDE ALL CONCRETE SURFACES AGAINST WHICH NEW CONCRETE IS TO BE PLACED.
10. ALL REINFORCING BARS, ANCHOR BOLTS, AND OTHER CONCRETE INSERTS SHALL BE WELL SECURED IN POSITION PRIOR TO PLACING CONCRETE.
11. CUT JOINTS FOR SLABS ON GRADE A MAXIMUM OF 12'-0" O.C., UNLESS NOTED OTHERWISE ON THE CONTRACT DOCUMENTS. CUT JOINTS WITHIN 8 (EIGHT) HOURS AFTER PLACING CONCRETE.
12. CONCRETE EXPOSED TO THE WEATHER, FREEZE-THAW, DEICING CHEMICALS, AND OR PARKED VEHICLES SHALL CONTAIN 6% (+/-1%) ENTRAINED AIR EITHER BY USING TYPE "A" PORTLAND CEMENTS OR ADMIXTURES CONFORMING TO ASTM C-260.
13. CURE CONCRETE BY WET CURING OR LIQUID SPRAY CONFORMING TO ASTM C-309. CONTRACTOR TO VERIFY CURING AGENT IS COMPATIBLE WITH ANY FLOOR ADHESIVES SPECIFIED WITHIN THE CONTRACT DOCUMENTS.
14. ALL ADMIXTURES SHALL BE COMPATIBLE WITH ONE ANOTHER. PREFERABLY ONE MANUFACTURER SHALL BE USED FOR ALL ADMIXTURES.
15. CALCIUM CHLORIDE OR CHLORIDE CONTAINING ADMIXTURES **WILL NOT** BE PERMITTED UNDER ANY CIRCUMSTANCES.
16. FLYASH CONTENT, IF APPROVED IN ADVANCE BY ENGINEER, SHALL BE LIMITED TO 20% OF TOTAL CEMENTITIOUS MATERIAL OR 25% OF PORTLAND CEMENT CONTENT. IF FLYASH IS USED, CONTRACTOR SHALL TAKE ADDITIONAL CONCRETE TEST CYLINDERS FOR 56 DAY BREAKS.
17. DURING HOT WEATHER PLACE CONCRETE IN ACCORDANCE WITH ACI 305. DURING COLD WEATHER PLACE CONCRETE IN ACCORDANCE WITH ACI 306.

REINFORCING STEEL

1. REINFORCING BARS SHALL CONFORM TO THE REQUIREMENTS OF CHAPTER 12 OF THE ACI CODE, ASTM A615, GRADE 60 U.N.O..
2. BARS SHALL BE CLEAN OF RUST, GREASE, OR OTHER MATERIALS LIKELY TO IMPAIR BOND. ALL REINFORCING BAR BENDS SHALL BE MADE COLD.
3. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A-185 (MATS ONLY). WWF SHALL BE SUPPORTED ON APPROVED CHAIRS.
4. REINFORCING BAR SPLICES SHALL BE MADE AS INDICATED ON THE DRAWINGS. MINIMUM SPLICE LENGTH FOR REINFORCING STEEL BARS IN MASONRY SHALL BE 40 BAR DIAMETERS, 24" MINIMUM. MINIMUM SPLICE LENGTH FOR REINFORCING STEEL BARS IN CONCRETE SHALL BE PER THE ACI CODE SECTION 12. LAP ALL HORIZONTAL BARS AT CORNERS AND INTERSECTIONS. DOWEL ALL VERTICAL REBAR TO FOUNDATIONS. ALL SPLICE LOCATIONS ARE SUBJECT TO APPROVAL BY STRUCTURAL ENGINEER. PROVIDE REQUIRED SHOP DRAWINGS AND FABRICATE AFTER ENGINEER'S APPROVAL.
5. ALL BARS SHALL BE MARKED SO THEIR IDENTIFICATION CAN BE MADE WHEN THE FINAL IN-PLACE INSPECTION IS MADE.
6. BARS IN SLABS SHALL BE SECURELY SUPPORTED ON WELL-CURED CONCRETE BLOCKS (MAX 2" HIGH) OR METAL CHAIRS, PRIOR TO PLACING CONCRETE.
7. REINFORCING STEEL SHALL BE DETAILED IN ACCORDANCE WITH THE FOLLOWING STANDARDS: "ACI DETAILING MANUAL MNL-66", LATEST EDITION; "CRSI MANUAL OF STANDARD PRACTICE", LATEST EDITION; AND "CRSI REINFORCING BAR DETAILING", LATEST EDITION. THE CONTRACTOR IS SOLELY RESPONSIBLE FOR THE DESIGN AND IMPLEMENTATION OF ADEQUATE PLACEMENT, SPACING, SUPPORT, AND TYING OF REINFORCING STEEL.
8. REBAR SPACINGS GIVEN ARE MAXIMUM ON CENTER WHETHER STATED AS "O.C." OR NOT. ALL REBAR IS CONTINUOUS WHETHER STATED AS "CONT." OR NOT.
9. WHERE REINFORCING IS SHOWN CONTINUOUS THROUGH CONSTRUCTION JOINTS, MECHANICAL BAR SPLICE DEVICES MAY BE USED. SIZES AND TYPES SHALL BE SELECTED TO DEVELOP THE FULL TENSION STRENGTH OF THE BAR PER ICBO RESEARCH REPORT. SUBMIT FOR APPROVAL BY STRUCTURAL ENGINEER.
10. CONTINUOUS INSPECTION OF CONCRETE SHALL INCLUDE INSPECTION DURING INSTALLATION OF REINFORCING STEEL. INSPECTION SHALL BE SCHEDULED SO THAT PLACEMENT OF REINFORCING STEEL, CONDUIT, SLEEVES, AND EMBEDDED ITEMS MAY BE CORRECTED PRIOR TO PLACEMENT OF OVERLYING GRIDS OF REINFORCING STEEL.
11. CONCRETE PROTECTION FOR REINFORCEMENT CAST-IN-PLACE CONCRETE (NON-PRESTRESSED) THE FOLLOWING MINIMUM CONCRETE COVER SHALL BE PROVIDED FOR REINFORCEMENT UNLESS NOTED OTHERWISE:

	COVER
A. CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH:	3"
B. CONCRETE EXPOSED TO EARTH OR WEATHER:	
NO. 6 THROUGH NO. 18 BAR 1 1/2"	2"
NO. 5 BAR, W31 OR D31 WIRE AND SMALLER	1 1/2"
C. CONCRETE NOT EXPOSED TO WEATHER NOR IN CONTACT WITH GROUND:	1 1/2"
D. SLABS, WALLS, JOISTS:	
NO. 11 BAR AND SMALLER	3/4"

12. REPAIR ANY DAMAGED EPOXY COATINGS AS REQUIRED BY ACI AND ASTM REQUIREMENTS.
13. MILL TEST REPORTS FOR GRADE 60 BARS SHALL BE SUBMITTED PRIOR TO PLACEMENT OF CONCRETE.

STRUCTURAL STEEL

1. STRUCTURAL STEEL SHALL BE DETAILED, FABRICATED, AND ERECTED IN ACCORDANCE WITH THE AISC SPECIFICATION FOR THE DESIGN, FABRICATION, AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS LATEST EDITION (EXCLUDING SECTION A7).
2. ALL STRUCTURAL STEEL SHALL CONFORM TO THE ASTM DESIGNATION AS INDICATED BELOW (U.N.O.):

• WF SHAPES:	ASTM A992 (ASTM A572, GR50)
• BASE PLATES, CONNECTION PLATES, ANGLES, CHANNELS, AND MISCELLANEOUS STEEL:	ASTM A-36
• PIPE:	A-53, GRADE B
• TUBE SECTIONS:	A-500
• H.S. BOLTS:	F-3125 GR 325
• NON-STRUCTURAL BOLTS:	A-307

3. THE STRUCTURAL STEEL FABRICATOR SHALL FURNISH SHOP DRAWINGS TO THE ENGINEER OF ALL STEEL FOR STRUCTURAL ENGINEER'S REVIEW AND APPROVAL BEFORE FABRICATION.
4. HOLES IN STEEL SHALL BE 1/16" LARGER DIAMETER THAN NOMINAL SIZE OF BOLT USED, EXCEPT AS NOTED.
5. BEAM END CONNECTIONS SHALL BE DOUBLE ANGLES, OR SHEAR TABS FOR GRAVITY LOADS ONLY, CAPABLE OF SUPPORTING A MINIMUM OF 50% OF THE TOTAL UNIFORM LOAD CAPACITY OF THE BEAM FOR THE GIVEN SPAN.
6. DOUBLE ANGLES SHALL BE SIZED FOR ALL APPLICABLE SPECIFICATIONS IN THE LATEST AISC STEEL CONSTRUCTION MANUAL INCLUDING BUT NOT LIMITED TO PRYING ACTION. MINIMUM 10 KIPS..
7. ALL SHEAR PLATES, WEB STIFFENERS, AND DOUBLE ANGLES SHALL BE MINIMUM 5/16" THICK.
8. BOLTS USED TO CONNECT BEAM TO BEAM GRAVITY LOADS SHALL BE DESIGNED FOR THREAD INCLUDED BEARING (TYPE N). BOLTS CONNECTING BEAMS TO COLUMNS, X-BRACING., OR OTHER SLIP CRITICAL LOADS SHALL BE DESIGNED USING TYPE SC CONNECTION SERVICEABILITY LIMIT VALUES.
9. ALL WELDING IS TO BE DONE BY CERTIFIED WELDERS USING E70XX LOW HYDROGEN ELECTRODES (U.N.O.).
10. ALL WELDS SHALL BE IN CONFORMITY WITH THE PROJECT SPECIFICATIONS AND THE CODE FOR WELDING IN BUILDING CONSTRUCTION (AWS D1.1 LATEST REVISION) OF THE AMERICAN WELDING SOCIETY.
11. THE CONTRACTOR SHALL PROVIDE WELDING INSPECTION REPORTS FROM AN INDEPENDENT INSPECTION SERVICE FOR ALL FIELD WELDING INCLUDING BUT NOT LIMITED TO THE INSPECTION OF SURFACE PREPARATION, WELD QUALITY, AND FINISH. REFER TO THE "SPECIAL INSPECTION" NOTES DRAWING FOR WELDING INSPECTION REQUIREMENTS.
12. WELD LENGTHS CALLED FOR ON PLANS ARE THE NET EFFECTIVE LENGTH REQUIRED. WHERE FILLET WELD SYMBOL IS GIVEN WITHOUT INDICATION OF SIZE, USE MINIMUM SIZE WELDS AS SPECIFIED IN AISC MANUAL OF STEEL CONSTRUCTION, LATEST EDITION.
13. ALL STRUCTURAL STEEL SURFACES THAT ARE ENCASED IN CONCRETE, MASONRY, OR SPRAY ON FIREPROOFING, OR ARE ENCASED BY BUILDING FINISH, SHALL BE LEFT UNPAINTED.
14. PAINT STRUCTURAL STEEL WITH FABRICATOR'S STANDARD LIGHT GRAY RUST INHIBITIVE OXIDE PRIME PAINT UNLESS DIRECTED OTHERWISE BY ENGINEER.

ROUGH CARPENTRY

1. DETAIL, FABRICATE, AND ERECT ALL STRUCTURAL LUMBER IN ACCORDANCE WITH NATIONAL DESIGN SPECIFICATION BY AMERICAN WOOD COUNCIL AND TIMBER CONSTRUCTION MANUAL BY BY AMERICAN INSTITUTE OF TIMBER CONSTRUCTION, LATEST EDITION
2. MATERIALS:
S4S LUMBER (ASLS PS 20)
CONCEALED LOCATIONS
SPECIES: DOUGLAS FIR, HEM FIR OR S-P-F
GRADE: NO. 2 OR BETTER, 19% MC, KILN DRIED
EXPOSED LOCATIONS
SPECIES: DOUGLAS FIR OR AS SELECTED BY ARCHITECT
GRADE: NO. 2 OR BETTER, 19% MC, KILN DRIED
EXTERIOR PRESSURE TREATED
SPECIES: SOUTHERN PINE
GRADE: NO. 2 OR BETTER, 19% MC, KILN DRIED
LAMINATED VENEER LUMBER (LVL) (ASTM D 5456)
Fb = 2800 PSI Fv = 105 PSI E = 1,900,000 PSI
SHEATHING - APA RATED FOR APPLICATION:
EXTERIOR GRADE AT ROOFS
EXPOSURE 1 FOR WALLS AND FLOORS
PLYWOOD - APA VOLUNTARY STANDARD PS-1
ORIENTATED STRAND BOARD - VOLUNTARY STANDARD PS-2
TREATED LUMBER - TO BE FACTORY PRESSURE APPLIED AS FOLLOWS:
EXTERIOR EXPOSURES GROUND CONTACT, AWPA UC3B OR UC4B
FIRE RESISTANT: AWPA UCFA FOR INTERIOR AND UCFB FOR EXTERIOR. SEE ARCHITECTURAL DRAWINGS FOR FIRE TREATED LUMBER LOCATION
3. LUMBER SUPPLIER SHALL FURNISH ALL APPROPRIATE CONNECTIONS FOR ATTACHING LUMBER FRAMING AND ANCHORING TO ADJACENT CONSTRUCTION. CONNECTIONS SHALL BE MADE WITH STANDARD DESIGNS, FABRICATED FROM 16 OR 18 GA. SHEET METAL FOR SINGLE OR DOUBLE 2x LUMBER MEMBERS OR 7, 12 OR 14 GA. STEEL PLATE FOR MULTIPLE PLY, GLULAM OR LVL MEMBERS, AS AS MANUFACTURED BY CLEVE STL SPEC, U S P, SIMPSON STRONGTIE, OR EQUAL. DETAILS SHALL CONFORM TO AITC STANDARD NO. 104.
4. PROVIDE SOLID BLOCKING OR 'X' BRIDGING FOR 2X FLOOR JOISTS AT MID-SPAN OR 8'-0" O.C. WHICHEVER IS LESS. BLOCKING OR BRIDGING IS NOT REQUIRED FOR "I" JOISTS.
5. BOLTS, NAILS, SPIKES, AND OTHER CONNECTORS SHALL BE APPROPRIATE FOR THE USE INTENDED. FASTENERS EXPOSED TO FIRE-TREATED LUMBER, CHEMICAL FUMES, WEATHER AND/OR HIGH HUMIDITY SHALL BE HOT DIPPED GALVANIZED. UNLESS INDICATED OTHERWISE ON DRAWINGS.
6. PROVIDE TEMPORARY BRACING UNTIL 50 % OF DRYWALL IS INSTALLED ON EACH SIDE OF WALL.
7. DESIGN, FABRICATE AND ERECT PRE-ENGINEERED WOOD TRUSSES IN ACCORDANCE WITH TRUSS PLATE INSTITUTE "DESIGN SPECIFICATION FOR METAL PLATE CONNECTED WOOD TRUSSES." SUBMIT SHOP DRAWINGS FOR APPROVAL PRIOR TO FABRICATION. DRAWINGS ARE TO INCLUDE: DESIGN LOADS, REACTIONS, MEMBER SIZES, STRESSES, PLATE SIZES, DIMENSIONS, AND ERECTION DRAWINGS AS REQUIRED. TRUSS MANUFACTURER TO PROVIDE CERTIFIED DOCUMENTS INDICATING THE MANUFACTURER HAS A MINIMUM OF 5 YEARS EXPERIENCE IN DESIGNING AND PRODUCING TRUSSES FOR NON-RESIDENTIAL CONSTRUCTION. FAILURE TO SUBMIT THIS DOCUMENT WILL BE CAUSE FOR REJECTION OF TRUSS MANUFACTURER AND ANY TRUSS SUBMITTALS.
8. SUBMIT PE SEALED "FOR CONSTRUCTION" TRUSS DESIGN AND ERECTION DRAWINGS TO LOCAL BUILDING DEPARTMENT PER PERMIT REQUIREMENTS.
9. ERECT PRE-ENGINEERED WOOD TRUSSES IN ACCORDANCE WITH TRUSS PLATE INSTITUTE DSB-89 "TEMPORARY BRACING OF METAL PLATE CONNECTED WOOD TRUSSES" INCLUDING GROUND BRACE, LATERAL BRACES AND DIAGONAL 'X' BRACES. IF NOT SHOWN OTHERWISE ON CONSTRUCTION DOCUMENTS, TEMPORARY BRACING IS TO BE LEFT PERMANENTLY IN PLACE. PROVIDE WOOD HEADERS AS PER THE FOLLOWING SCHEDULE IN ALL STUD WALL
10. OPENINGS WHEN NOT SHOWN ON DRAWINGS, OR IN OPENINGS REQUIRED BY THE ARCHITECTURAL, MECHANICAL, AND ELECTRICAL DRAWINGS.
FOR OPENINGS FROM 4'-0" TO 6'-0": 2-2x10's
FOR OPENINGS FROM 6'-1" TO 8'-0": 2-2x12's
FOR OPENINGS FROM 8'-1" TO 10'-0": 2-1 3/4 x 11 7/8 LVL
11. STUD SCHEDULE - USE THE FOLLOWING SCHEDULE, UNLESS NOTED OTHERWISE ON PLANS. PROVIDE ONE ADDITIONAL KING STUD EACH SIDE, FULLY NAILED TO JACK STUDS. FOR BEARING WALLS, PROVIDE ONE ADDITIONAL JACK STUD FOR EACH ADDITIONAL BEAM LAMINATION.

MAX OPENING SIZE	BEARING WALLS	NON-BEARING WALLS
UP TO 4'	ONE	ONE
OVER 4' TO 8'	2	ONE
OVER 8' TO 12'	3	2
OVER 12'	SEE PLAN	2

12. ADD ONE 2x MEMBER TO HEADER FOR EACH 2" NOMINAL WALL WIDTH.
13. PROVIDE BEARING JACK-STUDS EQUAL TO NUMBER OF BEAM LAMINATIONS PLUS ONE KING-STUD AT ALL BEAM BEARING LOCATIONS. STUDS ARE TO EXTEND DOWN TO SOLID OR BEAM BEARING OR AS NEEDED. BLOCK SOLID AS NEEDED.

COLD FORMED STEEL FRAMING

1. ALL LIGHT GAGE STEEL FRAMING AND CONNECTIONS SHALL BE DETAILED, FABRICATED AND ERECTED IN ACCORDANCE WITH AISI "SPECIFICATION FOR THE DESIGN OF COLD FORMED STEEL STRUCTURAL MEMBERS", SSMA "PRODUCT TECHNICAL INFORMATION" AND NAAMM ML/SFA 540, "LIGHTWEIGHT STEEL FRAMING SYSTEMS MANUAL".
2. THE CFS FRAMING SIZES SHOWN ON THE DRAWINGS ARE FOR BIDDING PURPOSES ONLY. THE GC WILL BE RESPONSIBLE FOR RETAINING THE SERVICES OF A DELEGATED DESIGN PROFESSIONAL ENGINEER, LICENSED IN THE STATE WHERE THE PROJECT IS LOCATED, TO PROVIDE THE FINAL DESIGN OF ALL CFS MEMBERS AND PROVIDE ALL THE CONNECTION DESIGN AND DETAILS NECESSARY TO CONSTRUCT THE CFS FRAMING. THE GC SHALL BE SOLELY RESPONSIBLE FOR ANY COST INCREASE SHOULD THE DELEGATED ENGINEER INCREASE THE MEMBER SIZES, GAUGES OR SPACING. NO CHANGE ORDER FOR CFS FRAMING CHANGES WILL BE ACCEPTED.
3. ALL LIGHT GAGE STEEL FRAMING SHALL CONFORM TO THE FOLLOWING:

GALVANIZED STUDS 97, 68, 0R 54 MILS	ASTM A653 GR D (Fy - 50,000 PSI)
GALVANIZED STUDS 43 OR 33 MILS	ASTM A653 GR A (Fy = 33,000 PSI)
GALVANIZED TRACK, END CLOSURES, BRIDGING, AND ACCESSORIES	ASTM A653 GR A (Fy = 33,000 PSI)

4. DOUBLE VERTICAL STUDS SHALL BE STITCH WELDED TOGETHER ON BOTH FLANGES WITH 1/16" GROOVE WELDS X 1" LONG AT 12" ON CENTER.
5. TOP STUDS TRACKS FOR EXTERIOR WALLS SHALL BE 54 MILS MATERIAL WITH 1.25" FLANGES. BOTTOM STUD TRACKS FOR EXTERIOR WALLS SHALL BE 54 MILS MATERIAL WITH 1.25" FLANGES.
6. MINIMUM STUD PROPERTIES SHALL BE PER STEEL STUD MANUFACTURER'S ASSOCIATION ICBO NO. 4943P, "PRODUCT TECHNICAL INFORMATION".
7. ALL SHEET METAL SCREWS SHALL PROTRUDE 1/4" THROUGH METAL FRAMING.
8. ALL METAL STUDS SHALL HAVE STIFFENED FLANGES. SEE MANUFACTURER'S SPECIFICATIONS AND RECOMMENDATIONS.
9. LIGHT GAGE STEEL FRAMING CONNECTIONS SHALL BE DETAILED BY CONTRACTOR. CONTRACTOR TO PROVIDE ALL CONNECTION DETAILS. SUBMIT SHOP DRAWINGS FOR REVIEW OF CONNECTIONS. CONNECTIONS TO BE DESIGNED BY LICENSED PROFESSIONAL ENGINEER. SUBMIT SEALED CALCULATIONS WITH DETAILS
10. ALL CONNECTIONS ARE TO BE SCREWED WITH MINIMUM 4 #12-14 SCREWS OR EQUIVALENT WELDS UNLESS NOTED.
11. DIAGONAL STRAP BRACING, WHERE SHOWN OR REQUIRED, SHALL BE CONNECTED TO ALL CROSSING VERTICAL MEMBERS.
12. WELDING SHALL BE DONE IN ACCORDANCE WITH AWS D1.3, STRUCTURAL WELDING CODE, SHEET STEEL.
13. JOIST BRIDGING SHALL BE PROVIDED ON 8 FOOT CENTER MAXIMUM UNLESS INDICATED OTHERWISE ON THE DRAWINGS.
14. SEQUENCING OF WELDS SHALL BE SUCH TO AVOID DISTORTION OF MEMBERS. REPLACE ALL MEMBERS WHEN BURN THROUGH OCCURS DURING WELDING OPERATIONS.
15. PROVIDE LIGHT GAGE STEEL HEADERS AS PER THE FOLLOWING SCHEDULE IN ALL STUD WALL OPENINGS WHEN NOT SHOWN ON DRAWINGS, OR IN OPENINGS REQUIRED BY THE ARCHITECTURAL, MECHANICAL, AND ELECTRICAL DRAWINGS.
OPENINGS UP TO 6'-0" 2-600S162-54
OPENINGS 6'-1" TO 8'-0"` 2-800S162-54
OPENINGS 8'-1" TO 10'-0" 2-1000S162-54
OVER 10'-0" SEE PLAN
PROVIDE NUMBER OF BEAM LAMINATIONS EQUAL TO WALL WIDTH
16. ALL HEADERS SHALL BEAR ON 2 STUDS AT EACH END, UNLESS NOTED.
17. ADD ONE JOIST MEMBER IN HEADERS FOR EACH ADDITIONAL 2" NOMINAL WALL WIDTH.
18. PROVIDE STUDS EQUAL TO NUMBER OF BEAM LAMINATIONS PLUS 2 UNDER ALL BEAM BEARING LOCATIONS. STUDS ARE TO EXTEND DOWN TO FOUNDATION OR OTHER SUPPORT POINTS AS NEEDED
19. PRODUCT PROPERTIES BASED UPON DIETRICH CATALOG 18. INFORMATION. CONTRACTOR MAY SUBSTITUTE MANUFACTURER UPON ENGINEER REVIEW AND APPROVAL
20. EXTERIOR SHEATHING TO BE 1/2" FIBERGLASS FACED GYPSUM BOARD . U.N.O. ON STRUCTURAL OR ARCHITECTURAL DRAWINGS
21. PROVIDE TEMPORARY BRACING UNTIL 50 % OF DRYWALL IS INSTALLED ON EACH SIDE OF WALL.
22. ALL LIGHT GAUGE STEEL JOISTS SHALL HAVE WEB STIFFENERS AT ALL BEARING LOCATIONS.

Statement of Special Inspections

(Section 1704 Ohio Building Code)

Application No.:
Project name: NORTH MAIN RETAIL
Project location: 201 N0RTH MAIN STREET, HUDSON, OHIO

Per section 1704.2 of the Ohio Building Code, where application is made to the building official for construction as specified in section 105, the owner or the owner's representative, shall employ one or more special inspectors to provide special inspections and tests during construction on the types of work specified in section 1705 and identify the approved agencies to the building official. These special inspections and tests are in addition to the inspections by the building official that are identified in section 108.

Per section 1704.2.3 of the Ohio Building Code, the applicant shall submit a statement of special inspections as a condition for the issuance of a plan approval. This statement shall be in accordance with section 1704.3. The statement of special inspections shall identify the following per OBC 1704.3.1:

1. The materials, systems, components and work required to have special inspections or tests by the registered design professional responsible for each portion of the work.
2. The type and extent of each special inspection.
3. The type and extent of each test.
4. Additional requirements for special inspections or tests for seismic or wind resistance as specified in sections 1705.11, 1705.12 and 1705.13.
5. For each type of inspection, identification as to whether it will be continuous special inspection, periodic special inspection or performed in accordance with the notation used in the referenced standard where the inspections are defined.

Contractor responsibility - OBC 1704.4:
Each contractor responsible for the construction of a main wind or seismic force-resisting system, designated seismic system or a wind or seismic force-resisting component listed in the statement of special inspections shall submit a written statement of responsibility to the building official and the owner or the owner's representative prior to the commencement of work on the system or component. The contractor's statement of responsibility shall contain acknowledgement of awareness of the special requirements contained in the statement of special inspections.

Special inspector qualifications - OBC 1704.2.1:
Prior to the start of the construction, the special inspectors shall provide written documentation to the building official demonstrating the competence and relevant experience or training of the special inspectors who will perform the special inspections and tests during construction. Experience or training shall be considered relevant where the documented experience or training is related in complexity to the same type of special inspection or testing activities for projects of similar complexity and material qualities. These qualifications are in addition to qualifications specified in other sections of this code.

The registered design professionals involved in the design of the project are permitted to act as special inspectors and their personnel are permitted to act as special inspectors for the work designed by them, provided they qualify as special inspectors.

Access for special inspection - OBC 1704.2.2:
The construction or work for which special inspection or testing is required shall remain accessible and exposed for special inspection or testing purposes until completion of the required special inspections or tests.

SCHEDULE OF SPECIAL INSPECTIONS

ITEM	Required	Continuous Inspection	Periodic Inspection
Structural Steel - Inspection Prior to Bolting (OBC 1705.2/AISC 360 Table N5.6-1)			
• Manufacturer's certifications available for fastener materials			
• Fasteners marked in accordance with ASTM requirements			
• Proper fastener selection for the joint detail			
• Proper bolting procedure selected for joint detail			
• Connecting elements			
• Pre-installation verification testing			
• Proper storage			
Structural Steel - Inspection During Bolting (OBC 1705.2/AISC 360 Table N5.6-2)			
• Fastener assemblies			
• Joint brought to snug-tight before pretensioning			
• Fastener component rotation prevention			
• Fastener pretensioning			
Structural Steel - Inspection After Bolting (OBC 1705.2/AISC 360 Table N5.6-3)			
• Document acceptance/rejection of bolted connections			
Structural Steel - Other Inspections (OBC 1705.2/AISC 360 N5.7)			
• Anchor bolt installation/verification			
• Compliance of fabricated steel & erected steel frame			

ITEM	Required	Continuous Inspection	Periodic Inspection
Concrete Construction (OBC 1705.3)			
• Reinforcing steel inspection and placement			
• Cast-in-place anchors			
• Post-installed anchors - mechanical and adhesive			
• Required mix design			
• On-site concrete testing			
• Concrete and shotcrete application techniques			
• Maintenance of curing temperature and techniques			
• Concrete formwork			
Wood Construction (OBC 1705.5)			
• Prefabricated structural elements and assemblies			
Soils (OBC 1705.6)			
• Bearing capacity of soils			
• Excavations			
• Compacted fill materials - classification and testing			
• Verify use of compacted fill materials, densities and lift thicknesses			
• Subgrade and site preparation			
Helical Pile Foundations (OBC 1705.9)			
• Record installation equipment, pile dimensions, tip elevations, final depth, final installation torque			

Final Report of Special Inspections and Tests

Application No.:
Project name: NORTH MAIN RETAIL
Project location: 201 NORTH MAIN STREET, HUDSON, OHIO

Pursuant to section 1704.2.4 of the Ohio Building Code, special inspectors shall keep records of special inspections and tests. The special inspectors shall submit reports of special inspections and tests to the building official and to the registered design professional in responsible charge. Reports shall indicate that work inspected or tested was or was not completed in conformance to approved construction documents. Discrepancies shall be brought to the immediate attention of the contractor for correction. If they are not corrected, the discrepancies shall be brought to the attention of the building official and to the registered design professional in responsible charge prior to the completion of that phase of the work. A final report documenting required special inspections and tests, and correction of any discrepancies noted in the inspections or tests, shall be submitted at a point in time agreed upon prior to the start of work by the owner or the owner's representative to the building official prior to the issuance of the certificate of occupancy.

FINAL REPORT OF SPECIAL INSPECTIONS AND TESTS

ITEM	Date	Corrections
Structural Steel - Inspection During Bolting (OBC 1705.2/AISC 360 Table N5.6-2)		
Structural Steel - Inspection After Bolting (OBC 1705.2/AISC 360 Table N5.6-3)		
Structural Steel - Other Inspections (OBC 1705.2/AISC 360 N5.7)		
Concrete Construction (OBC 1705.3)		
Wood Construction (OBC 1705.5)		
Soils (OBC 1705.6)		

3

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STATE OF OHIO JOSEPH MATAVA 0614145 REGISTERED ARCHITECT
JOSEPH MATAVA
0614145 | 12/31/23

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NORTH MAIN RETAIL
201 NORTH MAIN STREET, HUDSON, OHIO

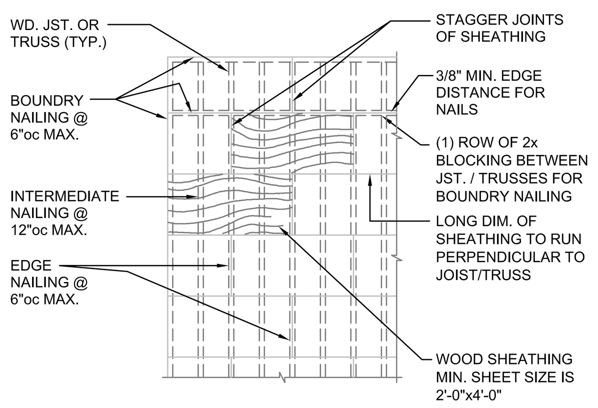
N

PROJECT #: 2317

ISSUE	ID	DATE
BULLETIN #2	2	01/02/24
BULLETIN #3	3	01/10/24

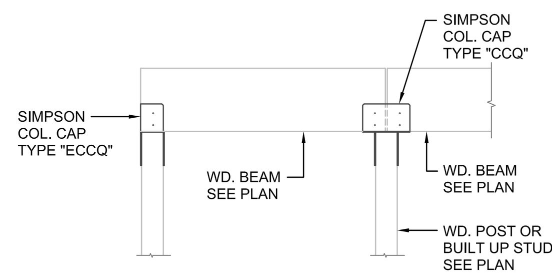
GENERAL STRUCTURAL NOTES

S0.01

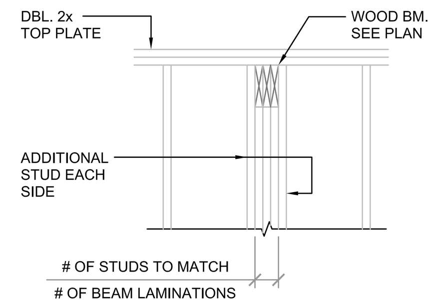


TYP. SHEATHING LAYOUT ROOF & FLR.

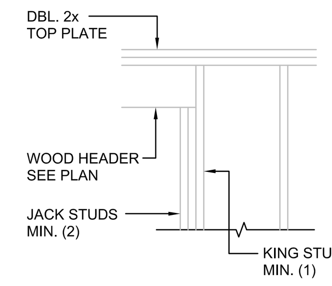
- ROOF SHEATHING SHALL BE WITH 8d COMMON NAILS PROVIDE 1/8" GAP BETWEEN SHEETS OF ROOF SHEATHING



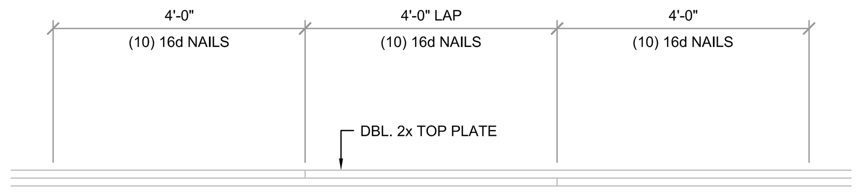
WOOD BM. TO WOOD POST CONNECTION



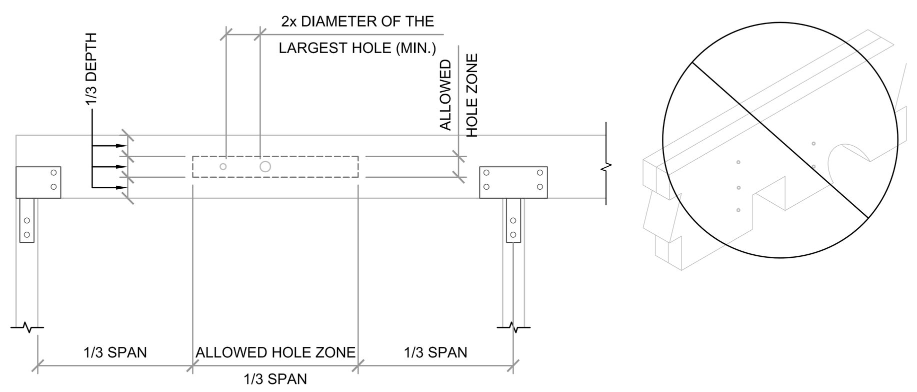
TYP. WOOD BM. SUPPORT DETAIL



TYP. HEADER SUPPORT DETAIL



TYP. LAP SPLICE OF DBL. 2x TOP PLATES



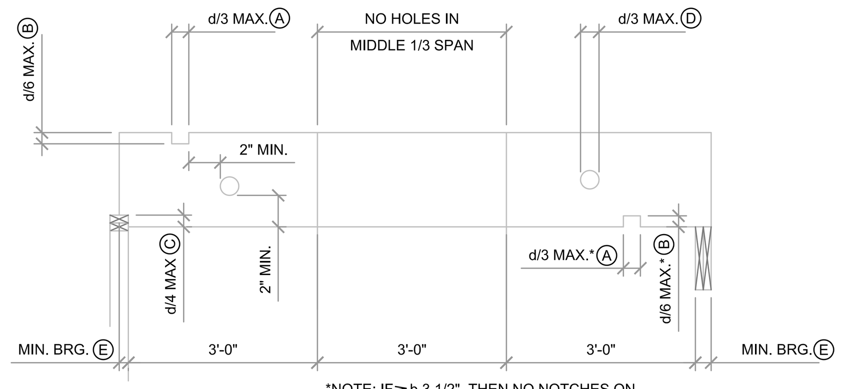
NOTES:

1. THE ALLOWED HOLE ZONE IS SUITABLE ONLY FOR UNIFORMLY LOADED BEAMS
2. RECTANGULAR HOLES ARE NOT ALLOWED
3. HOLES IN CANTILEVERS REQUIRE ADDITIONAL ANALYSIS
4. IF LARGER HOLES ARE REQUIRED, CONTACT STRUCTURAL ENGINEER OF RECORD FOR EVALUATION

DO NOT CUT, NOTCH OR DRILL HOLES IN MICROLAM LVL'S EXCEPT AS INDICATED IN TABLE BELOW AND ILLUSTRATION AT LEFT

BEAM DEPTH	MAX. ROUND HOLE SIZE
5 1/2"	1 3/4"Ø
7 1/4" TO 18"	2"Ø

ALLOWABLE HOLES IN LVL & PARALLAM BEAMS

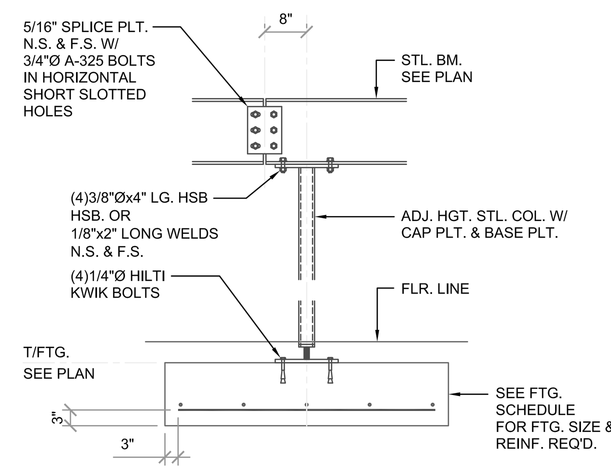


*NOTE: IF ≥ b 3 1/2", THEN NO NOTCHES ON TENSION SIDE EXCEPT AT ENDS
b = MEMBER THICKNESS

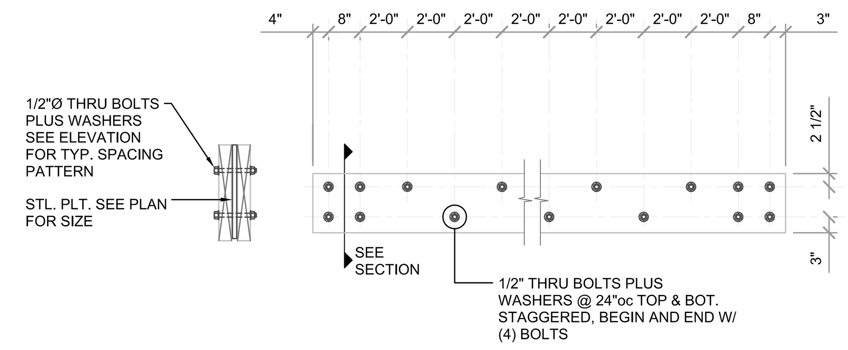
JST. SIZE	(A) MAX. NOTCH LENGTH	(B) MAX. NOTCH DEPTH	(C) MAX. END NOTCH DEPTH	(D) MAX. HOLE DEPTH	(E) MIN. (1) BRG. LENGTH
2x8	2 3/8"	1 3/16"	1 13/16"	2 3/8"	1 1/2" 3"
2x10	3 1/16"	1 1/2"	2 5/16"	3 1/16"	1 1/2" 3"
2x12	3 3/4"	1 7/8"	2 13/16"	3 3/4"	1 1/2" 3"

NOTE:
(1) MINIMUM BRG.: 1 1/2" ON WOOD OR STEEL, 3" ON MASONRY

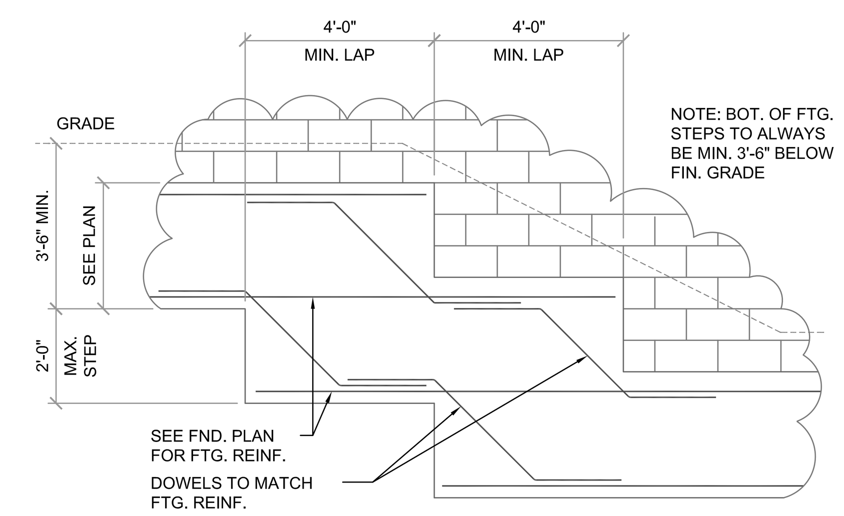
ALLOWABLE HOLES AND NOTCHES IN JOIST



TYP. STL. BM. TO ADJUSTABLE STL. COL.

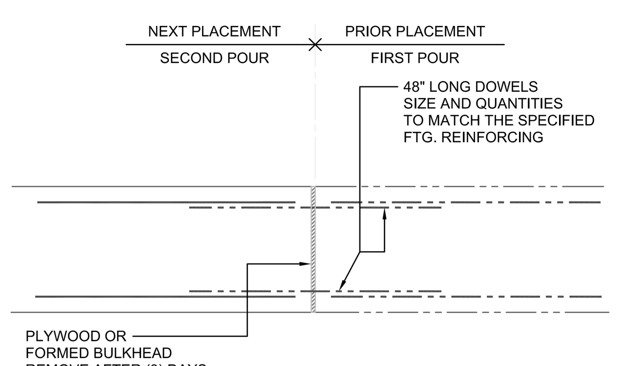


TYP. FLITCH BEAM DETAIL

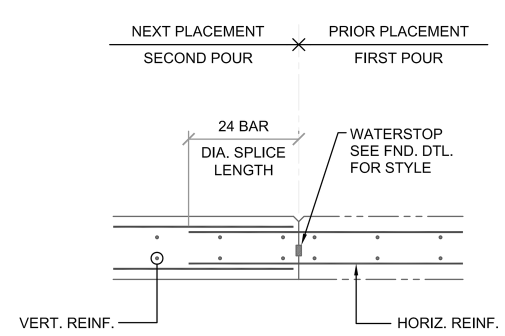


TYP. STEP FTG. DETAIL

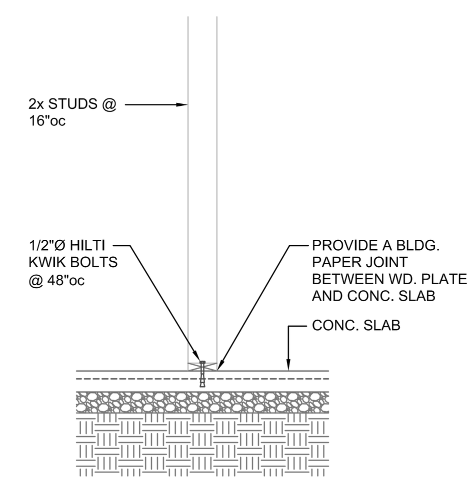
- USE AS REQ'D. G.C. TO COORD. WITH FINAL SITE GRADING PLAN ENGINEER OF RECORD IS NOT RESPONSIBLE FOR G.C. ERROR
- NOTED ON PLAN THUS S.F.



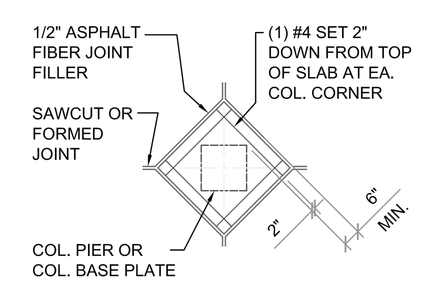
TYP. CONC. FND. CONST. JOINT



FOUNDATION WALL CONST. JOINT

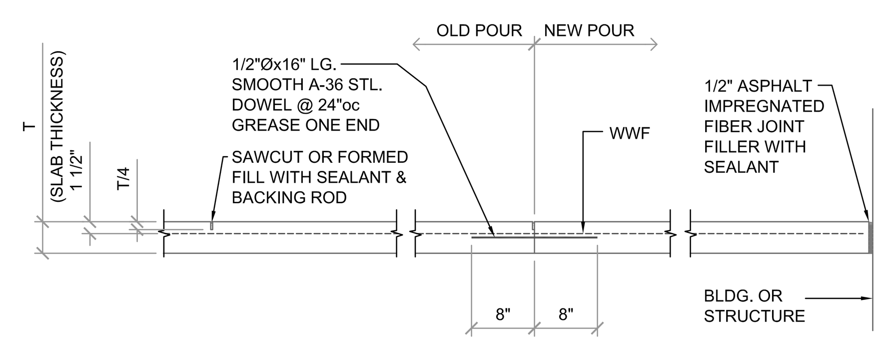


TYP. WOOD WALL TO CONC. SLAB



COL. ISOLATION

COL. NOT SHOWN SEE PLAN FOR SIZE & DIRECTION



CONTROL

CONSTRUCTION

ISOLATION

SLAB ON GRADE JOINT DETAILS

FASTENING SCHEDULE		
CONNECTION	FASTENING a, b	LOCATION
SOLE PLATE TO JST. OR BLOCKING	16d @ 16"oc 3" x 0.131" NAILS @ 8"oc	TYPICAL FACE NAILING
SOLE PLATE TO JST. OR BLOCKING AT BRACED WALL PANEL	(3)16d NAILS PER 16" 3" x 0.131" NAILS @ 16"oc	BRACED WALL PANELS
TOP PLATE TO STUD	(2)16d COMMON 3" x 0.131" NAILS	END NAIL
STUD TO SOLE PLATE	(4)8d COMMON 3" x 0.131" NAILS (2)16d COMMON 3" x 0.131" NAILS	TOE NAIL END NAIL
DOUBLE STUDS	16d NAILS @ 24"oc 3" x 0.131" NAILS @ 8"oc	TYPICAL FACE NAILING
DOUBLE TOP PLATES	16d NAILS @ 16"oc 3" x 0.131" NAILS @ 12"oc (8)16d COMMON (12) 3" x 0.131" NAILS	FACE NAIL LAP SPLICE
BLOCKING BETWEEN JST. OR RAFTERS TOP PLATE	(3)8d COMMON (3) 3" x 0.131" NAILS	TOE NAIL
RIM JST. TO TOP PLATE	8d @ 6"oc 3" x 0.131" NAILS @ 16"oc	TOE NAIL
TOP PLATES, LAPS AND INTERSECTIONS	(2)16d COMMON (3) 3" x 0.131" NAILS	FACE NAIL
RAFTERS TO PLATE SEE SECTION 2308.10.1 TABLE 2308.10.1	(3)8d COMMON (3) 3" x 0.131" NAILS	TOE NAIL
1" DIAGONAL BRACE TO EACH STUD AND PLATE	(2)8d COMMON (2) 3" x 0.131" NAILS	FACE NAIL
BUILT UP CORNER STUDS	16d COMMON 3" x 0.131" NAILS 3" x 14 GAGE STAPLES	24"oc 16"oc 16"oc
BUILT UP GIRDER AND BMS.	20d COMMON @ 32"oc 3" x 0.131" NAILS @ 24"oc (2)20d COMMON (3) 3" x 0.131" NAILS	FACE NAIL AT TOP & BOTTOM STAGGERED ON OPPOSITE SIDES
LEDGER STRIP	(3)16d COMMON (4) 3" x 0.131" NAILS	FACE NAIL

a. COMMON OR BOX NAILS ARE PERMITTED TO BE USED EXCEPT WHERE OTHERWISE NOTED
b. SEE SECTIONS FOR FASTENING NOTES NOT SHOWN IN THIS TABLE

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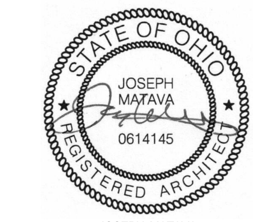

JOSEPH MATAVA
0614145 | 12/31/23

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NORTH MAIN RETAIL
201 NORTH MAIN STREET, HUDSON, OHIO

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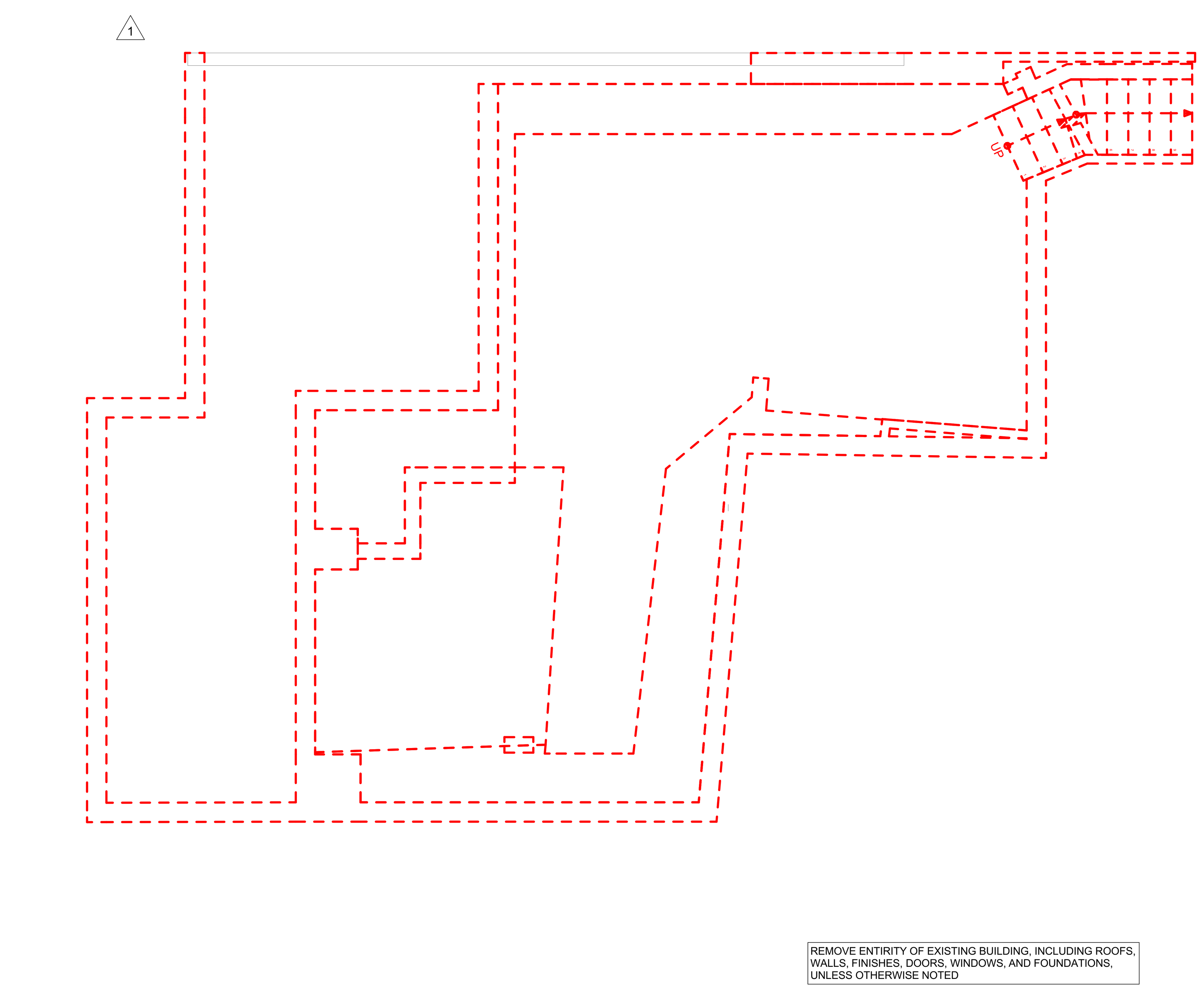
PROJECT #: 2317

ISSUE	ID	DATE
BULLETIN #3	3	01/10/24

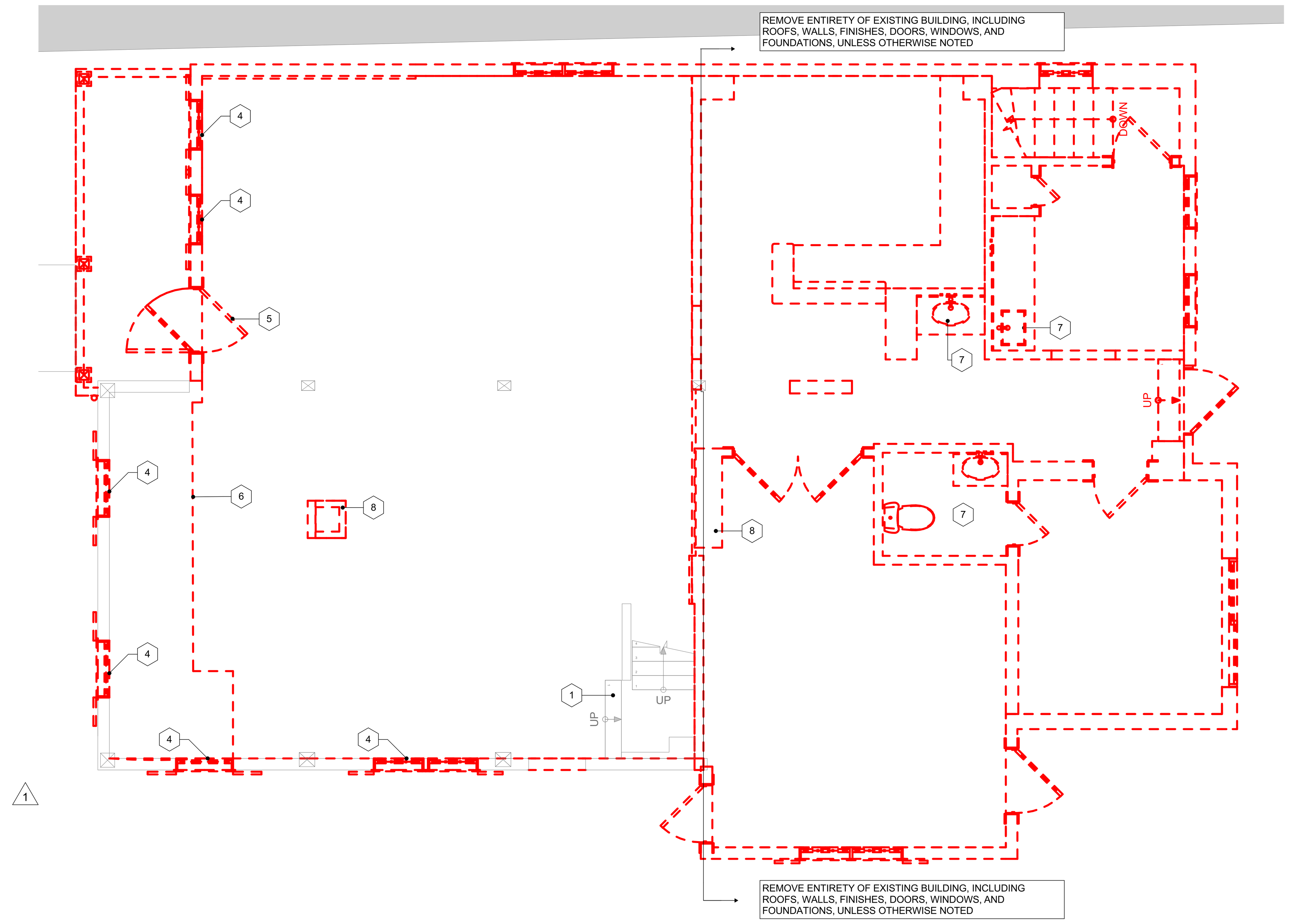
STRUCTURAL DETAILS

3

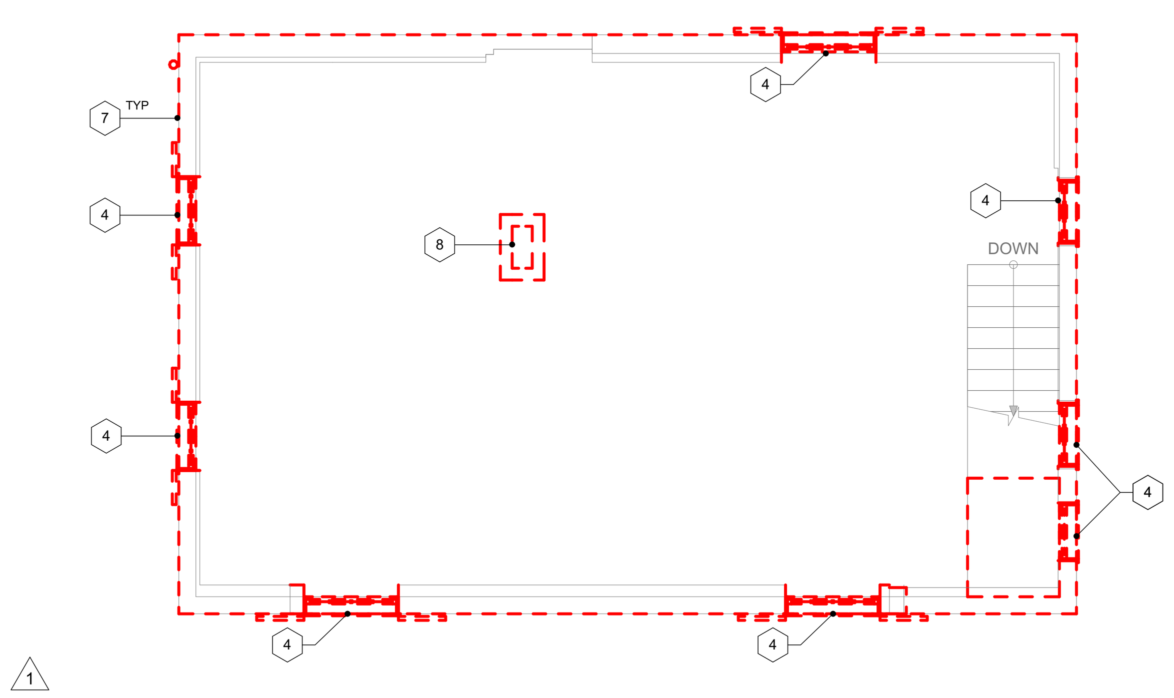
S0.02



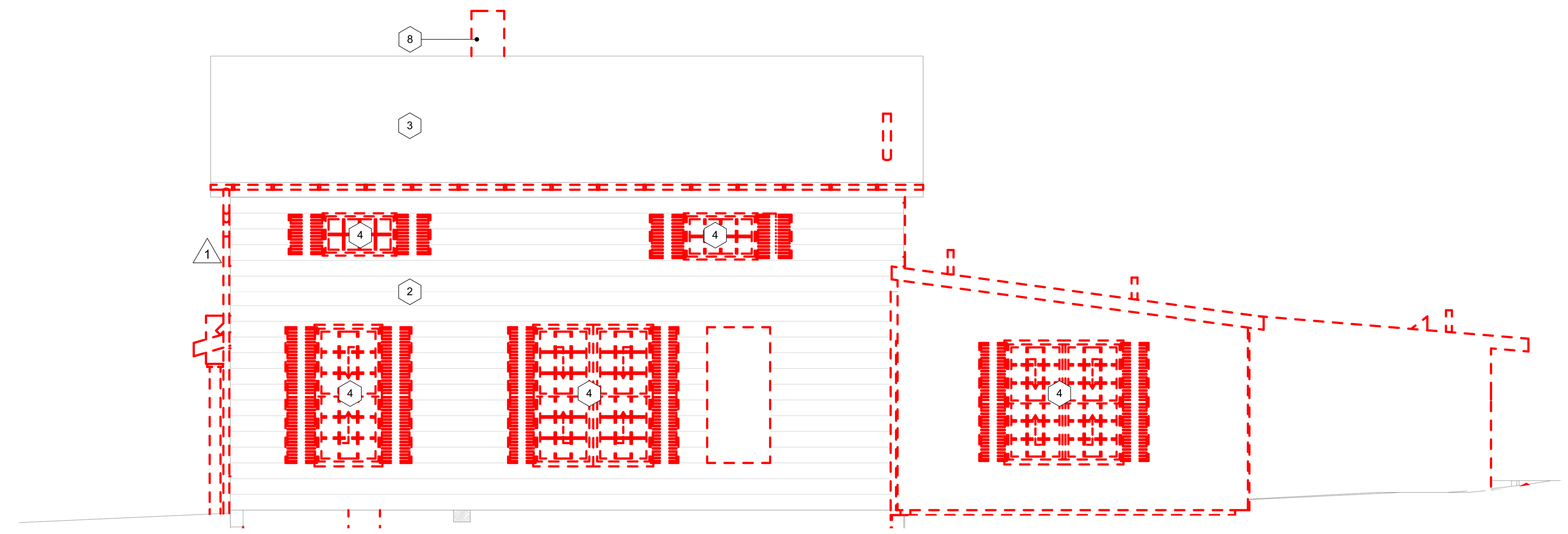
1 BASEMENT DEMOLITION FLOOR PLAN
SCALE: 1/4" = 1'-0"



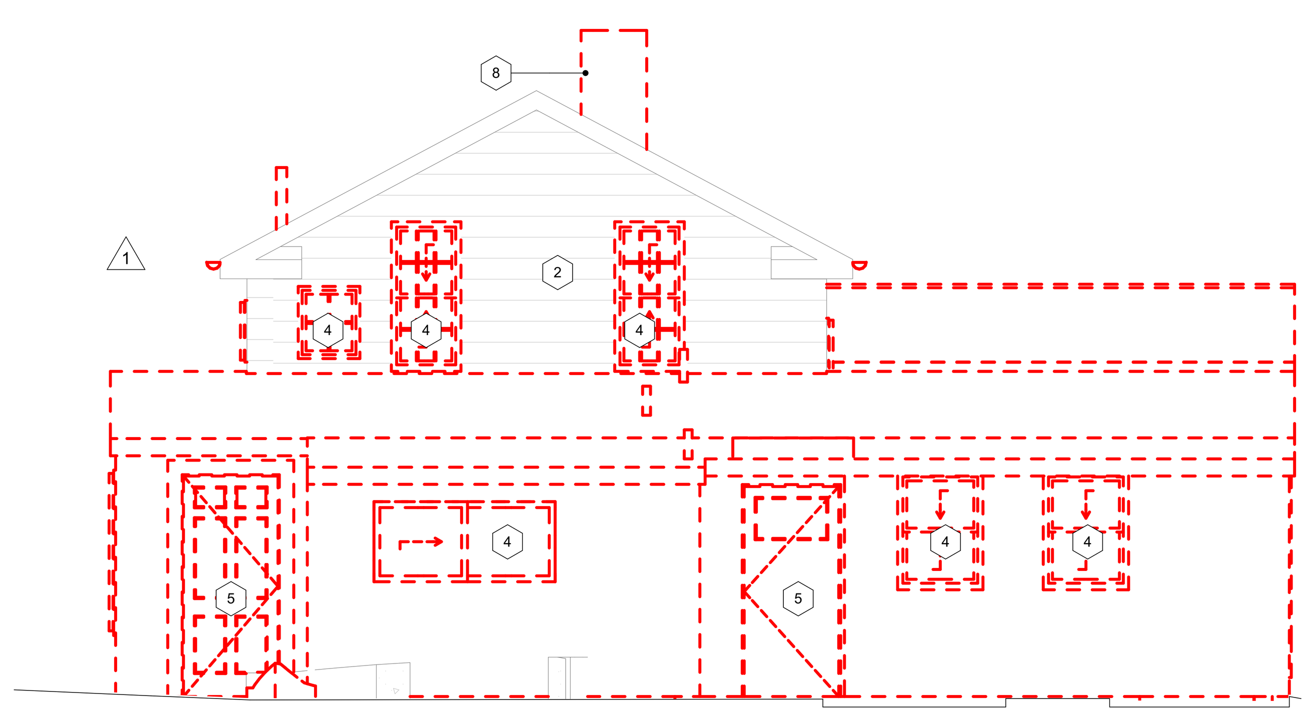
2 FIRST FLOOR DEMOLITION PLAN
SCALE: 1/4" = 1'-0"



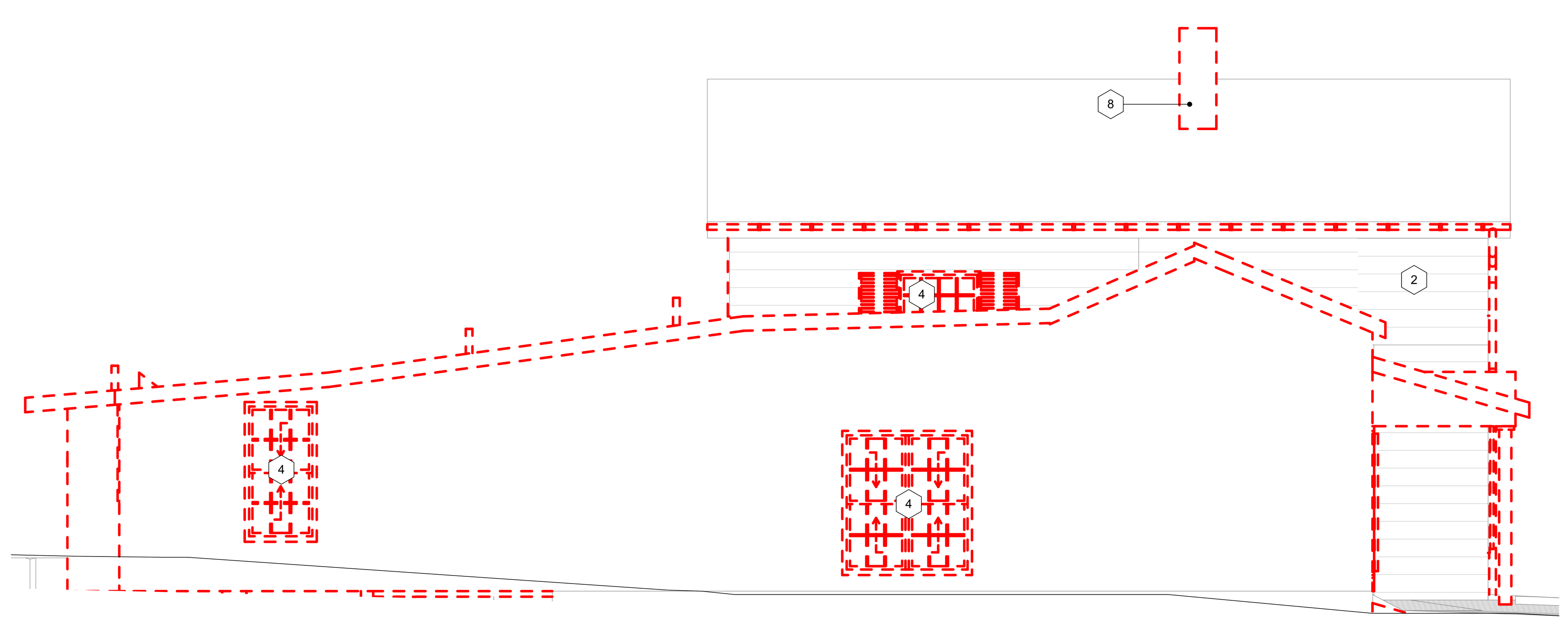
3 SECOND FLOOR DEMOLITION PLAN
SCALE: 1/4" = 1'-0"



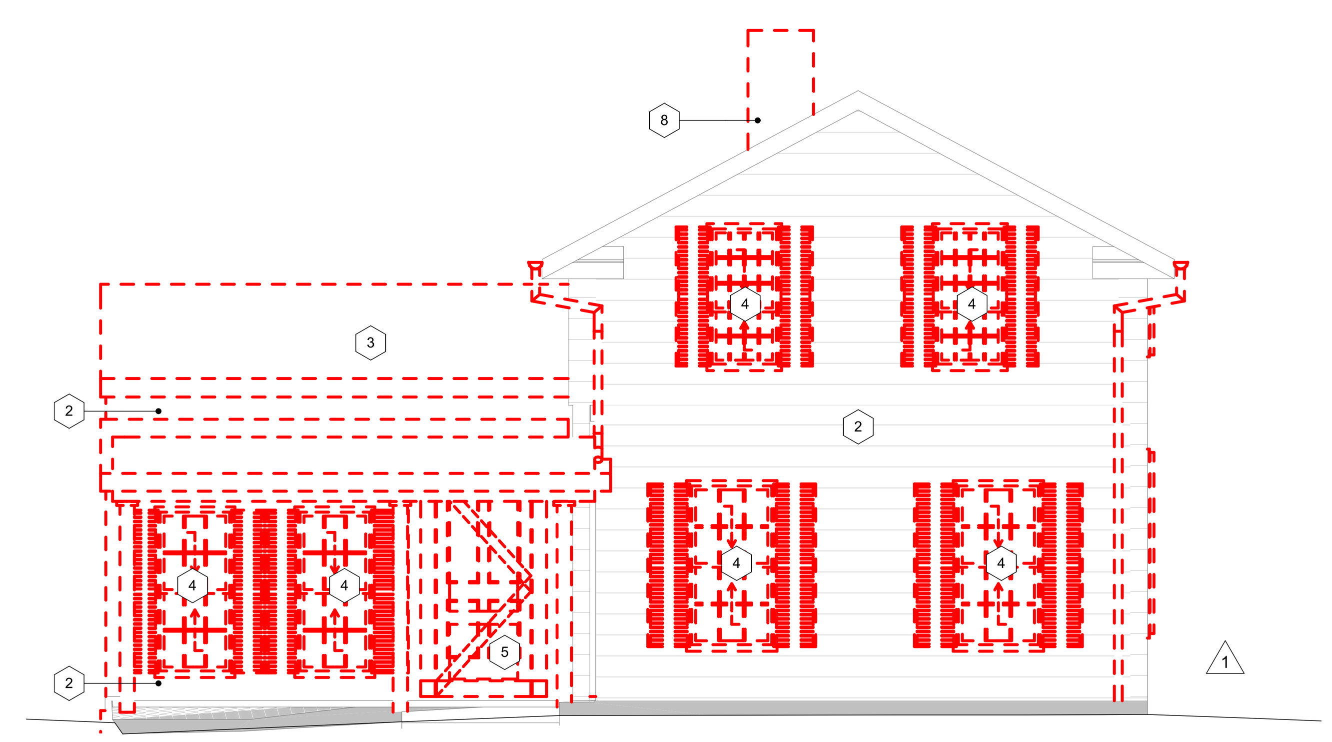
4 DEMOLITION - EXTERIOR ELEVATION - SOUTH (SIDE OF THE BUILDING)
SCALE: 1/4" = 1'-0"



5 DEMOLITION - EXTERIOR ELEVATION - EAST (BACK OF THE BUILDING)
SCALE: 1/4" = 1'-0"



6 DEMOLITION - EXTERIOR ELEVATION - NORTH (SIDE OF THE BUILDING)
SCALE: 1/4" = 1'-0"



7 DEMOLITION - EXTERIOR ELEVATION - WEST (FRONT OF THE BUILDING)
SCALE: 1/4" = 1'-0"

DEMOLITION GENERAL NOTES

1. REMOVE ENTIRE WALL TO UNDERSIDE OF DECK UNLESS OTHERWISE NOTED.
2. IN DEMOLITION AREAS, REMOVE ALL MISCELLANEOUS ITEMS ON WALLS, UNLESS OTHERWISE NOTED.
3. FILL AND LEVEL ALL HOLES IN FLOORS AND WALLS AFTER REMOVAL OR ADDITION OF PIPES, DUCTS, CONDUITS, AND OTHER PENETRATING ITEMS. MAINTAIN REQUIERD FIRE RATING.
4. REMOVE CEILING WHERE NEW CEILINGS ARE INDICATED AND NOTED.
5. TAKE ALL NECESSARY PRECAUSIONS TO PREVENT DAMAGE TO ADJACENT AREAS. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL DAMAGE INCURRED.
6. ALL DEMOLITION ACTIVITIES SHALL BE COORDINATE WITH THE OWNER.
7. PATCH AND REPAIR ALL EXISTING ADJACENT INTERIOR FINISHES TO REMAIN AS UNDISTURBED BY THE DEMOLITION TO MATCH ADJACENT SURFACES.
8. ALL DEMOLITION ITEMS ARE TO REMAIN THE PROPERTY OF THE OWNER AT THEIR DISCRETION.
9. ALL STRUCTURAL MEMBERS ARE TO REMAIN AND BE PROTECTED UNLESS OTHERWISE NOTED.
10. NOTIFY ARCHITECT PRIOR TO DEMOLITION IF ITEM INDICATED TO BE REMOVED IS SUSPECT AS STRUCTURAL ELEMENT.
11. MAINTAIN BUILDING IN WEATHERTIGHT CONDITIONS AT ALL TIMES.
12. ITEMS TO BE REMOVED SHOWN DASHED, TYPICAL
13. WHERE WINDOWS AND DOORS ARE TO BE REMOVED, INFILL WALLS WITH FINISHES TO MATCH SURROUNDING AREAS
14. COORDINATE WITH FLOOR PLANS FOR NEW DOOR AND WINDOW OPENINGS
15. PROVIDE TEMPORARY SHORING FOR FLOOR AND ROOF FRAMING TO REMAIN
16. PERFORM PARTIAL WALL DEMOLITION IN ORDER TO ACCOMODATE NEW HEADERS, BEAMS, AND POSTS/ COLUMNS
17. REMOVE ALL EXISTING RECEPTACLES AND RELATED CONDUITS AND WIRING THAT ARE AFFECTED BY THE WALL DEMOLITION BACK THE ELECTRICAL PANEL, MAINTAIN CIRCUIT CONTINIUTY.

DEMOLITION CODED NOTES

NOTE: ALL CODED NOTES MAY NOT OCCUR ON EVERY SHEET

1 EXISTING STAIR AND HANDRAIL TO REMAIN
2 EXISTING SIDING TO BE REMOVED
3 EXISTING ROOF TO BE REMOVED
4 EXISTING WINDOWS TO BE REMOVED
5 EXISTING DOOR TO BE REMOVED
6 REMOVE PORTION OF THE FLOOR FOR NEW INTERIOR STAIR
7 REMOVE EXISTING PLUMBING FIXTURES
8 REMOVE EXISTING CHIMNEY

DEMOLITION LEGEND

EXISTING ELEMENTS TO REMAIN
ELEMENTS TO BE DEMOLISHED
1 HATCH INDICATES FOR REFERENCE ONLY

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REGISTERED ARCHITECT
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NORTH MAIN RETAIL
201 NORTH MAIN STREET, HUDSON, OHIO

N

PROJECT #: 2317

ISSUE	ID	DATE
BID PACKAGE #1		08/23/23
BULLETIN #1	1	11/22/23

DEMOLITION PLANS AND ELEVATIONS

AD1.01

UNA-CLAD™ Metal Roofing Systems

VERSATILE • SUSTAINABLE • BEAUTIFUL

Firestone BUILDING PRODUCTS NOBODY COVERS YOU BETTER.™

UNA-CLAD™ METAL ROOFING SYSTEMS

Since the introduction of UNA-CLAD Metal Roofing Systems more than 30 years ago, they have become one of the leading architectural metal roofing products in the construction industry. Firestone Building Products systems are recognized the world over for their outstanding construction and innovative design.

UNA-CLAD offers a variety of sustainable, reliable and environmentally-friendly roofing system options. UNA-CLAD allows for maximum design freedom with a wide range of materials, widths and finishes.

To further protect your investment, install the roofing system over our innovative new CLAD-GARD® roofing underlayment. This easy-to-handle skid-resistant material is the ideal moisture barrier to protect your roofing investment from corrosion and other environmental concerns.

And once your roof is installed, the unparalleled Red Shield® Warranty assures that your roof will provide outstanding performance for decades to come.

Thank you for your interest in Firestone Building Products UNA-CLAD roofing systems. For detailed information about anything covered in this brochure, please contact your local Firestone Representative or visit www.firestonebpco.com.

UNA-CLAD™ METAL ROOFING SYSTEMS OVERVIEW

RED SHIELD® WARRANTY SYSTEMS
Durable metal roofing systems that allow maximum versatility with the extra assurance of the Firestone Red Shield Warranty—the strongest protection available today.
• UC-3 • UC-4 • UC-6 • UC-14

ARCHITECTURAL SYSTEMS
Allow maximum design freedom to create roofs with distinct lines, radii and features while providing outstanding wind-load performance and aesthetics.
• UC-7

SOFFIT SYSTEMS
Superior versatility, security and weatherability in systems that allow great installation flexibility, including either vertical or horizontal applications.
• UC-500 • UC-501 • UC-750

PROFILED PANEL SYSTEMS
Strong, economical and low profile panels designed for a variety of Profiled Panel Systems, environments and applications.
• UR-PRO OMEGA • 5V-CRIMP
• HR-ULTRA OMEGA • UC-600
• VR-CLASSIC OMEGA • UC-601

ROOFING PANEL PROFILES

Striated Profile
Flat Rib Profile
Pencil Rib Profile

To help prevent distortion (called "oil-canning") that can occur on metal roofs, Firestone Building Products offers a variety of stiffening rib profile options. Striations, Flat Ribs and Pencil Ribs are subtly raised sections that run parallel to panel seams and add rigidity and visual interest. Not all profiles are available on all panels, please see each product description for details.

FIRESTONE RED SHIELD WARRANTY

5-20

When we say "Nobody Covers You Better" we mean it. Red Shield Warranties provide industry-leading coverages from 5 to 20 years for most roofing systems, and can even be transferred from owner to owner. No competitor can match our 100 year+ heritage of innovation and customer support. When you have a Firestone roof and Red Shield Warranty over your head, you can be confident of outstanding roofing performance for decades to come.*

* Subject to certain terms and conditions. 20 year warranty coverage may be available with the UC-4 system.

RED SHIELD™ WARRANTY SYSTEMS

RED SHIELD STANDING SEAM ROOFING SYSTEMS COMPRISE A SYSTEM OF INTERLOCKING METAL PANELS THAT GENERALLY RUN VERTICALLY FROM THE ROOF RIDGE TO THE EAVES. THE SEAM WHERE THE TWO PANELS JOIN TOGETHER IS RAISED ABOVE THE SURFACE OF THE PANEL, THEREBY GIVING THE PRODUCT ITS NAME.

THESE METAL ROOFS FREELY EXPAND AND CONTRACT WITH THE WEATHER WITHOUT DAMAGE TO THE SUBSTRATE, PROVIDE EXCEPTIONAL RAIN AND SNOW RUNOFF, PERFORM WELL IN HIGH-WIND ENVIRONMENTS, MINIMIZE OIL-CANNING AND PROVIDE GREAT DESIGN VERSATILITY.

FIRESTONE UNA-CLAD™ RED SHIELD SERIES
Building on proven standing seam technology, Firestone developed the **UNA-CLAD Red Shield Series** to provide designers, architects and building owners with a wide selection of beautiful, durable and functional metal roofs. The Red Shield Series is available in a variety of colors, materials, widths, textures, profiles and seam designs.

UNA-CLAD UC-3
This double-lock standing seam system uses mechanical sealing for a durable, virtually leak proof seal. UC-3 roofs create a traditional look, and allow specified radius profiles to enhance the architect's design.
Standard UC-3 materials are painted steel; painted and anodized aluminum; architectural grade sheet copper and zinc. Available with embossing, striations and ribs (flat and pencil) to minimize oil-canning.

UNA-CLAD UC-4
The patented seam design snap locks securely and fastens to a solid substrate without the use of clips. This provides superior leak resistance and can reduce labor by up to 50%.
Standard UC-4 materials are painted steel; painted and anodized aluminum and architectural grade sheet copper. Available with embossing, striations and ribs (flat and pencil) to minimize oil-canning.

PROTECT YOUR INVESTMENT

5-20 Red Shield Warranty Protection

UNA-CLAD UC-6
The double-lock standing seam system utilizes proven Pittsburg Locking in conjunction with a floating concealed-clip assembly. This design allows for free expansion and contraction, and outstanding wind uplift resistance.
Standard UC-6 materials are painted steel; painted and anodized aluminum; architectural grade sheet copper and zinc. Available with embossing, striations and ribs (flat and pencil) to minimize oil-canning.

UNA-CLAD UC-14
The concealed-clip snap-lock seam system eliminates the need for mechanical seaming or separate battens, accommodating very long panels while maintaining free expansion and contraction capability.
Standard UC-14 materials are painted steel; painted and anodized aluminum and architectural grade sheet copper. Available with embossing, striations and ribs (flat and pencil) to minimize oil-canning.

ARCHITECTURAL METAL ROOFING

BATTENS ARE DISTINCTIVE ARCHITECTURAL FEATURES. PANEL SYSTEMS WITH BATTENS OR CAPS ALLOW FOR THE PANELS TO BE INSTALLED IN NON-SEQUENTIAL ORDER, THUS ACCOMMODATING COMPLEX ROOF DESIGNS. THESE PANELS CAN BE COUNTED ON TO ALLOW FREE EXPANSION AND CONTRACTION IN CHANGING WEATHER CONDITIONS, AS WELL AS PROVIDE EXCEPTIONAL RAIN/ SNOW RUNOFF AND GOOD WIND UPLIFT PERFORMANCE.

THESE PANELS AND BATTENS ARE ROLL-FORMED AND PROCESSED WITH STATE-OF-THE-ART TECHNOLOGY TO ENSURE UNPARALLELED QUALITY AND SERVICE FROM COIL TO FINISHED PRODUCT.

FIRESTONE UNA-CLAD™ ARCHITECTURAL SERIES
Firestone developed the **UNA-CLAD Architectural Series** to allow designers, architects and building owners to utilize the unique profile a batten roof delivers. In addition to their beautiful appearance, these metal roofing systems are reliable and durable. The Architectural Series is available in a variety of colors, batten widths, materials and panel widths.

UC-7 SNAP-ON ARCHITECTURAL SERIES
This concealed clip fastening system is capped with a narrow, low-profile snap-on batten that provides a thin-line appearance while providing the designer freedom to specify radius or high profile features. UC-7 delivers good thermal movement.
Standard UC-7 materials are painted steel; painted and anodized aluminum; architectural grade sheet copper and zinc. Available in heavier gauges, narrower widths, embossing and ribs (flat and pencil) to minimize oil-canning.

METAL SOFFIT SYSTEMS

SOFFITS SERVE MANY PURPOSES IN COMMERCIAL ARCHITECTURE, INCLUDING WEATHER-SHELTERING OVERHANGS, BUILDING VENTILATION PORTS, CONVENIENT CONCEALMENT SECTIONS FOR HVAC OR ELECTRICAL SYSTEMS AND OTHER APPLICATIONS. THE RIGHT SOFFIT AND FASCIA TREATMENT CAN PROVIDE A FINISHED LOOK TO THE STRUCTURE, AS WELL AS AID WATER RUNOFF, BUILDING VENTILATION AND COOLING.

METAL SOFFIT SYSTEMS PROVIDE A STRONG, DURABLE AND RELIABLE MATERIAL THAT ENHANCES THE LONGEVITY OF THE SOFFIT AND CREATES AN AESTHETICALLY-PLEASING APPEARANCE.

FIRESTONE UNA-CLAD™ METAL SOFFIT SYSTEMS
Designers, architects and building owners can create a comprehensive, finished look to their buildings with Firestone **UNA-CLAD Metal Soffit Systems**. The wide selection of metal soffit and fascia products allows for an uninterrupted and consistent appearance. The Metal Soffit Systems are available in a variety of colors, materials, widths, textures, profiles and seam designs to visually integrate with Firestone metal roofing products.

UC-500 FLUSH PANEL
This interlocking, concealed fastening metal panel system is engineered for both soffit and fascia applications. UC-500 delivers a flat or flush look that provides a neat, finished appearance.
Standard UC-500 materials are painted steel; painted and anodized aluminum; architectural grade sheet copper and zinc. Available in heavier gauges, narrower widths, embossing and pencil ribs to minimize oil-canning. Venting available for soffit applications.

UC-501 REVEAL FLUSH PANEL
Similar in characteristics to UC-500, the Reveal Flush Panel system creates a flat appearance with a bold channels (reveals) that provide visual interest to the soffit.
Standard UC-501 materials are painted steel; painted and anodized aluminum; architectural grade sheet copper and zinc. Available in heavier gauges, narrower widths, embossing and pencil ribs to minimize oil-canning. Venting available for soffit applications.

UC-750 V-GROOVE SOFFIT PANEL
This interlocking architectural aluminum panel is engineered specifically for soffit applications. UC-750 utilizes a concealed fastening system with interlocking leg. Subtle V-grooves run the length of the panel providing a clean, traditional appearance.
Standard UC-750 material is painted and anodized aluminum. Full- or half-vent options available.

PROFILED METAL ROOFING

FOR CENTURIES, LARGE STRUCTURES SUCH AS CATHEDRALS HAVE RELIED ON THE INHERENT DURABILITY OF METAL ROOFING. IN MODERN USAGE, CORRUGATED METAL ROOFING PANELS HAVE TWICE THE AVERAGE LIFESPAN OF ASPHALT ROOFING, AND PROVIDE EXCEPTIONAL RAIN AND SNOW SHEDDING ABILITIES.

DESIRABLE BECAUSE OF THEIR DESIGN VERSATILITY, LOW MAINTENANCE REQUIREMENTS AND HIGH STRENGTH-TO-WEIGHT RATIO, ARCHITECTS, DESIGNERS AND BUILDING OWNERS ARE TURNING TO METAL ROOFS FOR NEW CONSTRUCTION AND RENOVATION.

FIRESTONE UNA-CLAD™ PROFILED PANEL SERIES
To answer the need for a durable and economical metal roofing solution, Firestone created the **UNA-CLAD Profiled Panel Series**. Commercial-grade metal roof and wall panels provide a traditional metal panel appearance with bold recurring or successive ribs. Architects, designers and building owners can choose from a variety of materials and rib patterns to best accentuate their structure.

UR-Pro HR-Ultra VR-Classic 5-V-Crimp UC-600 UC-601

OMEGA SERIES UR-PRO OMEGA, HR-ULTRA OMEGA, VR-CLASSIC OMEGA
The Omega Series is factory-formed corrugated metal roofing with exposed mechanical fasteners. Panels have overlapping seams with bold ribs for an appealing accent to any architectural design. Can be installed in non-sequential patterns. Each Omega Series product has identical physical characteristics but with a unique rib pattern to provide greater design versatility.
Standard Omega Series materials are painted steel; painted aluminum and architectural grade sheet copper.Omega Series panels are suitable for wall cladding.

5-V-CRIMP PROFILED PANELS
Like Omega Series panels, 5-V-Crimp panels are factory-formed corrugated metal roofing with exposed mechanical fasteners. Panels have overlapping seams with V-shaped ribs to add a unique "old world" styling to your roof profile.
Standard 5-V-Crimp materials are painted steel; painted aluminum and architectural grade sheet copper. 5-V-Crimp panels are suitable for wall cladding.

UC-600 & UC-601 CORRUGATED PANELS
Like Omega Series panels, UC-600 and UC-601 panels are factory-formed corrugated metal roofing with exposed mechanical fasteners. Panels have overlapping seams with gently undulating striations to give you the option of a softer roof profile.
Standard UC-600 materials are painted steel; painted aluminum; architectural grade sheet copper and zinc. Standard UC-601 materials are painted steel and zinc. UC-600/UC-601 panels are suitable for wall cladding.

CLAD-GARD™ ROOFING UNDERLAYMENT

CLAD-GARD Roofing Underlayment is easy to install and provides the ideal moisture barrier between the roof deck and the adjacent metal roofing system. It features a skid-resistant surface; is watertight around fasteners such as nails or other anchors; offers great high-temperature resistance; and is unaffected by water so it can be left exposed for up to 3 months before applying the metal roofing system.

CLAD-GARD SA Underlayment
Self-adhered underlayment for quick, simple installation.
– Premium non-reinforced product that minimizes wrinkling
– Available in a convenient 2-square roll 3' x 67' (.09 m x 20 m)
– Heavy-duty adhesive is protected by a split-release plastic liner, which is easily removed for quick installation and placement of the underlayment
– Two available formulations: CLAD-GARD SA-S (heat resistant up to 250°F [121°C]) and CLAD-GARD SA-N (heat resistant up to 230°F [110°C])

CLAD-GARD R Underlayment
Self-adhered underlayment at an economical price.
– Split-release backer for easy installation
– Comes unboxed which means less waste on the jobsite.
– Available in a 2-square roll (3' x 66.7') (.09 m x 20 m).
– Can be applied directly to Firestone HailGard® Composite, OSB and plywood substrates.
– Eligible for up to a 20-year Red Shield® Warranty

CLAD-GARD MA Underlayment
Mechanically-attached choice for reliable results.
– Available in 10-square rolls that are 4' (1.2 m) wide for more square feet per roll and fewer seams
– Lighter than roofing felt at only 31 lb (14 kg) per roll
– Lays flat and stays wrinkle-free, meaning fewer nails are needed to fasten compared to felt underlayment
– Heat resistant up to 230°F (110°C) without degradation of the material
– Heat resistant up to 230°F (110°C) without degradation of the material

A VALUABLE ADDITION TO A SUSTAINABLE BUILDING PROJECT.

COLOR SELECTION GUIDE

STONE WHITE	BONE WHITE	ALMOND	SANDSTONE
SLATE GRAY	CITYSCAPE	CHARCOAL GRAY	SIERRA TAN
MEDIUM BRONZE	DARK BRONZE	EXTRA DARK BRONZE	MATTE BLACK
BRANDYWINE	COLONIAL RED	TERRA COTTA	MANSARD BROWN
REGAL RED	AWARD BLUE	SKY BLUE	ELECTRIC BLUE
REGAL BLUE	TEAL	PATINA GREEN	DARK IVY
SHERWOOD GREEN	HARTFORD GREEN	HEMLOCK GREEN	TROPICAL PATINA
SILVER METALLIC	CLASSIC COPPER	CHAMPAGNE METALLIC	

Colors shown are as close to actual colors as allowed by the printing process. Your local sales rep can provide actual metal samples and answer questions about custom colors and other special requests.

Firestone BUILDING PRODUCTS NOBODY COVERS YOU BETTER.™

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0614145 | 12/31/23

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NORTH MAIN RETAIL
201 NORTH MAIN STREET, HUDSON, OHIO

N

PROJECT #: 2317

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BID PACKAGE #1		08/23/23

SPECIFICATIONS

A0.01

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insite.

DECEMBER 2019
JamesHardie

HardiePlank® Lap Siding — Submittal Form 01

Submitted to: ....
Project Name: ....
Submitted by: ....
Date: ....

☐ HZ5® Product Zone ☐ HZ10® Product Zone
Product Width: ☐ 5-1/4in ☐ 6-1/4in ☐ 7-1/4in ☐ 8in ☐ 8-1/4in ☐ 9-1/4in ☐ 12in
Product Finish: ☐ Primed ☐ ColorPlus® Technology
Product Texture: ☐ Smooth ☐ Select Cedarmill® ☐ Colonial Roughsawn® ☐ Colonial Smooth® ☐ Rustic Cedar

HardiePlank® Lap Siding — Specification Sheet 01

DIVISION: 07 00 00 THERMAL AND MOISTURE PROTECTION SECTION: 07 46 46 FIBRE CEMENT SIDING

HARDIEPLANK® LAP SIDING

Manufacturer
James Hardie Building Products, Inc.
The products are manufactured at the following locations, with quality control inspections by ICC-ES:
- Cleburne, Texas
- Plant City, Florida
- Reno, Nevada
- Waxahachie, Texas
- Peru, Illinois
- Pulaski, Virginia
- Tacoma, Washington
- Fontana, California

Compliance with the following codes
- 2012, 2009 and 2006 International Building Code® (IBC)
- 2012, 2009 and 2006 International Residential Code® (IRC)

Features
- Noncombustible
- Dimensionally Stable
- Resists damage from pests
- Weather Resistant-Engineered for Climate®
- Impact resistant
- Sustainable

Use

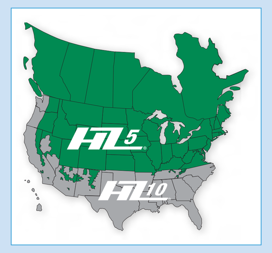
Description

Available Sizes

Product	Width (in)	Length	Thickness (in)
HardiePlank lap siding	5-1/4, 6-1/4, 7-1/4, 8, 8-1/4, 9-1/4, 12	12 feet	5/16

Texture & Finish

Engineered for Climate®



SPECIFICATION SHEET 01 DECEMBER 2019

Performance Properties

	General Property	Test Method	Unit or Characteristic	Requirement	Result
PHYSICAL ATTRIBUTES	Dimensional Tolerances	ASTM C1185	Length, Width, Thickness, Squareness, Edge Straightness	± 0.5% or ± 1/4 in; ± 0.5% or ± 1/4 in; ± 0.04 in; Δ in diagonals ≤ 1/32 in/ft of sheet length; ≤ 1/32 in/ft of length	Pass
	Density, lb/ft³	ASTM C1185		As reported	83
	Water Absorption, % by mass	ASTM C1185		As reported	36
	Water Tightness	ASTM C1185	Physical Observations	No drop formation	Pass
	Flexural Strength	ASTM C1185	Wet conditioned, psi; Equilibrium conditioned, psi	≥1015 psi; ≥1450 psi	Pass
THERMAL	Thermal Conductivity; Actual Thermal Conductivity; Thermal Resistance; Actual Thermal Resistance	ASTM C177		As reported	2.07; 6.62; 0.48; 0.15
DURABILITY	Warm Water Resistance	ASTM C1185	Physical Observations	No visible cracks or structural alteration	Pass
	Heat/Rain Resistance	ASTM C1185	Physical Observations	No visible cracks or structural alteration	Pass
	Freeze/Thaw Resistance	ASTM C1185	Physical Observations; Mass Loss, %; Freeze/Thaw, % strength retention	No visible cracks or structural alteration; ≤ 3.0%; ≥ 80%	Pass
	UV Accelerated Weathering Test	ASTM G23	Physical Observations	No cracking, checking, or crazing	Pass
FIRE CHARACTERISTICS	Surface Burning Characteristics	ASTM E84	Flame Spread Index (FSI); Smoke Developed Index (SDI); Fuel Contributed; NFPA Class; Uniform Building Code Class; International Building Code® class	As reported	0; ≤ 5; 0; A; 1; A
	Noncombustibility	ASTM E136	Noncombustible	Pass/fail	Pass
	Fire Resistance Rated Construction	ASTM E119	Fire Resistance Rating	1-hour	Note 1

Note 1: listed on Warnock Hersey and ESR 2290

Installation
Install HardiePlank lap siding in accordance with:
- HardiePlank lap siding installation instructions
- ICC-ES ESR 2290
- Requirements of authorities having jurisdiction

Sustainable Design Contribution
- Regionally sourced content - varies by project location
- Avoidance of certain chemicals or Red List Compliance

Detailed product information for LEED projects, or other state or regional sustainability programs is available through James Hardie Technical Services.

Warranty
HardiePlank lap siding: 30-year, Non-Prorated, Limited Warranty
ColorPlus Technology: 15-year Limited Finish Warranty

Storage and Handling
Store flat and keep dry and covered prior to installation.

Technical Services
Contact James Hardie Technical Services online at JamesHardie.com, or by phone at (800)426-4051

JamesHardie
Additional Installation Information, Warranties, and Warning are available at JamesHardie.com
1 866 442 7343 | www.jameshardie.com

Pella® Reserve™
Traditional Wood & Clad/Wood

Exquisitely designed windows and doors with unparalleled historical detailing.

Double-Hung Interior

Double-Hung Exterior

- **Historical details**
Our most historically authentic line of wood windows and patio doors. Featuring through-stile construction, deliberate proportions and intricate profiles. Pella Reserve – Traditional products are the ideal choice for historical renovations and traditional building projects.
- **Authentic hardware**
Complement your project with historically authentic spoon-lock window hardware. Our Antiek casement window hardware is inspired by period furniture to deliver authentic traditional style.
- **Architectural interest**
Featuring the industry's only foam spacer solution, Pella's Integral Light Technology® grille helps capture the look of true-divided-light without sacrificing energy performance. Further your aesthetic with the putty profile, recreated with historically accurate angles – providing meaningful depth and a realistic shadow. Pella Reserve products offer the industry's deepest sash dimension.
- **Virtually unlimited customization**
If you can dream it, we can build it with our most customizable product line. From extra tall to extra wide, Pella can craft unique windows that complement your aesthetic. Custom sizes, grille patterns and designs, finishes, wood types and glass options are available.
- **Tailor-made solutions**
From preliminary drawings to installation, Pella's expert team of architects, engineers, drafters and consultants can work to deliver custom window and door solutions for your project. Partner with Pella to achieve your unique vision without concessions.
- **Intentional innovation**
Winner of the 2019 Most Innovative Window from Window and Door Magazine, the Integrated Rolscreen® retractable screen preserves aesthetics and the view. It is a double- and single-hung screen that appears when you open the window, and rolls away, out of sight, when you close it.
- **Durable interiors and extruded aluminum exteriors**
To help save you time on the jobsite, interior finish options are available in a variety of paints and stains, or primed and ready-to-paint. To complement your exterior aesthetic, choose from our carefully curated color palette or define your own custom color for your project.
- **ENERGY STAR® certified[1]**
Pella wood products offer energy-efficient options that will meet or exceed ENERGY STAR guidelines in all 50 states.
- **Testing beyond requirements**
At Pella, our products are tested beyond requirements to help ensure they have long-lasting performance and reduce call-backs for you.
- **Best limited lifetime warranty[2]**
Pella Reserve products are covered by the best limited lifetime warranty in the business for wood windows and patio doors.[2]

Available in these window and patio door styles:

Special shape windows also available.

[1,2] See back cover for disclosures.

Product Specifications

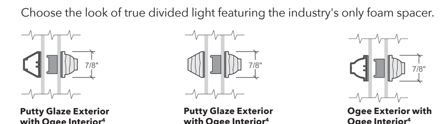
Window & Patio Door Styles	Min. Width	Min. Height	Max. Width	Max. Height	Performance Class & Grade	U-Factor	SHGC	STC	Frame / Install
Awning	13-¾"	13-¾"	59"	59"	LC40-CW50	0.25-0.29	0.18-0.47	27-35	Fold-out Fin, Block Frame, EnduraClad Exterior Trim / Brickmould
Precision Fit Awning	17"	17"	53"	29"	R45-CW50	0.28-0.32	0.18-0.47	27-30	Pocket Replacement
Casement	13-¾"	13-¾"	47"	108"	R35-CW50	0.25-0.29	0.18-0.47	27-34	Fold-out Fin, Block Frame, EnduraClad Exterior Trim / Brickmould
Precision Fit Casement	17"	17"	35"	73"	R35-CW50	0.28-0.33	0.18-0.47	27-30	Pocket Replacement
Fixed Casement	10"	10"	144"	144"	R35-CW50	0.25-0.29	0.18-0.47	27-35	Fold-out Fin, Block Frame, EnduraClad Exterior Trim / Brickmould
Precision Fit Fixed Casement	17"	17"	59"	73"	R45-CW50	0.28-0.33	0.18-0.47	27-30	Pocket Replacement
Double-Hung	14"	24-⅜"	48"	96"	CW30-CW50	0.25-0.30	0.19-0.53	28-35	Fold-out Fin, Block Frame, EnduraClad Exterior Trim / Brickmould
Precision Fit Double-Hung	13-½"	23-¾"	48"	84"	CW40-CW50	0.25-0.31	0.19-0.53	26-30	Pocket Replacement
Monumental Hung	13-¾"	24"	72"	144"	LC25-CW50	0.25-0.30	0.17-0.47	29-34	Fold-out Fin, Block Frame, EnduraClad Exterior Trim / Brickmould
In-Swing Hinged Patio Door (Single)	18"	36"	48"	199-½"	LC40-LC55	0.25-0.29	0.14-0.40	31-35	
In-Swing Hinged Patio Door (Double)	36"	36"	96"	119-½"	LC40-LC55	0.25-0.29	0.14-0.40	31-35	
Out-Swing Hinged Patio Door (Single)	18"	36"	48"	119-½"	R50-LC70	0.25-0.30	0.14-0.39	30-36	
Out-Swing Hinged Patio Door (Double)	36"	36"	96"	119-½"	R50-LC70	0.25-0.30	0.14-0.39	30-36	
Sliding Patio Door (O)	30-¾"	74"	60-¾"	119-½"	LC40-LC70	0.29-0.32	0.15-0.42	–	
Sliding Patio Door (OX, XO)	59-¼"	74"	119-½"	119-½"	LC35-LC65	0.29-0.32	0.15-0.42	29-35	
Sliding Patio Door (OXO)	90"	74"	180"	119-½"	LC30-LC45	0.29-0.32	0.15-0.42	–	
Sliding Patio Door (OXXO)	116-⅛"	74"	236-⅛"	119-½"	LC25-LC40	0.29-0.32	0.15-0.42	–	
Multi-Slide Patio Door	40-¼"	50-½"	701-⅝"	119-½"	R15-LC25[3]	0.30 – 0.36	0.15 – 0.46	31	For more info visit PellaADM.com
Bifold Patio Door	31-¾"	55-½"	312"	119-½"	R15-LC25[3]	0.26-0.44	0.13-0.45	–	

U-Factor, SHGC and STC are grouped under "Performance Values". The Frame / Install entry for Monumental Hung through Sliding Patio Door (OXXO) is one cell spanning those rows, and the Multi-Slide and Bifold rows share one cell.

Window sizes available in 1/8" increments
Special sizes available. For more information regarding performance, visit pella.com/performance. For more information regarding frame and installation types, visit PellaADM.com.

Grilles

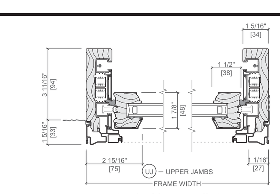
Integral Light Technology®
Choose the look of true divided light featuring the industry's only foam spacer.



Putty Glaze Exterior with Ogee Interior[4] 7/8", 1-1/4" or 2"
Putty Glaze Exterior with Ogee Interior[4] 7/8", 1-1/4" or 2"
Ogee Exterior with Ogee Interior[4] 7/8", 1-1/4" or 2"

Cross Sections

Cross Sections



Optional Fold-out Installation Fin

The double-hung cross sections provide visual reference to the historic putty exterior profile and traditional, beveled Ogee interior that add architectural interest to your project.

[3,4] See back cover for disclosures.

Window Hardware

Classic Collection — Get a timeless look with authentic styles in classic finishes.
Fold-away Crank (Antiek); Spoon-Style Lock
Finishes: Champagne, White, Brown, Matte Black, Oil-Rubbed Bronze, Satin Nickel, Satin Brass

Rustic Collection — Create a distinct and charming look with distressed finishes.
Fold-away Crank (Antiek); Spoon-Style Lock
Finishes: Distressed Bronze, Distressed Nickel

Essential Collection — Select from popular designs and finishes to suit every style.
Fold-away Crank; Cam-Action Lock
Finishes: Champagne, White, Brown, Matte Black, Oil-Rubbed Bronze, Satin Nickel, Satin Brass

Patio Door Hardware

Classic Collection — BALDWIN — Choose timeless pieces, created in collaboration with Baldwin® Hardware, for a look that will never go out of style.
Hinged & Bifold Patio Door Handle (Virago); Sliding & Multi-Slide Patio Door Handle (Ambrose); Multi-Slide Patio Door Handle[5,6]
Finishes: Matte Black, Oil-Rubbed Bronze, Satin Nickel, Satin Brass

Essential Collection — Elevate your style and transform a home with elegant selections.
Hinged & Bifold Patio Door Handle (Standard); Sliding Patio Door Handle (Standard); Multi-Slide Patio Door Handle[5,6]
Finishes: Champagne, White, Brown, Matte Black, Oil-Rubbed Bronze, Satin Nickel, Satin Brass

Additional hardware collections available. Visit PellaADM.com for more information.

[5,6] See back cover for disclosures.

Colors

Wood Types — Choose the wood species that best complements your project's interior.
Pine; Custom solutions: Douglas Fir, Mahogany, White Oak, Red Oak, Cherry, Maple

Prefinished Pine Interior Colors — Custom interior finishes, unfinished or primed and ready-to-paint are also available.
White, Bright White, Linen White, Natural Stain, Golden Oak Stain, Early American Stain, Provincial Stain, Dark Mahogany Stain, Red Mahogany Stain, Espresso Stain, Charcoal Stain, Black Stain

Extruded Aluminum-Clad Exterior Colors — Our low-maintenance EnduraClad® exterior finish resists fading. Take durability one step further with EnduraClad Plus which also resists chalking and corrosion.[7]
Black, White, Brown, Fossil, Iron Ore, Portobello, Putty, Almond, Classic White, Brick Red, Hartford Green, Pearl Gray, Soft Linen, Satin Steel, Matte Gray, Wolf Gray, Spice Red, Sage, Frost Blue, Blue Ash

Custom colors are also available.

Added Peace of Mind

Integrated Security Sensors — Integrated wireless security sensors maintain aesthetics, streamline security installation and ensure no warranty loss is caused by post-installation drilling. Sensors can be monitored via the free Pella® Insynctive® App and are compatible with major security panel systems.[8] For more information, go to connectpella.com.

Pella LIMITED LIFETIME WARRANTY
The Best Limited Lifetime Warranty in the Industry
We know your reputation matters and you stake your reputation on quality, dependable products. That's why we have the best limited lifetime warranty in the industry for wood windows and patio doors.[2]

1. Some Pella products may not meet ENERGY STAR® guidelines in Canada. For more information, contact your local Pella sales representative or go to energystar.gc.ca.
2. Based on comparing written limited warranties of leading national wood window and wood patio door brands. See written limited warranty for details, including exceptions and limitations, at pella.com/warranty or contact Pella Customer Service.
3. Ratings are contingent on product configurations.
4. Color-matched to your product's interior and exterior color.
5. Flush multi-slide handle is a Pella exclusive design.
6. Flush multi-slide handle is not available in Champagne.
7. EnduraClad Plus protective finish is not available with all colors. See your local Pella sales representative for availability.
8. Requires the Insynctive App on a smart device, an Insynctive Bridge and a wireless home router with internet connection.

NORTH MAIN RETAIL
201 NORTH MAIN STREET, HUDSON, OHIO

N

PROJECT #: 2317

ISSUE	ID	DATE
BID PACKAGE #1		08/23/23

SPECIFICATIONS

A0.02

TABLE 1. Shrinkage of Western Red Cedar

Direction of Shrinkage	Shrinkage in Percentage: From green (25% or greater moisture content) to: 15%	12%	6%	From kiln dried (15% average moisture content) to: 15%	12%	6%
Radial	0.95	1.2	1.9	0	0.3	1.0
Tangential	2.0	2.6	3.8	0	0.7	2.1

Notes:

1. Radial shrinkage applies to the width of vertical grain lumber, tangential to the width of flat grain lumber.
2. Shrinkage does not begin until the fiber saturation point is reached.
3. 15% is the average equilibrium moisture content of wood during the summer in the Pacific Northwest.
4. 12% is the average equilibrium moisture content in most areas of the US.
5. 6% is the average equilibrium moisture content for interiors of heated buildings.

TABLE 2. Base design values for use in the USA for Western Red Cedar. Visually graded (WCLIB, WWPA) 2–4 in. thick x 2 in. & wider.

Grade	Extreme Fiber Stress in bending Fb	Tension Parallel Parallel to Grain Ft	Horizontal Shear Fv	Compression: Perpendicular to Grain Fc	Compression: Parallel to Grain Fc	Modulus of Elasticity (million psi) E
Select Structural No. 1/No. 2 No. 3	1,000 725/700 400	600 425/425 250	155 155 155	425 425 425	1,000 825/650 375	1.1 1.0/1.0 0.9
Construction Standard Utility	800 450 225	475 275 125	155 155 155	425 425 425	850 650 425	1.0 0.9 0.8
Stud	550	325	155	425	400	0.9

TABLE 3. Sizes of rough Western Red Cedar

Thickness (in.)	1	1-3/4	2	3	4	6	8	10	12	14
Width (in.)	2	4	6	8	10	12	14			

TABLE 4. Comparative Softwood Densities (pcf)

Species	Oven-Dry Density
Western Red Cedar	21
Douglas Fir	31
Ponderosa Pine	27
Southern Pine	34

TABLE 5. Size differential between unseasoned & dry lumber

Nominal Dimension (in.)	Size Differential Between Unseasoned and Dry Lumber After Surfacing (in.)
1-1/2 or less	1/32
2 to 4	1/16
5 to 7	1/8
8 or more	¼

TABLE 6. Wet Use Factors (CM) for tabulated design values

The recommended design values are for applications where the moisture content of the wood does not exceed 19%. For use conditions where the moisture content of dimension lumber will exceed 19%, the Wet Use Adjustment Factors below are recommended.

	Property	Adjustment Factor
Fb	Extreme Fiber Stress in Bending	0.85*
Ft	Tension Parallel to Grain	1.0
Fc	Compression Parallel to Grain	0.8**
Fv	Horizontal Shear	0.97
Fc	Compression Perpendicular to Grain	0.67
E	Modulus of Elasticity	0.9

** Bending Wet Use Factor = 1.0 where Fb x CF (Base Value x Size Factor) does not exceed 1,150 psi.*

*** Compression Parallel Wet Use Factor = 1.0 where Fc x CF (Base Value x Size Factor does not exceed 750 psi.*

Ijsselstein Train Station, Netherlands

Architect:
Hans Goverde - Kraaijvanger Urbis

Selected for its beauty, performance and sustainability, Western Red Cedar is featured as cladding on this high profile commercial project in the Netherlands. It beautifully complements the modern design and offers a warm contrast to the "coolness" of the glass, steel and concrete. ◆

TABLE 7. Western Red Cedar Products

Product	Thickness in. (nominal)	Width in. (nominal)	Grade	Grading Rule Paragraph: NLGA	WWPA	WCLIB
Bevel Siding Clear	1/2, 3/4	4, 6, 8, 10	Clear V.G. Heart A Rustic B C Proprietary	201a 201b 201d 201c 201e N/A	21.11 21.12 N/A 21.13 21.14 N/A	106-aa 106-a N/A 106-b 106-c N/A
Knotty	5/8, 11/16, 3/4, 7/8	6, 8, 10, 12	Select Knotty Quality Knotty Proprietary	205a 205b N/A	N/A N/A N/A	111-e 111-f N/A
Knotty-Rabbetted	3/4, 5/4	6, 8, 10	Select Knotty Quality Knotty	205a 205b	N/A N/A	111-e 111-f
Knotty-Wavy Edged	7/8	10, 12	Select Knotty Quality Knotty	205a 205b	N/A N/A	111-e 111-f
Pattern Siding Clear	1	4, 6, 8	Clear Heart A B	200a 200b 200c	20.11 20.12 20.13	102-b 102-c 102-d
Knotty	1	4, 6, 8	Select Knotty Quality Knotty Standard and Better Proprietary	204a 204b 3 114-a, b, c N/A	N/A N/A N/A N/A	111-e 111-f 118-a, b, c N/A
Clear Finish	1, 5/4, 2	2, 3, 4, 6, 8, 10, 12	Clear Heart A B	200a 200b 200c	20.11 20.12 20.13	102-b 102-c 102-d
Boards	1	2, 3, 4, 6, 8 10, 12	Select Knotty Quality Knotty Standard and Better	204a 204b 114-a, b, c	N/A N/A 30.50 (118-a, b, c)	111-e 111-f 118-a, b, c
Fence Boards	1	6, 8, 10	Select Knotty Quality Knotty Rustic	210a 210b N/A	N/A N/A N/A	117-a 117-b 117-c
Roof Decking	2, 3, 4	6, 8	Select Commercial	127b 127c	55.11 55.12	127-b 127-c
Outdoor Decking Clear	5/4, 2	4, 6	Architect Clear Architect Knotty	N/A N/A	N/A N/A	N/A N/A
Knotty	5/4, 2	4, 6	Architect Knotty Custom Knotty	N/A N/A	N/A N/A	N/A N/A
Paneling Clear	1	4, 6, 8	Clear Heart A B	200a 200b 200c	20.11 20.12 20.13	102-b 102-c 102-d
Paneling Knotty	1	4, 6, 8	Select Knotty Quality Knotty	204a 204b	N/A N/A	111-e 111-f
Joists & Planks	2, 3, 4 2, 3, 4	2, 3, 4 6, 8, 10, 12	Appearance Standard and Better Appearance No. 2 and Better	N/A 122-b, c N/A 124-a, b, c	N/A 40.11, 40.12 N/A 62.10, 62.11, 62.12	N/A 122-b, c N/A 123-a, b, c
Timbers	5, 6, 8, 10, 12	5, 6, 8, 10 12 and wider	Appearance Standard and Better	N/A 131-a, b, c	N/A 80.10, 80.11, 80.12	N/A 131-a, b, c
Beams & Stringers	5, 6, 8, 10, 12	5, 6, 8, 10, 12	No. 2 and Better Width more than 2" greater than thickness	130-a, b, cc	70.10, 70.11, 70.12	130-a, b, cc

TABLE 8. Flat Use Factors (Cfu)

The recommended design values are for applications where the moisture content of the wood does not exceed 19%. For use conditions where the moisture content of dimension lumber will exceed 19%, the Wet Use Adjustment Factors below are recommended.

Nominal Width (in.)	Nominal Thickness (in.): Less than 4	4
Less than 4	1.00	—
4	1.10	1.00
5	1.10	1.05
6	1.15	1.05
8	1.15	1.05
10 & wider	1.20	1.10

Note:

These factors apply to all dimension lumber except tongue and groove decking grades. For T&G decking, the following adjustments may be used.

TABLE 9. Repetitive Member Factor (Cr)

Applies to Tabulated Design Values for Extreme Fiber Stress in Bending when members are used as joists, truss chords, rafters, studs, planks, decking or similar members which are in contact or spaced not more than 24_ on centers, are not less than 3 in number and are joined by floor, roof or other load distributing elements adequate to support the design load.

1.5

Lake Washington Residence, USA

Architect:
The Miller Hull Partnership

This 4000 sf residence and carport is located on a steep wooded site on Lake Washington. Entered from above via a bridge, this residence transitions between a steep forest and open sweeping views of Lake Washington. The architect left the flora intact, where possible, and site the two volumes among mature cedars, firs, maples and ferns. The structure is clad in clear Western Red Cedar which carries through to the interior ceiling. Edges are trimmed in contrasting aluminum. ◆

TABLE 10. Coverage of Western Red Cedar Siding

To obtain the coverage of a specified width of siding from Table 7, perform the following calculations:

1. Calculate total wall area (length x height).
2. Subtract square footage of openings (windows, doors) to determine wall area for siding.
3. Add 10% for trim loss.
4. Multiply figure by the appropriate factor from the table for linear or board feet.

Example:

1. Length x height = 160 square feet
2. Door = 20 square feet; Window = 20 square feet; Area for siding = 120 square feet
3. Add 10% for trim loss = 120 + 12 = 132 square feet
4. Assuming 6 in. siding: 132 x 2.67 = 357.4 linear feet; 132 x 1.33 = 175.6 board feet

Siding Type	Nominal Width (in)	Dressed Width (in)	Exposed Face Width (in)	Linear Feet Factor	Board Feet Factor
Bevel Siding1	4 6 8 10 12	3-1/2 5-1/2 7-1/2 9-1/2 11-1/2	2-1/2 4-1/2 6-1/2 8-1/2 10-1/2	4.80 2.67 1.85 1.41 1.14	1.60 1.33 1.23 1.18 1.14
Tongue and Groove Siding	4 6 8	3-3/8 5-3/8 7-1/8	3 5 6-3/4	4.00 2.40 1.77	1.33 1.20 1.19
Channel Siding	6 8 10	5-1/2 7-3/8 9-3/8	4-3/4 6-5/8 8-5/8	2.53 1.81 1.39	1.27 1.21 1.16
Board-and-Batten Siding	2 4 6 8 10 12	1-9/16 3-9/16 5-9/16 7-3/8 9-3/8 11-3/8	varies with width of board	see footnote 2	

TABLE 11. Design values for use in the USA for visually graded (WCLIB, WWPA) Western Red Cedar timbers (5 x 5 and larger)

Size Grade	Classification	Extreme Fiber Stress in Bending Fb	Tension Parallel to Grain Ft	Shear Parallel to Grain Fv	Compression Perpendicular to Grain Fc	Compression Parallel to Grain Fc	Modulus of Elasticity E
Select Structural	Beams and Stringers	1,150	675	140	425	875	1,000,000
No. 1 No. 2		975 625	475 325	140 140	425 425	725 475	1,000,000 800,000
Select Structural	Posts and Timber	1,100	725	140	425	925	1,000,000
No. 1 No. 2		875 550	600 350	140 140	425 425	800 550	1,000,000 800,000

Design values in pounds per square inch (psi)

TABLE 12. Duration of Load Adjustment (CD) for tabulated design values

Load Duration	Factor
Permanent	0.9
Ten Years (Normal Load)	1.0
Two Months (Snow Load)	1.15
Seven Days	1.25
Ten Minutes (Wind, Earthquake)	1.6
Impact	2.0

Confirm load requirements with local codes. Refer to Model Building Codes or the National Design Specification for high temperature or fire-retardant treated adjustment factors.

TABLE 13. Adjustments for compression perpendicular to grain to deformation basis of 0.02

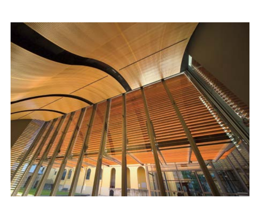
Design values for compression perpendicular to grain are established in accordance with the procedures set forth in ASTM D 2555 and D 245. ASTM procedures consider deformation under bearing loads as a serviceability limit state comparable to bending deflection because bearing loads rarely cause structural failures. Therefore, ASTM procedures for determining compression perpendicular to grain values are based on a deformation of 0.04_ and are considered adequate for most classes of structures. Where more stringent measures need be taken in design, the following formula permits the designer to adjust design values to a more conservative deformation basis of 0.02_.

Y02 = 0.73Y04 + 5.60

The Mint, Australia

Architect:
Francis-Jones Morehen Thorp

Western Red Cedar was chosen for the new head office of the historic houses trust of New South Wales, Australia for its appearance, durability and color. It simultaneously compliments and contrasts the sandstone of the existing building. Cedar was used for interior and exterior cladding and to form the dynamic louvred screen. ◆



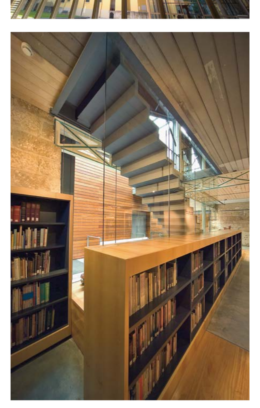


TABLE 14. Sizes of Dressed Western Red Cedar Lumber

Item	Thickness (in.): Nominal	Actual: Dry	Actual: Green	Width (in.): Nominal	Actual: Dry	Actual: Green
Boards	1 1-1/4	11/16 1	3/4 1-1/32	2 3 4 6 8 10 12	1-1/2 2-1/2 3-1/2 5-1/2 7-1/4 9-1/4 11-1/4	1-9/16 2-9/16 3-9/16 5-9/16 7-3/8 9-3/8 11-3/8
Garden Lumber	2 3 4	1-1/2 2-1/2 3-1/2	1-9/16 2-9/16 3-9/16	2 3 4 6 8 10 12	1-1/2 2-1/2 3-1/2 5-1/2 7-1/4 9-1/4 11-1/4	1-9/16 2-9/16 3-9/16 5-9/16 7-3/8 9-3/8 11-3/8
Timbers*	5 6 8		4-1/2 5-1/2 7-1/2	5 6 8		4-1/2 5-1/2 7-1/2

TABLE 15. Size Factors (CF) for tabulated design values

Grades	Nominal Width (depth) (in.)	Fb less than 4 in. thick	Fb 4 in. thick nominal	Ft	Fc	Other Properties
Select Structural No. 1 No. 2 & No. 3 14 & wider	4 & less 5 6 8 10 12 0.9	1.5 1.4 1.3 1.2 1.1 1.0 1.0	1.5 1.4 1.3 1.3 1.2 1.1	1.5 1.4 1.3 1.2 1.1 1.0 0.9	1.15 1.1 1.1 1.05 1.0 1.0 0.9	1.0 1.0 1.0 1.0 1.0 1.0 1.0
Construction & Standard	4 & less	1.0	1.0	1.0	1.0	1.0
Utility	4	1.0	1.0	1.0	1.0	1.0
Stud*	4 & less 5 & 6	1.1 1.0	1.1 1.0	1.1 1.0	1.05 1.0	1.0 1.0
MSR and plank Decking All grades & sizes	1.0	1.0		1.0	1.0	1.0

TABLE 16. Horizontal Shear Adjustment for tabulated design values (CH)

All horizontal shear base values are established as if a piece were split full length and as such the values are reduced from those permitted to be assigned in accordance with ASTM standards. This reduction is made to compensate for any degree of shake, check or split that might develop in a piece.

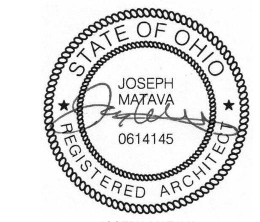
2 in. Thick (Nom.) Lumber		3 in. and Thicker (Nom.) Lumber	
For convenience, the table below may be used to determine horizontal shear values for any grade of 2"thick lumber in any species when the length of split or check is known:		Horizontal shear values for 3_ and thicker lumber also are established as if a piece were split full length. When specific lengths of splits are known and any increase in them is not anticipated, the following adjustments may be applied:	
When length of split on wide face does not exceed:	Multiply tabulated Fv value by:	When length of split on wide face does not exceed:	Multiply tabulated Fv value by:
No split 1/2 x wide face 3/4 x wide face 1 x wide face 1-1/2 x wide face or more	2.00 1.67 1.50 1.33 1.00	No split 1/2 x narrow face 1 x narrow face 1-1/2 x narrow face or more	2.00 1.67 1.33 1.00

Yale Sculpture Gallery

Architect:
KieranTimberlake

The Yale Sculpture Gallery sits at the end of a row of two-story 19th century houses. Scaled to fit this streetscape, the gallery is a taut wood box with side walls that subtly bow out. The warm Western Red Cedar cladding contrasts with the glass walls of the adjacent studio building and the cement cladding of the parking structure. The entrance is set within a recessed glass enclosure along the north end of the gallery, where an open screen of spaced cedar slats is suspended veil-like from the roof plate. The gallery's exterior wall is a ventilated wood rain screen. Strips of glass at open corners and the clerestory admit daylight. The cedar cladding pulls back at the corners to reveal narrow slit windows, while bands of horizontal metal wrap the walls at intervals, forming a visual strapping to hold the wall planes together. ◆

Peninsula
www.pa-architects.com
1775 Main Street
Peninsula, Ohio 44264
T 330.657.2800



JOSEPH MATAVA
0614145 | 12/31/23

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NORTH MAIN RETAIL
201 NORTH MAIN STREET, HUDSON, OHIO

N

PROJECT #: 2317

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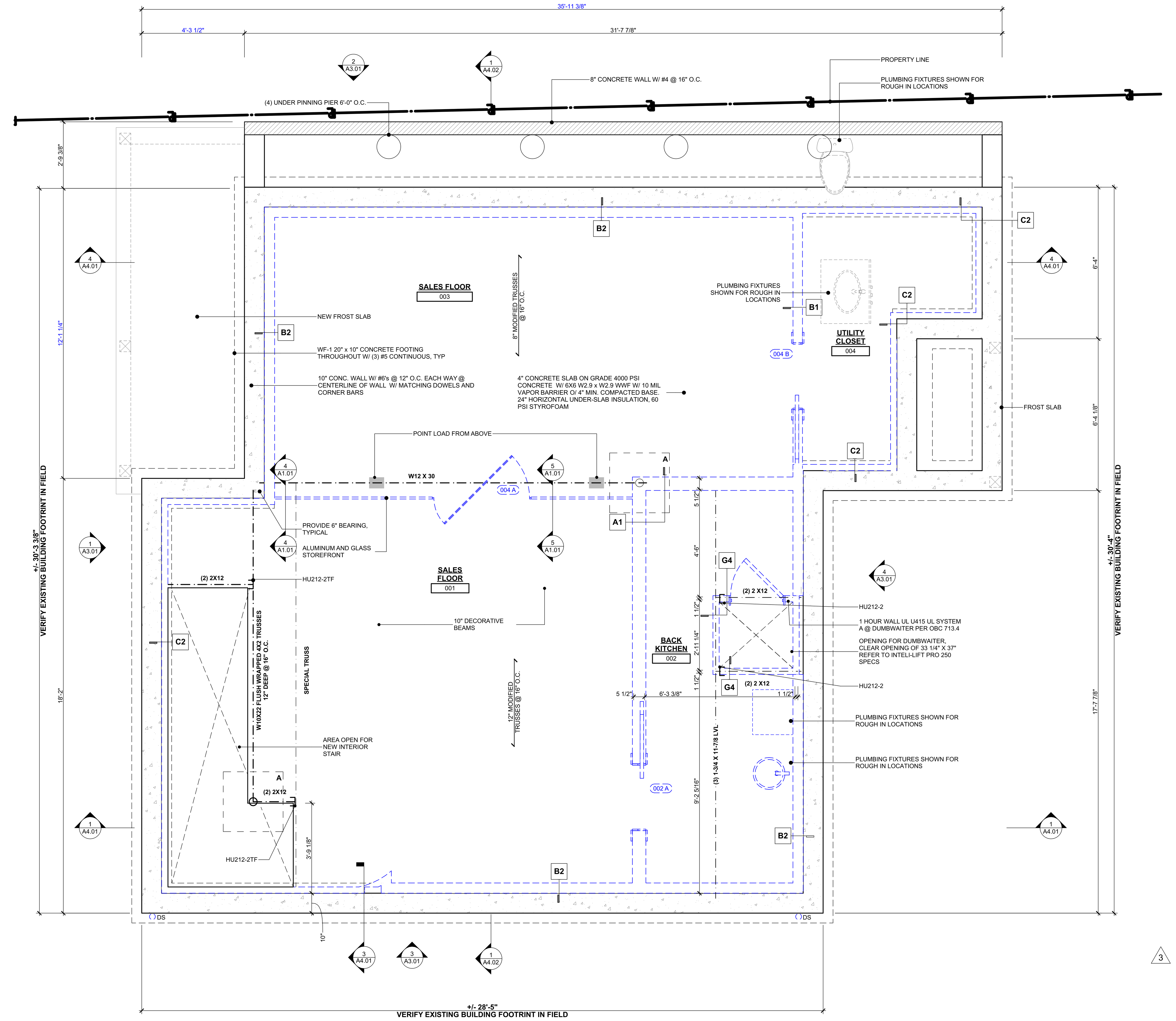
SPECIFICATIONS

A0.03



1 FOUNDATION PLAN

SCALE: 3/8" = 1'-0"

ITEMS SHOWN IN BLUE DASHED LINES ARE FOR REFERENCE ONLY AND NOT TO BE REVIEWED BY PLANS EXAMINER AT THIS TIME.

DOOR SCHEDULE

ID	ROOM NAME	DOOR WIDTH	DOOR HEIGHT	OPERATION	DOOR PANEL TYPE	DOOR PANEL MATERIAL	FRAME TYPE	FRAME MATERIAL	HARDWARE	REMARKS
002 A	SALES FLOOR	2'-8"	6'-8"	POCKET	D1	SC WOOD	F1	WD	---	
004 A	PROJECT AREA	4'-0"	6'-8"	PIVOT	D3	ALUM / GLASS	F1	HM	---	
004 B	SALES FLOOR	2'-8"	6'-8"	POCKET	D1	SC WOOD	F1	WD	---	
101 A	SALES FLOOR	3'-0"	6'-8"	SWING	D1	SC WOOD	F1	HM	DH-1	
101 B	SALES FLOOR	3'-0"	6'-8"	SWING	D2	ALUM / GLASS	F1	HM	DH-1	
101 C	PROJECT AREA	3'-0"	6'-8"	SWING	D1	SC WOOD	F1	WD	---	
102 A	ACCESSIBLE RESTROOM	3'-0"	6'-8"	SWING	D1	SC WOOD	F1	WD	---	
201 A	OFFICE	3'-0"	6'-0"	SWING	D1	SC WOOD	F1	WD	---	

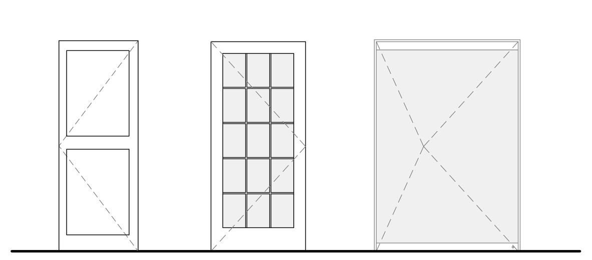
COLUMN & FOOTING SCHEDULE

MARK	COLUMN	LENGTH	WIDTH	DEPTH	REINFORCEMENT
WF-1	-	-	20"	10"	(3) #5 CONT.
A	4" X 11 GA.	2'-6"	2'-6"	12"	-

DOOR HARDWARE NOTES

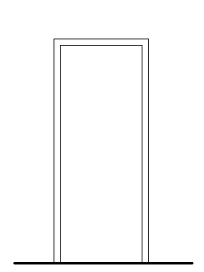
DH-1 ALL PASSAGE SETS, PRIVACY SETS AND LOCKSETS SHALL BE ANSI/BHMA GRADE 1 COMMERCIAL WITH ADA COMPLIANT LEVER HARDWARE. ALL LOCKSETS SHALL BE THE PRODUCTS OF A SINGLE MANUFACTURER. ALL DOOR LOCKS SHALL ALLOW EGRESS WITHOUT THE USE OF A KEY AND BY MEANS OF A SINGLE OPERATION.

DOOR PANEL TYPE



TYPE D1 TYPE D2 TYPE D3

DOOR FRAME TYPE



TYPE F1

FOUNDATION PLAN GENERAL NOTES

FLOOR TRUSS CRITERIA

12" FLOOR TRUSSES

TCLL= 100 PSF
TCDL= 25 PSF
BCDC = 5PSF
ΔTTL = L/360

SUBSTITUTION CHART:

LVL SIZE	WF STEEL SIZE
(2) 12" LVL	W10 x 15
(2) 14" LVL	W12 x 16
(2) 16" LVL	W14 x 22
(2) 18" LVL	W16 x 26
(3) 12" LVL	W10 x 17
(3) 14" LVL	W12 x 16
(3) 16" LVL	W14 x 22
(3) 18" LVL	W16 x 26
(2) 16" LVL W/ 1/2" PL	W16 x 26
(2) 16" LVL W/ (2) 1/2" PL	W16 x 40

GENERAL NOTES

BLOCK WEBS SOLID AT BEARING WALL LOCATIONS ABOVE

CONTRACTOR TO EXTEND ALL POSTS DOWN TO SOUND FOUNDATION, INSTALL FULL DEPTH SOLID BLOCKING AT ALL POINT LOAD LOCATIONS

ALL FOOTINGS TO EXTEND DOWN TO FROST LEVEL MIN.

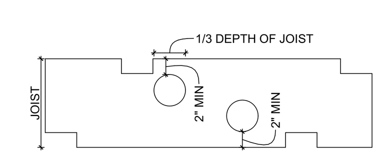
INDICATES LOCATION OF POINT LOAD ABOVE

INDICATES LOCATION OF BEARING WALL ABOVE

COORDINATE EXACT LOCATIONS OF FLOOR DRAIN WITH MECH.CONTRACTOR

IN ORDER TO REMAIN EXPOSED, WEBS OF TJI'S MUST BE PROTECTED WITH APPLIED FIRE PROTECTION (I.E. GYPSUM BOARD, FIBER BLANKET, INTUMESCENT COATING)

FOAM INSULATION (BOARD OR SPRAY) MUST BE CLASS ONE FIRE RATED OR COVERED BY A THERMAL BARRIER (I.E. GYPSUM BOARD, INTUMESCENT COATING)



SECTION 502.8
NOTCHES IN THE TOP OR BOTTOM OF JOISTS SHALL NOT EXCEED ONE-SIXTH THE DEPTH OF THE JOIST AND SHALL NOT BE LOCATED IN THE MIDDLE THIRD OF THE SPAN. WHERE JOISTS ARE NOTCHED ON THE ENDS FOR A LEDGER, THE NOTCH SHALL NOT EXCEED ONE-FOURTH THE JOISTS DEPTH. CANTILEVERED JOISTS SHALL NOT BE NOTCHED UNLESS THE REDUCED SECTION PROPERTIES AND LUMBER DEFECTS ARE CONSIDERED IN THE DESIGN.
HOLES DRILLED OR BORED IN JOISTS SHALL NOT BE WITHIN 2 INCHES OF THE TOP OR BOTTOM OR THE JOISTS AND THEIR DIAMETER SHALL NOT EXCEED ONE-THIRD THE DEPTH OF THE JOIST.

SECTION 602.6
ANY STUD IN AN EXTERIOR BEARING PARTITION MAY BE CUT OR NOTCHED TO A DEPTH NOT EXCEEDING 25% OF ITS WIDTH. STUDS IN NON-BEARING PARTITIONS MAY BE NOTCHED TO A DEPTH NOT TO EXCEED 40% OF A SINGLE STUD WIDTH. ANY STUD MAY BE BORED OR DRILLED, PROVIDED THAT THE DIAMETER OF THE RESULTING HOLES IS NO GREATER THAN 40% OF THE STUD WIDTH, THE EDGE OF THE HOLE IS NO CLOSER THAN 5/8 INCH TO THE EDGE OF THE STUD, AND THE HOLE IS NOT LOCATED IN THE SAME SECTION AS A CUT OR NOTCH.

STEEL LINTEL SCHEDULE

PROVIDE STEEL LINTELS AS PER THE FOLLOWING SCHEDULE IN MASONRY WALL OPENINGS WHEN NOT SHOWN ON DRAWINGS, OR IN OPENINGS REQUIRED BY THE ARCHITECTURAL, MECHANICAL, AND ELECTRICAL DRAWINGS.

L3 1/2 x 3 1/2 x 1/4 FOR OPENINGS UP TO 4'-0".
L5 x 3 1/2 x 5/16 FOR OPENINGS FROM 4'-1" TO 6'-0"
L6 x 3 1/2 x 5/16 FOR OPENINGS FROM 6'-1" TO 7'-0"
W8 x 18 with 5/16 PLATE FOR OPENINGS FROM 7'-1" TO 10'-0"

FOR OPENINGS GREATER THAN 10'-0" AND NOT SHOWN ON PLANS, ALLOW FOR MINIMUM BEAM WEIGHT OF 36 PLF PLUS A 5/16" x 11" BOTTOM PLATE

ALL LINTELS SHALL BEAR ON 8" OF SOLID MASONRY, UNO.

USE ONE ANGLE FOR EACH 4" WHYTHE OF MASONRY. PLATES ARE TO BE 1" LESS THAN NOMINAL WALL THICKNESS.

MINIMUM THICKNESS OF LINTELS IN EXTERIOR WALLS TO BE 5/16".

ANGLES OR PLATES IN EXTERIOR WIDTHS OF MASONRY WALLS TO BE HOT DIPPED GALVANIZED.

FOR MULTI WYTHE WALLS WITH AIR SPACES, CONTRACTORS IS TO INCLUDE (6) ADDITIONAL ANGLES, PLATES, AND CHANNELS TO CLOSE OFF AIRSPACE AT LINTEL LOCATIONS. SEE DETAILS ON DRAWINGS. IF NO DETAILS ARE SHOWN, CONTACT ENGINEER FOR FURTHER INFORMATION AND DETAILS.

FLOOR PLAN GENERAL NOTES

1. ALL INTERIOR WALLS SHALL BE 2x6 WOOD STUDS WITH (1) LAYER 5/8" GYPSUM BOARD ON BOTH SIDES UNLESS NOTED OTHERWISE. REFER TO G1.01 FOR TERMINATION DETAILS AND FLOOR TO FLOOR HEIGHTS, TYPICAL.
2. CIVIL GROUND FLOOR FINISH ELEVATION = **1073.60'**. ARCHITECTURAL GROUND FLOOR FINISH ELEVATION = 0'-0".
3. VERIFY DIMENSIONS AND CONDITIONS IN FIELD. WHEN DIMENSIONS AND/OR CONDITIONS AS INDICATED ON DRAWINGS CONFLICT WITH ACTUAL, CONTACT ARCHITECT FOR CLARIFICATION.
4. TYPICAL MOUNTING HEIGHTS & LOCATIONS OF FIXTURES, ACCESSORIES AND SELECT EQUIPMENT ARE SHOWN ON SHEET G1.01.
5. FIELD VERIFY ALL EXISTING CONDITIONS PRIOR TO BIDDING, DEMOLITION, AND NEW CONSTRUCTION.
6. ALL WOOD BLOCKING AND BACKER BOARDS ARE TO BE FIRE RETARDANT TREATED TYPE.
7. PROVIDE DOUBLE TOP TRACK DEFLECTION JOINT AT ALL WALLS WHICH EXTEND TO UNDERSIDE OF ROOF DECK ABOVE.
8. COORDINATE LOCATION OF EQUIPMENT AND PENETRATIONS WITH MECHANICAL, ELECTRICAL, AND PLUMBING DRAWINGS.
9. SEE EXTERIOR ELEVATIONS FOR IDENTIFICATION OF WINDOW TYPES AND SIZES.
10. HINGE SIDE OF DOOR JAMB LOCATED IN METAL STUD WALLS TO BE 4" FROM ADJACENT PERPENDICULAR WALLS, UNO.
11. ALL DOORS ARE DIMENSIONED TO THE CENTER LINE OF THE ROUGH OPENING.
12. VERIFY LOCATIONS OF FIRE EXTINGUISHERS WITH LOCAL FIRE DEPARTMENT OFFICIAL AND ARCHITECT PRIOR TO PLACEMENT. ALL EXTINGUISHERS TO BE RECESSED CABINET MOUNTED TYPE, UNO.
13. PROVIDE BLOCKING BEHIND ALL A/V EQUIPMENT, WALL SPEAKERS, CEILING MOUNTED PROJECTORS, HANDRAILS, ACCESSORIES, ETC. CONTRACTOR RESPONSIBLE FOR MISC STEEL, UNISTRUT, FRT WOOD BLOCKING, STRAPPING ETC AS REQUIRED TO FULLY SUPPORT EQUIPMENT AND ACCESSORIES AS INDICATED IN DOCUMENTS.
14. MOISTURE RESISTANT GYP BD TO BE USED ON WALLS AND CEILINGS OF ALL RESTROOMS AND KITCHENS.
15. PROVIDE LEVEL SURFACES AND PREP FLOOR TO RECEIVE SCHEDULED FINISH

FLOOR PLAN CODED NOTES

NOTE: ALL CODED NOTES MAY NOT OCCUR ON EVERY SHEET

1 4'-0" x 8'-0" OPEN TABLE FOR SAMPLING & SALES
2 WALL MOUNTED TV
3 LF DISPLAY CABINET
4 TALL CABINETS
5 DUMBWAITER THAT TRAVELS FROM THE BASEMENT TO FIRST FLOOR WITH UL415 SYSTEM A (1HR) SHAFT WALLS, BASIS OF DESIGN: INTELI-LIFT PRO 250 28x28x28 SAME SIDE WITH BI-PARTING GATE & SWING DOORS
6 NEW STAIR TO BASEMENT WITH CUSTOM DISPLAY MILLWORK
7 CONCEALED DOOR
8 EXISTING PILASTER TO BE FURRED OUT
9 DISHWASHER
10 LF DISPLAY CABINET SUSPENDED

INTERIOR STUD WALL SYMBOL KEY

A1

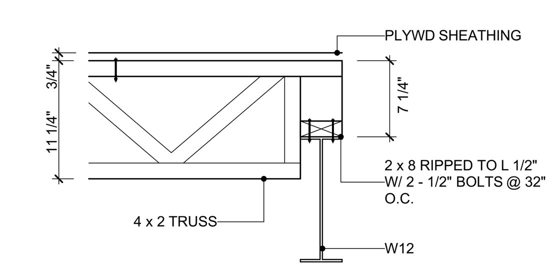
GYP. BOARD CONFIGURATION

1 - GYP. BOARD FULL HEIGHT,BOTH SIDES
2 - GYP. BOARD FULL HEIGHT,ONE SIDE
3 - GYP. BOARD TO 6" ABOVE FINISH CEILING, BOTH SIDES, NON-RATED
4 - 1" GYP BOARD TYPE SLX ON ONE SIDE, 5/8" GYP BOARD TYPE SCX ON OTHER SIDE
5 - 2 HR FIRE BARRIER
6 - 1 HR FIRE PARTITION

STUD WIDTH / TYPE DESIGNATION

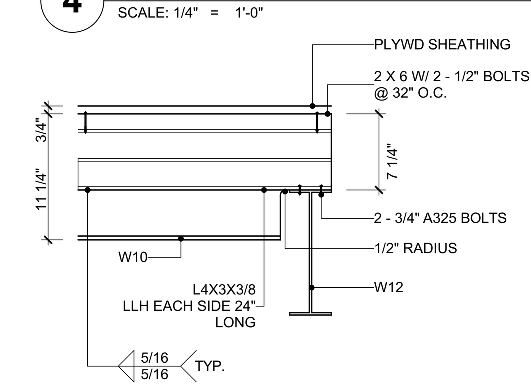
A = 5 1/2" / WD
B = 3 1/2" / WD
C = 1 1/2" / WD
D = 7 1/4" / WD
H = RESERVED
E = 9 1/4" / WD
F = 11 1/4" / WD
G = 2 1/2" CH MTL STUDS, 20 GA. @ 24" O.C.

TYPES:
WD DIMENSIONAL LUMBER
MTL METAL STUDS
ST STRUCTURAL STUDS



4 TRUSS SECTION

SCALE: 1/4" = 1'-0"



5 TRUSS SECTION

SCALE: 1/4" = 1'-0"

SQUARE FOOTAGE

FLOOR	AREA
FOUNDATION	852
FIRST FLOOR	968
SECOND FLOOR	520
	2,340 ft²

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JOSEPH MATAVA
0614145 | 12/31/23

PROJECT TEAM:
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WRIGHT ENGINEERING
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NORTH MAIN RETAIL

201 NORTH MAIN STREET, HUDSON, OHIO

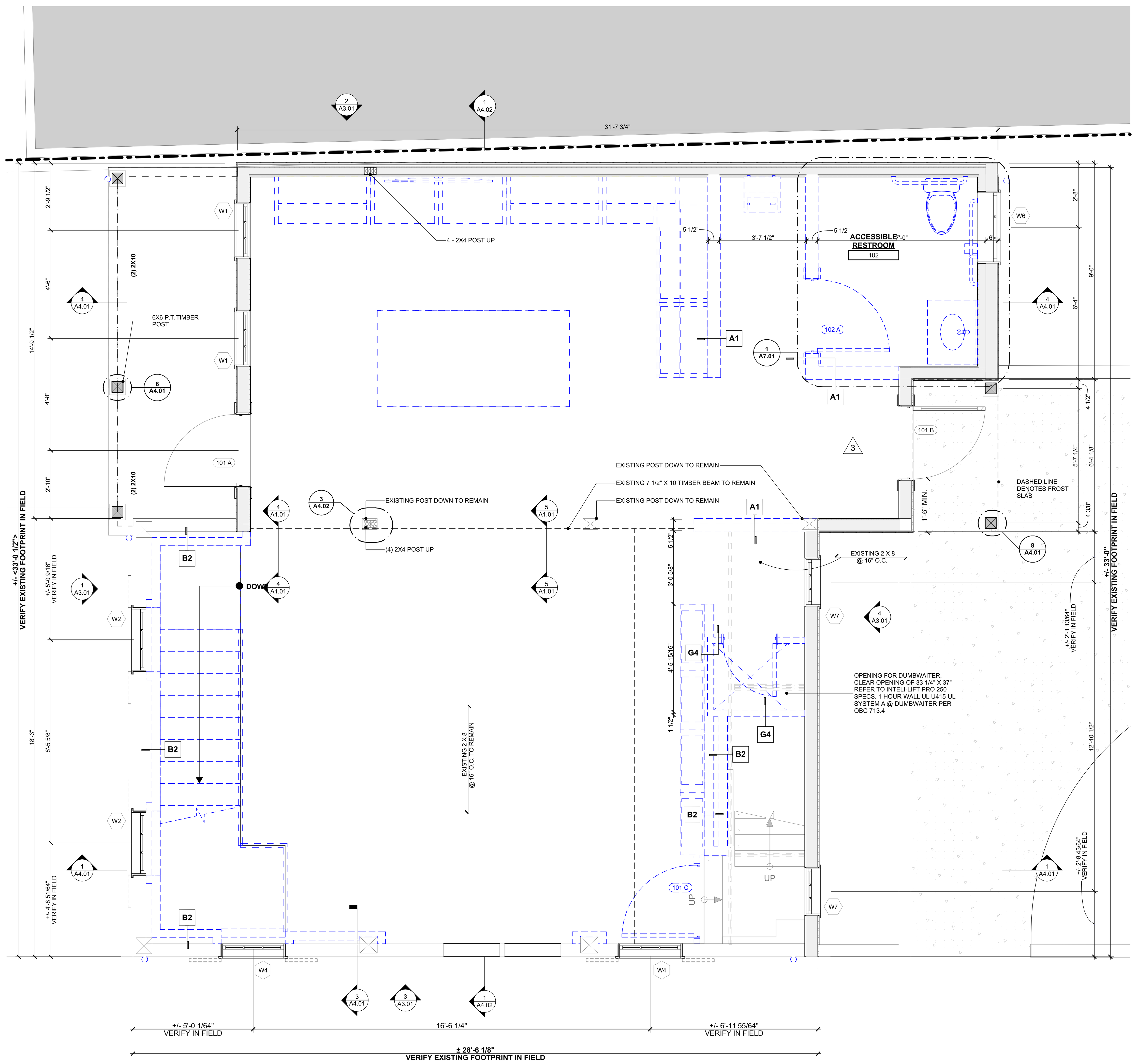
N

PROJECT #: 2317

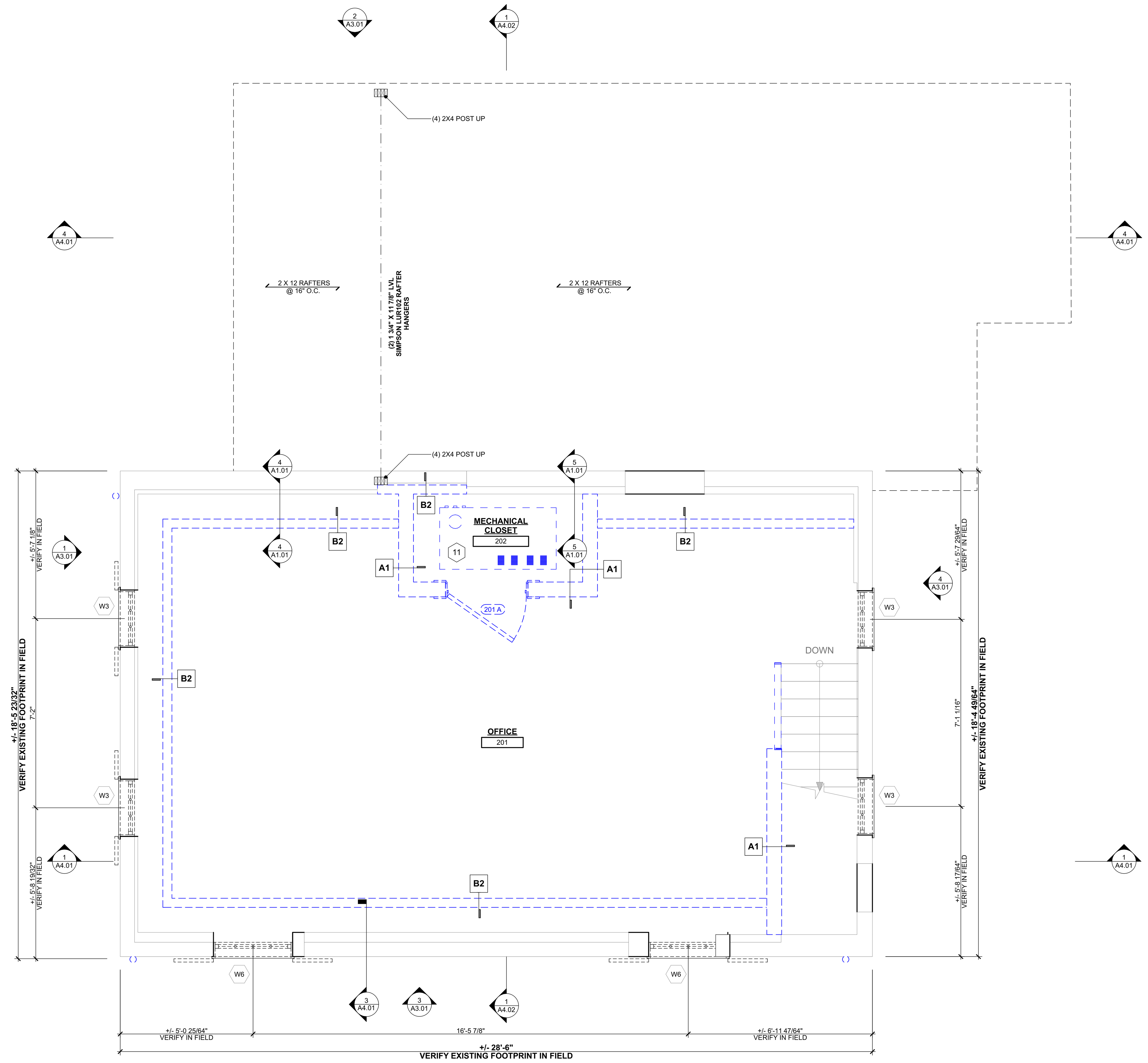
ISSUE	ID	DATE
BID PACKAGE #1		08/23/23
BULLETIN #1	1	11/22/23
BULLETIN #2	2	01/02/24
BULLETIN #3	3	01/10/24

BASEMENT AND FOUNDATION PLANS

A1.01



1 FIRST FLOOR PLAN
SCALE: 3/8" = 1'-0"



2 SECOND FLOOR PLAN
SCALE: 3/8" = 1'-0"

ITEMS SHOWN IN BLUE DASHED LINES ARE FOR REFERENCE ONLY AND NOT TO BE REVIEWED BY PLANS EXAMINER AT THIS TIME.

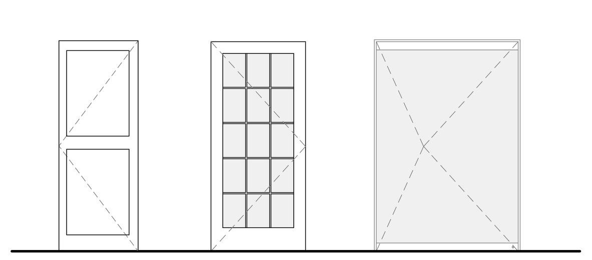
DOOR SCHEDULE

ID	ROOM NAME	DOOR WIDTH	DOOR HEIGHT	OPERATION	DOOR PANEL TYPE	DOOR PANEL MATERIAL	FRAME TYPE	FRAME MATERIAL	HARDWARE	REMARKS
002 A	SALES FLOOR	2'-8"	6'-8"	POCKET	D1	SC WOOD	F1	WD	---	
004 A	PROJECT AREA	4'-0"	6'-8"	PIVOT	D3	ALUM / GLASS	F1	HM	---	
004 B	SALES FLOOR	2'-8"	6'-8"	POCKET	D1	SC WOOD	F1	WD	---	
101 A	SALES FLOOR	3'-0"	6'-8"	SWING	D1	SC WOOD	F1	HM	DH-1	
101 B	SALES FLOOR	3'-0"	6'-8"	SWING	D2	ALUM / GLASS	F1	HM	DH-1	
101 C	PROJECT AREA	3'-0"	6'-8"	SWING	D1	SC WOOD	F1	WD	---	
102 A	ACCESSIBLE RESTROOM	3'-0"	6'-8"	SWING	D1	SC WOOD	F1	WD	---	
201 A	OFFICE	3'-0"	6'-0"	SWING	D1	SC WOOD	F1	WD	---	

DOOR HARDWARE NOTES

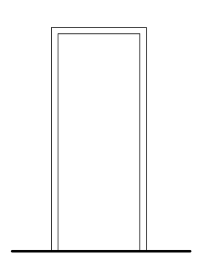
DH-1 ALL PASSAGE SETS, PRIVACY SETS AND LOCKSETS SHALL BE ANSI/BHMA GRADE 1 COMMERCIAL WITH ADA COMPLIANT LEVER HARDWARE. ALL LOCKSETS SHALL BE THE PRODUCTS OF A SINGLE MANUFACTURER. ALL DOOR LOCKS SHALL ALLOW EGRESS WITHOUT THE USE OF A KEY AND BY MEANS OF A SINGLE OPERATION.

DOOR PANEL TYPE



TYPE D1 TYPE D2 TYPE D3

DOOR FRAME TYPE



TYPE F1

FOUNDATION PLAN GENERAL NOTES

FLOOR TRUSS CRITERIA

12" FLOOR TRUSSES

TCLL= 100 PSF
TCDL= 25 PSF
BCDC = 5PSF
ΔTTL = L/360

SUBSTITUTION CHART:

LVL SIZE	WF STEEL SIZE
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(2) 14" LVL	W12 x 16
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(2) 18" LVL	W16 x 26
(3) 12" LVL	W10 x 17
(3) 14" LVL	W12 x 16
(3) 16" LVL	W14 x 22
(3) 18" LVL	W16 x 26
(2) 16" LVL W/ 1/2" PL	W16 x 26
(2) 16" LVL W/ (2) 1/2" PL	W16 x 40

GENERAL NOTES

BLOCK WEBS SOLID AT BEARING WALL LOCATIONS ABOVE

CONTRACTOR TO EXTEND ALL POSTS DOWN TO SOUND FOUNDATION, INSTALL FULL DEPTH SOLID BLOCKING AT ALL POINT LOAD LOCATIONS

ALL FOOTINGS TO EXTEND DOWN TO FROST LEVEL MIN.

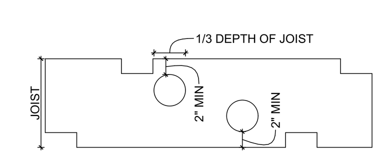
INDICATES LOCATION OF POINT LOAD ABOVE

INDICATES LOCATION OF BEARING WALL ABOVE

COORDINATE EXACT LOCATIONS OF FLOOR DRAIN WITH MECH.CONTRACTOR

IN ORDER TO REMAIN EXPOSED, WEBS OF TJI'S MUST BE PROTECTED WITH APPLIED FIRE PROTECTION (I.E. GYPSUM BOARD, FIBER BLANKET, INTUMESCENT COATING)

FOAM INSULATION (BOARD OR SPRAY) MUST BE CLASS ONE FIRE RATED OR COVERED BY A THERMAL BARRIER (I.E. GYPSUM BOARD, INTUMESCENT COATING)



SECTION 502.8
NOTCHES IN THE TOP OR BOTTOM OF JOISTS SHALL NOT EXCEED ONE-SIXTH THE DEPTH OF THE JOIST AND SHALL NOT BE LOCATED IN THE MIDDLE THIRD OF THE SPAN. WHERE JOISTS ARE NOTCHED ON THE ENDS FOR A LEDGER, THE NOTCH SHALL NOT EXCEED ONE-FOURTH THE JOISTS DEPTH. CANTILEVERED JOISTS SHALL NOT BE NOTCHED UNLESS THE REDUCED SECTION PROPERTIES AND LUMBER DEFECTS ARE CONSIDERED IN THE DESIGN.
HOLES DRILLED OR BORED IN JOISTS SHALL NOT BE WITHIN 2 INCHES OF THE TOP OR BOTTOM OR THE JOISTS AND THEIR DIAMETER SHALL NOT EXCEED ONE-THIRD THE DEPTH OF THE JOIST.

SECTION 602.6
ANY STUD IN AN EXTERIOR BEARING PARTITION MAY BE CUT OR NOTCHED TO A DEPTH NOT EXCEEDING 25% OF ITS WIDTH. STUDS IN NON-BEARING PARTITIONS MAY BE NOTCHED TO A DEPTH NOT TO EXCEED 40% OF A SINGLE STUD WIDTH. ANY STUD MAY BE BORED OR DRILLED, PROVIDED THAT THE DIAMETER OF THE RESULTING HOLES IS NO GREATER THAN 40% OF THE STUD WIDTH, THE EDGE OF THE HOLE IS NO CLOSER THAN 5/8 INCH TO THE EDGE OF THE STUD, AND THE HOLE IS NOT LOCATED IN THE SAME SECTION AS A CUT OR NOTCH.

STEEL LINTEL SCHEDULE

PROVIDE STEEL LINTELS AS PER THE FOLLOWING SCHEDULE IN MASONRY WALL OPENINGS WHEN NOT SHOWN ON DRAWINGS, OR IN OPENINGS REQUIRED BY THE ARCHITECTURAL, MECHANICAL, AND ELECTRICAL DRAWINGS.

L3 1/2 x 3 1/2 x 1/4 FOR OPENINGS UP TO 4'-0".
L5 x 3 1/2 x 5/16 FOR OPENINGS FROM 4'-1" TO 6'-0"
L6 x 3 1/2 x 5/16 FOR OPENINGS FROM 6'-1" TO 7'-0"
W8 x 18 with 5/16 PLATE FOR OPENINGS FROM 7'-1" TO 10'-0"

FOR OPENINGS GREATER THAN 10'-0" AND NOT SHOWN ON PLANS, ALLOW FOR MINIMUM BEAM WEIGHT OF 36 PLF PLUS A 5/16" x 11" BOTTOM PLATE

ALL LINTELS SHALL BEAR ON 8" OF SOLID MASONRY, UNO.

USE ONE ANGLE FOR EACH 4" WHYTHE OF MASONRY. PLATES ARE TO BE 1" LESS THAN NOMINAL WALL THICKNESS.

MINIMUM THICKNESS OF LINTELS IN EXTERIOR WALLS TO BE 5/16".

ANGLES OR PLATES IN EXTERIOR WIDTHS OF MASONRY WALLS TO BE HOT DIPPED GALVANIZED.

FOR MULTI WYTHE WALLS WITH AIR SPACES, CONTRACTORS IS TO INCLUDE (6) ADDITIONAL ANGLES, PLATES, AND CHANNELS TO CLOSE OFF AIRSPACE AT LINTEL LOCATIONS. SEE DETAILS ON DRAWINGS. IF NO DETAILS ARE SHOWN, CONTACT ENGINEER FOR FURTHER INFORMATION AND DETAILS.

FLOOR PLAN GENERAL NOTES

1. ALL INTERIOR WALLS SHALL BE 2x6 WOOD STUDS WITH (1) LAYER 5/8" GYPSUM BOARD ON BOTH SIDES UNLESS NOTED OTHERWISE. REFER TO G1.01 FOR TERMINATION DETAILS AND FLOOR TO FLOOR HEIGHTS, TYPICAL.
2. CIVIL GROUND FLOOR FINISH ELEVATION = **1073.60'**. ARCHITECTURAL GROUND FLOOR FINISH ELEVATION = 0'-0".
3. VERIFY DIMENSIONS AND CONDITIONS IN FIELD. WHEN DIMENSIONS AND/OR CONDITIONS AS INDICATED ON DRAWINGS CONFLICT WITH ACTUAL, CONTACT ARCHITECT FOR CLARIFICATION.
4. TYPICAL MOUNTING HEIGHTS & LOCATIONS OF FIXTURES, ACCESSORIES AND SELECT EQUIPMENT ARE SHOWN ON SHEET G1.01.
5. FIELD VERIFY ALL EXISTING CONDITIONS PRIOR TO BIDDING, DEMOLITION, AND NEW CONSTRUCTION.
6. ALL WOOD BLOCKING AND BACKER BOARDS ARE TO BE FIRE RETARDANT TREATED TYPE.
7. PROVIDE DOUBLE TOP TRACK DEFLECTION JOINT AT ALL WALLS WHICH EXTEND TO UNDERSIDE OF ROOF DECK ABOVE.
8. COORDINATE LOCATION OF EQUIPMENT AND PENETRATIONS WITH MECHANICAL, ELECTRICAL, AND PLUMBING DRAWINGS.
9. SEE EXTERIOR ELEVATIONS FOR IDENTIFICATION OF WINDOW TYPES AND SIZES.
10. HINGE SIDE OF DOOR JAMB LOCATED IN METAL STUD WALLS TO BE 4" FROM ADJACENT PERPENDICULAR WALLS, UNO.
11. ALL DOORS ARE DIMENSIONED TO THE CENTER LINE OF THE ROUGH OPENING.
12. VERIFY LOCATIONS OF FIRE EXTINGUISHERS WITH LOCAL FIRE DEPARTMENT OFFICIAL AND ARCHITECT PRIOR TO PLACEMENT. ALL EXTINGUISHERS TO BE RECESSED CABINET MOUNTED TYPE, UNO.
13. PROVIDE BLOCKING BEHIND ALL A/V EQUIPMENT, WALL SPEAKERS, CEILING MOUNTED PROJECTORS, HANDRAILS, ACCESSORIES, ETC. CONTRACTOR RESPONSIBLE FOR MISC STEEL, UNISTRUT, FRT WOOD BLOCKING, STRAPPING ETC AS REQUIRED TO FULLY SUPPORT EQUIPMENT AND ACCESSORIES AS INDICATED IN DOCUMENTS.
14. MOISTURE RESISTANT GYP BD TO BE USED ON WALLS AND CEILINGS OF ALL RESTROOMS AND KITCHENS.
15. PROVIDE LEVEL SURFACES AND PREP FLOOR TO RECEIVE SCHEDULED FINISH

FLOOR PLAN CODED NOTES

NOTE: ALL CODED NOTES MAY NOT OCCUR ON EVERY SHEET

1 4'-0" x 8'-0" OPEN TABLE FOR SAMPLING & SALES
2 WALL MOUNTED TV
3 LF DISPLAY CABINET
4 TALL CABINETS
5 DUMBWAITER THAT TRAVELS FROM THE BASEMENT TO FIRST FLOOR WITH UL415 SYSTEM A (1HR) SHAFT WALLS, BASIS OF DESIGN: INTELI-LIFT PRO 250 28x28x28 SAME SIDE WITH BI-PARTING GATE & SWING DOORS
6 NEW STAIR TO BASEMENT WITH CUSTOM DISPLAY MILLWORK
7 CONCEALED DOOR
8 EXISTING PILASTER TO BE FURRED OUT
9 DISHWASHER
10 LF DISPLAY CABINET SUSPENDED

INTERIOR STUD WALL SYMBOL KEY

A1

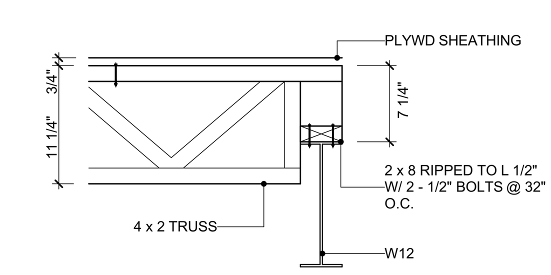
GYP. BOARD CONFIGURATION

1 - GYP. BOARD FULL HEIGHT,BOTH SIDES
2 - GYP. BOARD FULL HEIGHT,ONE SIDE
3 - GYP. BOARD TO 6" ABOVE FINISH CEILING, BOTH SIDES, NON-RATED
4 - 1" GYP BOARD TYPE SLX ON ONE SIDE, 5/8" GYP BOARD TYPE SCX ON OTHER SIDE
5 - 2 HR FIRE BARRIER
6 - 1 HR FIRE PARTITION

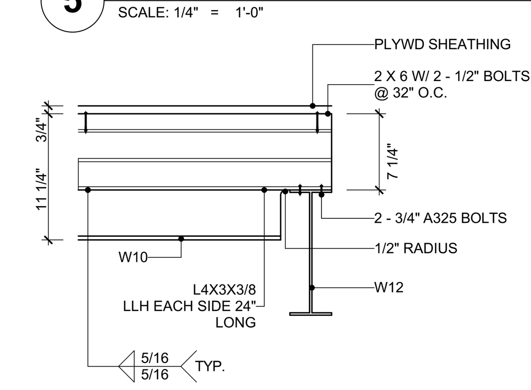
STUD WIDTH / TYPE DESIGNATION

A = 5 1/2" / WD
B = 3 1/2" / WD
C = 1 1/2" / WD
D = 7 1/4" / WD
H = RESERVED
E = 9 1/4" / WD
F = 11 1/4" / WD
G = 2 1/2" CH MTL STUDS, 20 GA. @ 24" O.C.

TYPES:
WD DIMENSIONAL LUMBER
MTL METAL STUDS
ST STRUCTURAL STUDS



5 TRUSS SECTION
SCALE: 1/4" = 1'-0"



6 TRUSS SECTION
SCALE: 1/4" = 1'-0"

SQUARE FOOTAGE	
FLOOR	AREA
FOUNDATION	852
FIRST FLOOR	968
SECOND FLOOR	520
	2,340 ft²

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PROJECT TEAM:
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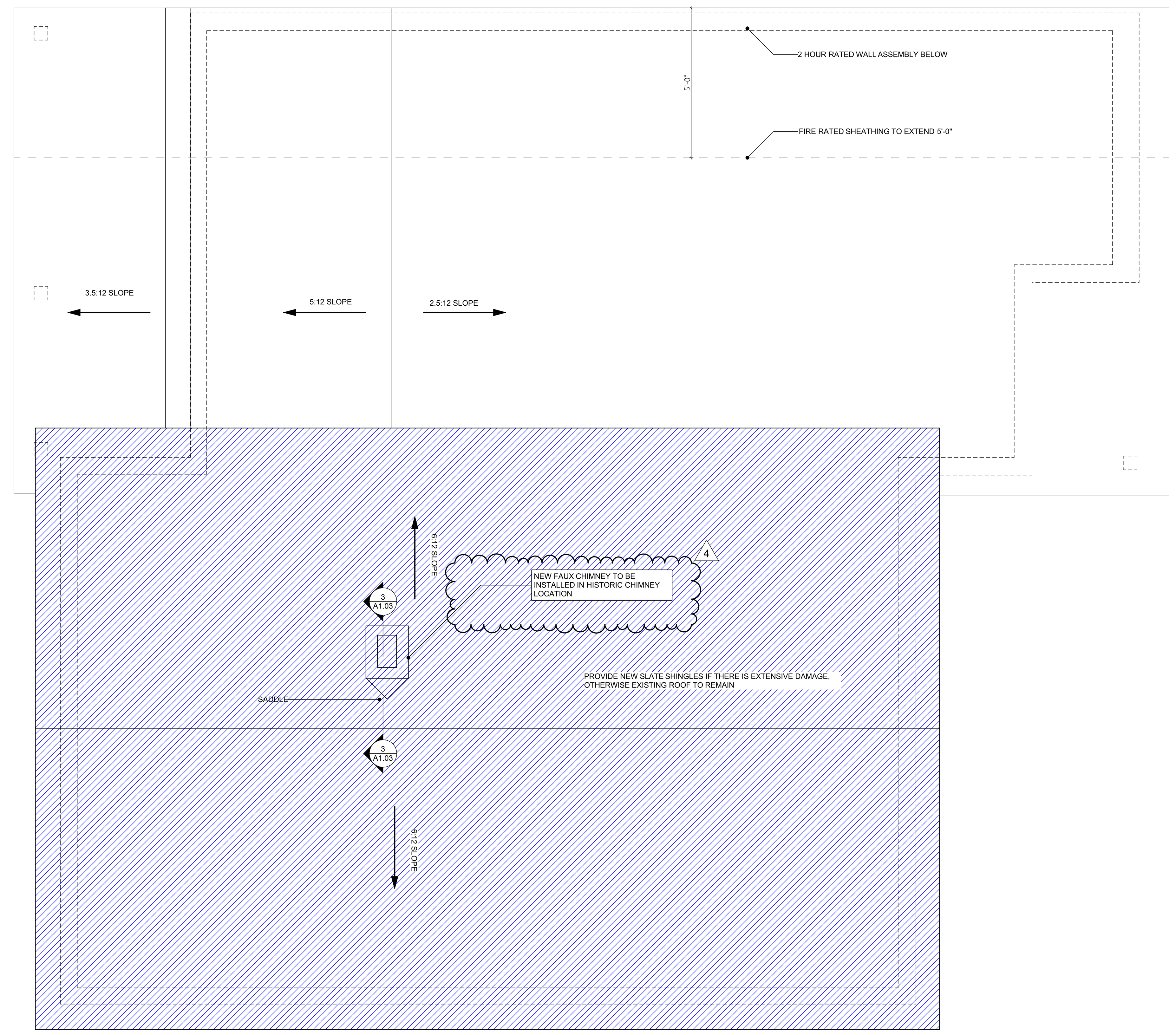
NORTH MAIN RETAIL
201 NORTH MAIN STREET, HUDSON, OHIO

N

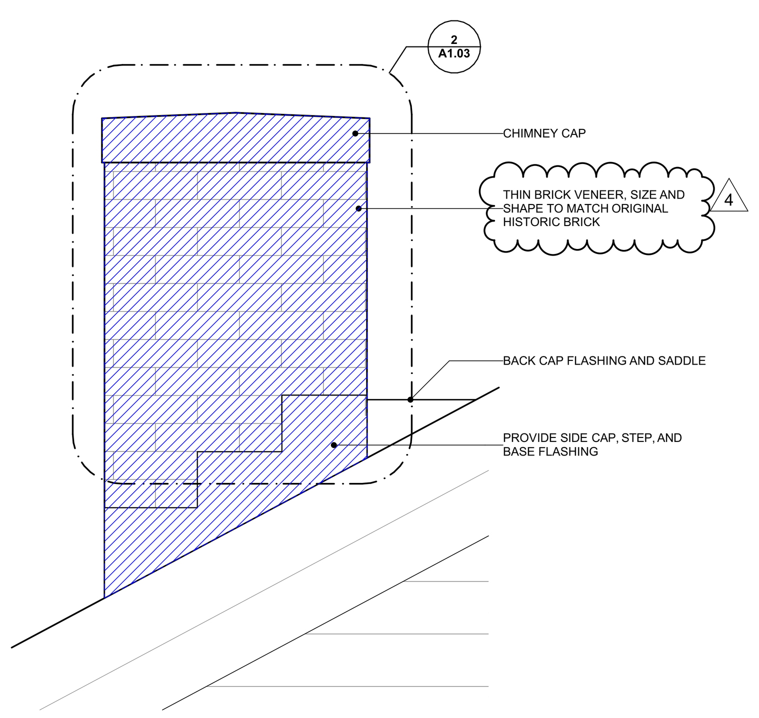
PROJECT #: 2317

ISSUE	ID	DATE
BID PACKAGE #1		08/23/23
BULLETIN #1	1	11/22/23
BULLETIN #2	2	01/02/24
BULLETIN #3	3	01/10/24

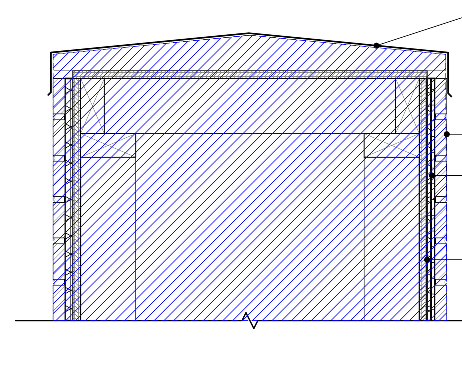
FIRST AND SECOND FLOOR PLANS (FOR REFERENCE)
A1.02



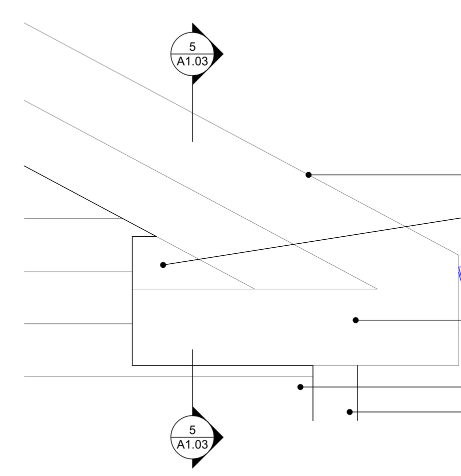
1 **ROOF PLAN**
SCALE: 3/8" = 1'-0"



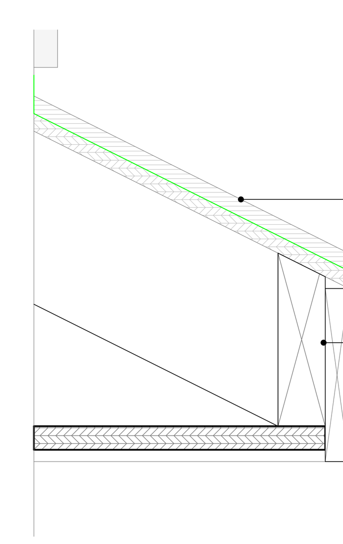
2 **ENLARGED CHIMNEY ELEVATION**
SCALE: 1" = 1'-0"



3 **CHIMNEY CAP DETAIL**
SCALE: 1 1/2"= 1'-0"



4 **ROOF DETAIL**
SCALE: 1" = 1'-0"



5 **ROOF SECTION DETAIL**
SCALE: 3" = 1'-0"

FLOOR PLAN GENERAL NOTES

1. ALL INTERIOR WALLS SHALL BE 2x6 WOOD STUDS WITH (1) LAYER 5/8" GYPSUM BOARD ON BOTH SIDES UNLESS NOTED OTHERWISE. REFER TO G1.01 FOR TERMINATION DETAILS AND FLOOR TO FLOOR HEIGHTS, TYPICAL.
2. CIVIL GROUND FLOOR FINISH ELEVATION = **1073.60'**. ARCHITECTURAL GROUND FLOOR FINISH ELEVATION = 0'-0".
3. VERIFY DIMENSIONS AND CONDITIONS IN FIELD. WHEN DIMENSIONS AND/OR CONDITIONS AS INDICATED ON DRAWINGS CONFLICT WITH ACTUAL, CONTACT ARCHITECT FOR CLARIFICATION.
4. TYPICAL MOUNTING HEIGHTS & LOCATIONS OF FIXTURES, ACCESSORIES AND SELECT EQUIPMENT ARE SHOWN ON SHEET G1.01.
5. FIELD VERIFY ALL EXISTING CONDITIONS PRIOR TO BIDDING, DEMOLITION, AND NEW CONSTRUCTION.
6. ALL WOOD BLOCKING AND BACKER BOARDS ARE TO BE FIRE RETARDANT TREATED TYPE.
7. PROVIDE DOUBLE TOP TRACK DEFLECTION JOINT AT ALL WALLS WHICH EXTEND TO UNDERSIDE OF ROOF DECK ABOVE.
8. COORDINATE LOCATION OF EQUIPMENT AND PENETRATIONS WITH MECHANICAL, ELECTRICAL, AND PLUMBING DRAWINGS.
9. SEE EXTERIOR ELEVATIONS FOR IDENTIFICATION OF WINDOW TYPES AND SIZES.
10. HINGE SIDE OF DOOR JAMB LOCATED IN METAL STUD WALLS TO BE 4" FROM ADJACENT PERPENDICULAR WALLS, UNO.
11. ALL DOORS ARE DIMENSIONED TO THE CENTER LINE OF THE ROUGH OPENING.
12. VERIFY LOCATIONS OF FIRE EXTINGUISHERS WITH LOCAL FIRE DEPARTMENT OFFICIAL AND ARCHITECT PRIOR TO PLACEMENT, ALL EXTINGUISHERS TO BE RECESSED CABINET MOUNTED TYPE, UNO.
13. PROVIDE BLOCKING BEHIND ALL A/V EQUIPMENT, WALL SPEAKERS, CEILING MOUNTED PROJECTORS, HANDRAILS, ACCESSORIES, ETC. CONTRACTOR RESPONSIBLE FOR MISC STEEL, UNISTRUT, FRT WOOD BLOCKING, STRAPPING ETC AS REQUIRED TO FULLY SUPPORT EQUIPMENT AND ACCESSORIES AS INDICATED IN DOCUMENTS.
14. MOISTURE RESISTANT GYP BD TO BE USED ON WALLS AND CEILINGS OF ALL RESTROOMS AND KITCHENS.
15. PROVIDE LEVEL SURFACES AND PREP FLOOR TO RECEIVE SCHEDULED FINISH

FLOOR PLAN CODED NOTES

NOTE: ALL CODED NOTES MAY NOT OCCUR ON EVERY SHEET

1 4'-0" x 8'-0" OPEN TABLE FOR SAMPLING & SALES
2 WALL MOUNTED TV
3 LF DISPLAY CABINET
4 TALL CABINETS
5 DUMBWAITER THAT TRAVELS FROM THE BASEMENT TO FIRST FLOOR WITH UL415 SYSTEM A (1HR) SHAFT WALLS, BASIS OF DESIGN: INTELI-LIFT PRO 250 28x28x28 SAME SIDE WITH BI-PARTING GATE & SWING DOORS
6 NEW STAIR TO BASEMENT WITH CUSTOM DISPLAY MILLWORK
7 CONCEALED DOOR
8 EXISTING PILASTER TO BE FURRED OUT
9 DISHWASHER
10 LF DISPLAY CABINET SUSPENDED

INTERIOR STUD WALL SYMBOL KEY

A1

GYP. BOARD CONFIGURATION

1 - GYP. BOARD FULL HEIGHT,BOTH SIDES
2 - GYP. BOARD FULL HEIGHT,ONE SIDE
3 - GYP. BOARD TO 6" ABOVE FINISH CEILING, BOTH SIDES, NON-RATED
4 - 1" GYP BOARD TYPE SLX ON ONE SIDE, 5/8" GYP BOARD TYPE SCX ON OTHER SIDE
5 - 2 HR FIRE BARRIER
6 - 1 HR FIRE PARTITION

STUD WIDTH / TYPE DESIGNATION

A = 5 1/2" / WD	**E** = 9 1/4" / WD
B = 3 1/2" / WD	**F** = 11 1/4" / WD
C = 1 1/2" / WD	**G** = 2 1/2" CH MTL STUDS, 20 GA. @ 24" O.C.
D = 7 1/4" / WD	
H = RESERVED	

TYPES:
WD DIMENSIONAL LUMBER
MTL METAL STUDS
ST STRUCTURAL STUDS

SQUARE FOOTAGE

FLOOR	AREA
FOUNDATION	852
FIRST FLOOR	968
SECOND FLOOR	520
	2,340 ft²

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201 NORTH MAIN STREET, HUDSON, OHIO

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BULLETIN #2	2	01/02/24
BULLETIN #3	3	01/10/24
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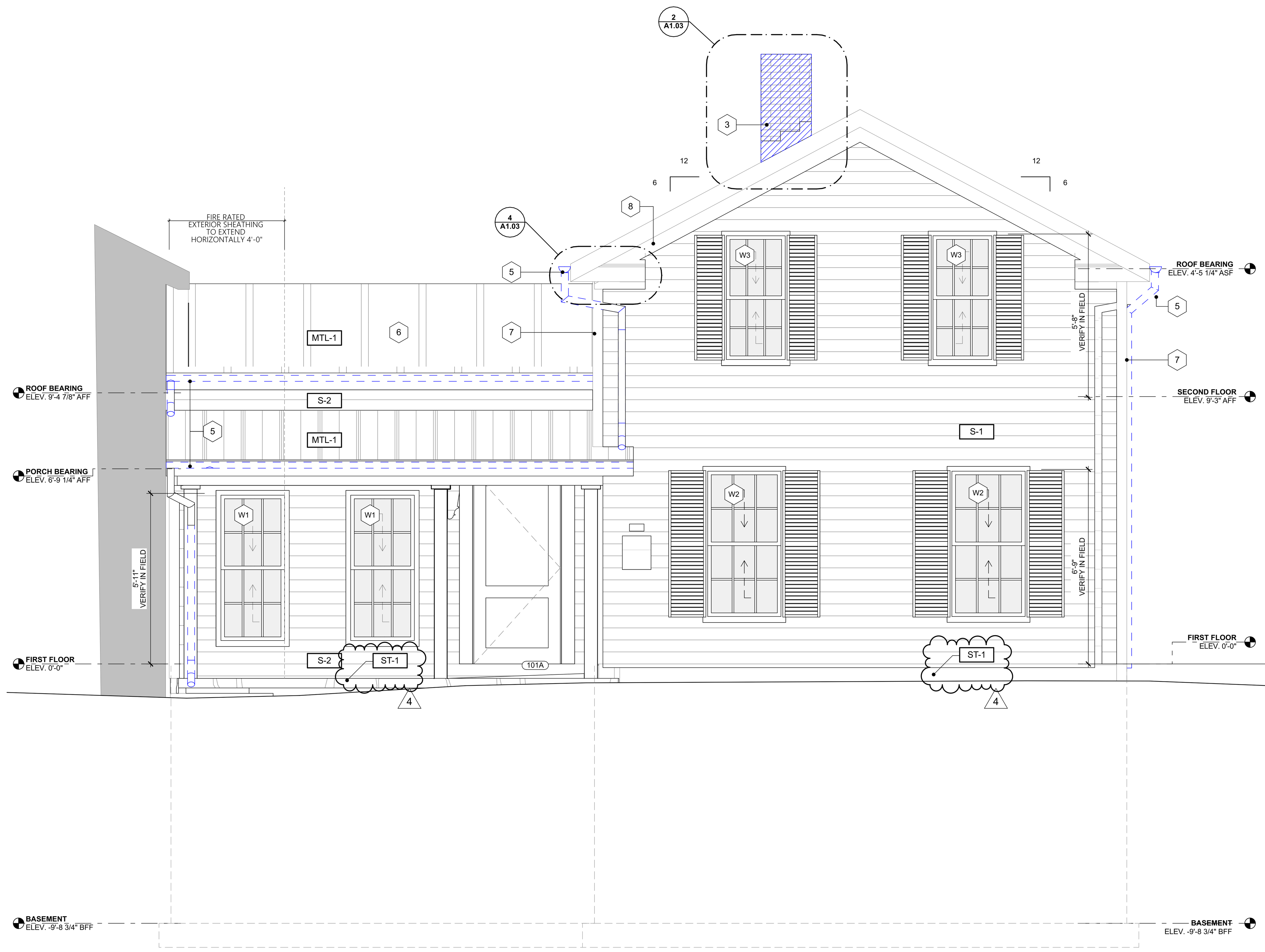
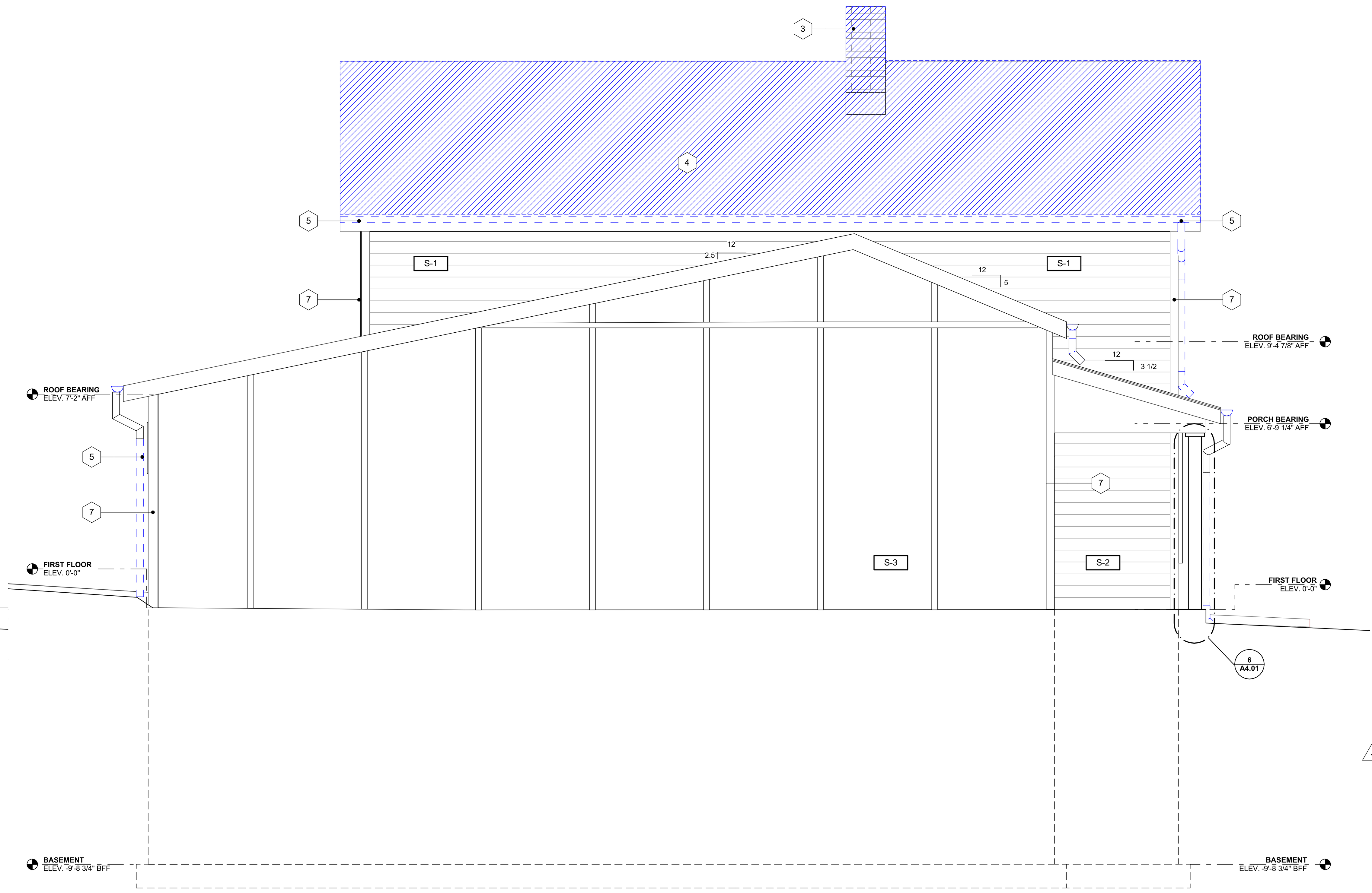
ROOF FLOOR PLAN

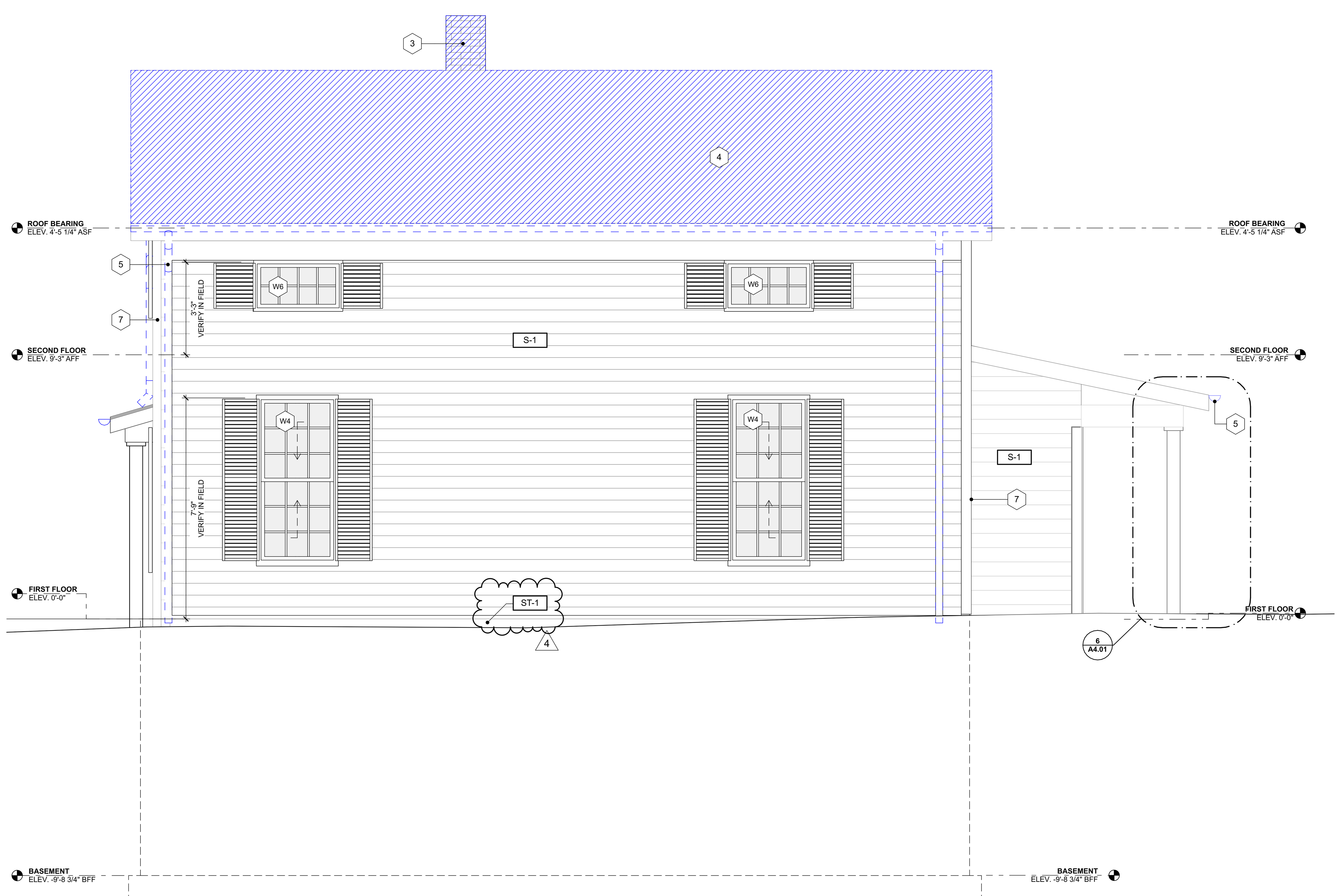
A1.03



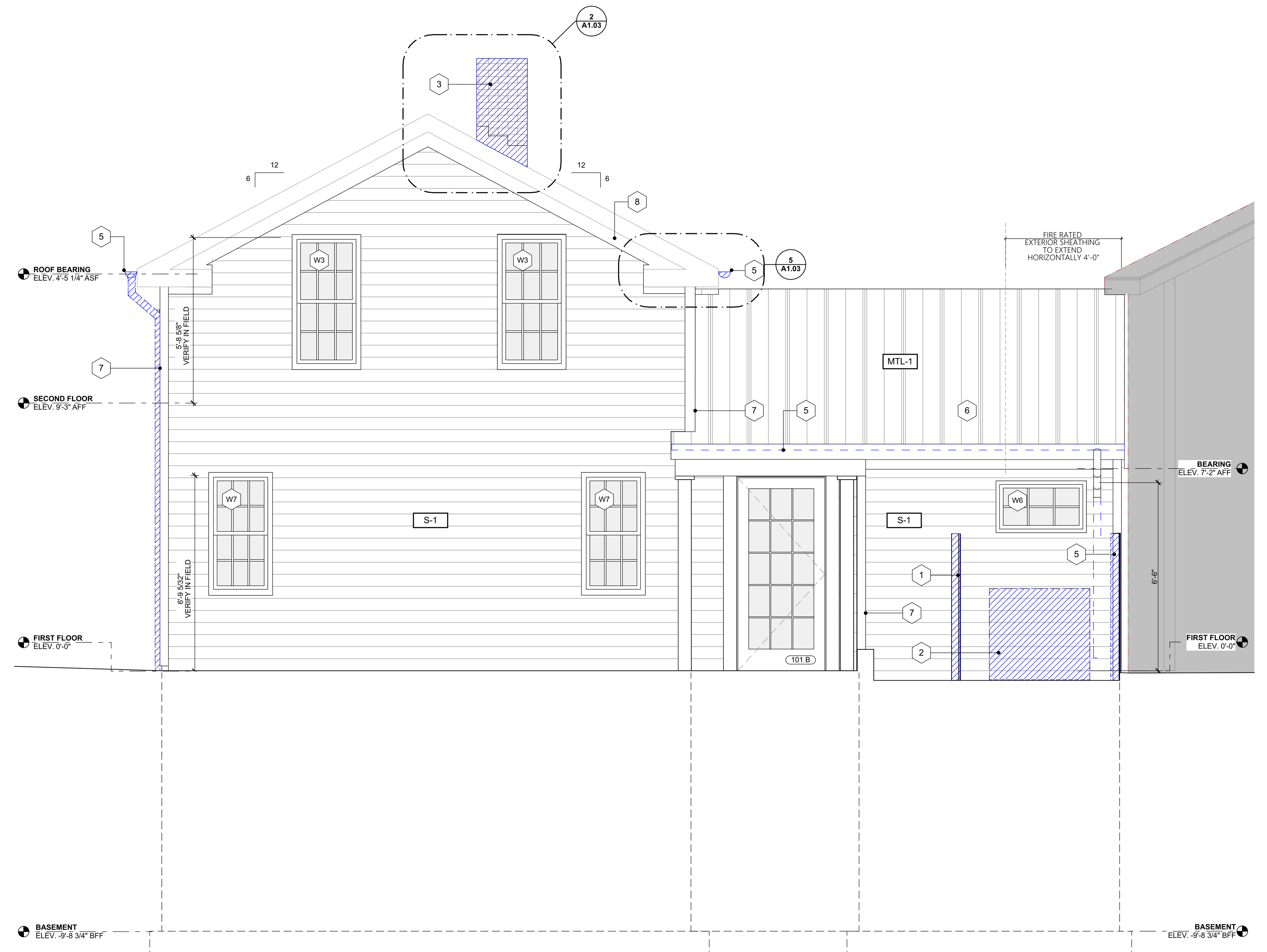
1 **EXTERIOR ELEVATION - WEST (FRONT OF THE BUILDING)**
SCALE: 3/8" = 1'-0"



2 **EXTERIOR ELEVATION - NORTH (SIDE OF THE BUILDING)**
SCALE: 3/8" = 1'-0"



3 **EXTERIOR ELEVATION - SOUTH (SIDE OF THE BUILDING)**
SCALE: 3/8" = 1'-0"



4 **EXTERIOR ELEVATION - EAST (BACK OF THE BUILDING)**
SCALE: 3/8" = 1'-0"

GENERAL ELEVATION NOTES

REFER TO EXTERIOR WALL TYPES LEGEND BELOW FOR TYPICAL WALL CONSTRUCTION INFORMATION. HORIZONTAL EXTENTS OF EXTERIOR WALL CONSTRUCTION ARE INDICATED ON THE FLOOR PLAN

ROOF SOFFITS TO BE AC PLYWOOD WITH A CONTINUOUS LINEAR VENT UNLESS NOTED OTHERWISE.

ALL WOOD EXTERIOR TRIM TO BE PAINTED.

ALL ROOF PENETRATIONS TO BE COORDINATED WITH ARCHITECT PRIOR TO INSTALLTION TO ENSURE AESTHETIC EXPECTATIONS ARE MAINTAINED.

ALL FOUNDATIONS TO BE PARGED ABOVE GRADE.

GUTTER PROFILES SHALL BE SUBMITTED FOR APPROVAL PRIOR TO ORDERING

ELEVATION KEYNOTES

1. NEW WOOD PRIVACY FENCE, PAINTED OR STAINED
2. NEW HVAC AND GENERATOR
3. NEW FAUX BRICK CHIMNEY TO MATCH HISTORIC. CHIMNEY TO BE INSTALLED IN HISTORIC LOCATION. 4
4. EXISTING SLATE SHINGLE ROOF TO REMAIN
5. NEW HALF ROUND COPPER GUTTERS AND DOWNSPOUTS
6. NEW METAL STANDING SEAM ROOF TO MATCH EXISTING HISTORIC ROOF
7. 1X4 WOOD CORNER BOARDS
8. 1X6 WOOD RAKE TRIM, PAINT TO MATCH TRIM

MATERIAL SCHEDULE

ALL MATERIAL ASSEMBLIES LISTED BELOW TO OCCUR OVER THE FOLLOWING UNLESS NOTED OTHERWISE:

O/ WEATHER BARRIER
O/ APA RATED EXTERIOR GRADE OSB SHEATHING
O/ 2x6 STUDS W/ R-19 BATT INSULATION

4 **(S-1) - WOOD SIDING**
WESTERN RED CEDAR LAP SIDING COORDINATE EXPOSURE WIDTH W/ ARCHITECT & OWNER TO MATCH HISTORIC EXPOSURE. LAP SIDING IS TO BE PRIMED ON ALL FOUR SIDES, PAINTED, COORDINATE FINAL COLOR WITH ARCHITECT AND OWNER.

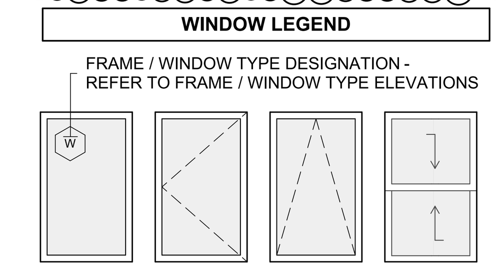
(S-2) - WOOD SIDING
FLUSH BOARD CEDAR SIDING IS TO BE PRIMED ON ALL FOUR SIDES, PAINTED, COORDINATE FINAL COLOR WITH ARCHITECT AND OWNER.

(S-3) - FIBER CEMENT SIDING
HARDIE FIBER CEMENT SIDING, COORDINATE FINAL COLOR WITH ARCHITECT AND OWNER.

(MTL-1) - STANDING SEAM METAL ROOF
METAL STANDING SEAM ROOF, NO STRIATIONS OR RIBS, TO MATCH EXISTING PROPORTIONS O/ MANUFACTURER'S RECOMMENDED UNDERLAYMENT O/ 19/32" APA RATED EXTERIOR GRADE SHEATHING. ICE GUARD SHOULD BE INSTALLED AT ALL EAVES AND VALLEYS, UP 72", AND WRAPPED OVER THE FACE OF ALL FASCIAS.

4 **(ST-1) - STONE SIDING**
SALVAGED SANDSTONE VENEER

WINDOW LEGEND



FIXED | CASEMENT | AWNING | DOUBLE HUNG

4 **BASIS OF DESIGN:**
ALL WINDOWS ARE TO BE PELLA RESERVE - TRADITIONAL, DOUBLE HUNG - OR APPROVED EQUAL. PROVIDE NEW PAINTED WOOD SHUTTERS.

WINDOW HEAD GIVEN ABOVE FIRST AND SECOND FLOOR, CONFIRM WINDOW QUANTITIES WITH ELEVATIONS.

**** WINDOW COUNT PROVIDED FOR GENERAL REFERENCE AND VERIFICATION ONLY. CONTRACTOR TO VERIFY TOTAL NUMBER OF WINDOWS WITH DOCUMENTS.**

WINDOW SCHEDULE REMARKS

1. WINDOWS INDICATED WITH ZERO SEPARATION HAVE MULLED FRAMES. REFER TO EXTERIOR ELEVATIONS FOR CONFIGURATIONS AND QUANTITY.
2. PROVIDE PRIVACY WINDOW FILM AT ACCESSIBLE RESTROOM ON FIRST FLOOR.

WINDOW SCHEDULE

ID	W x H	OPERATION	EGRESS	REMARKS
W1	2'-4"×5'-2"	DOUBLE HUNG	---	
W2	2'-8"×5'-2"	DOUBLE HUNG	---	
W3	2'-2"×4'-5"	DOUBLE HUNG	---	
W4	2'-8"×5'-9"	DOUBLE HUNG	---	
W6	3'-0"×1'-8"	FIXED	---	
W7	2'-0"×4'-0"	DOUBLE HUNG	---	

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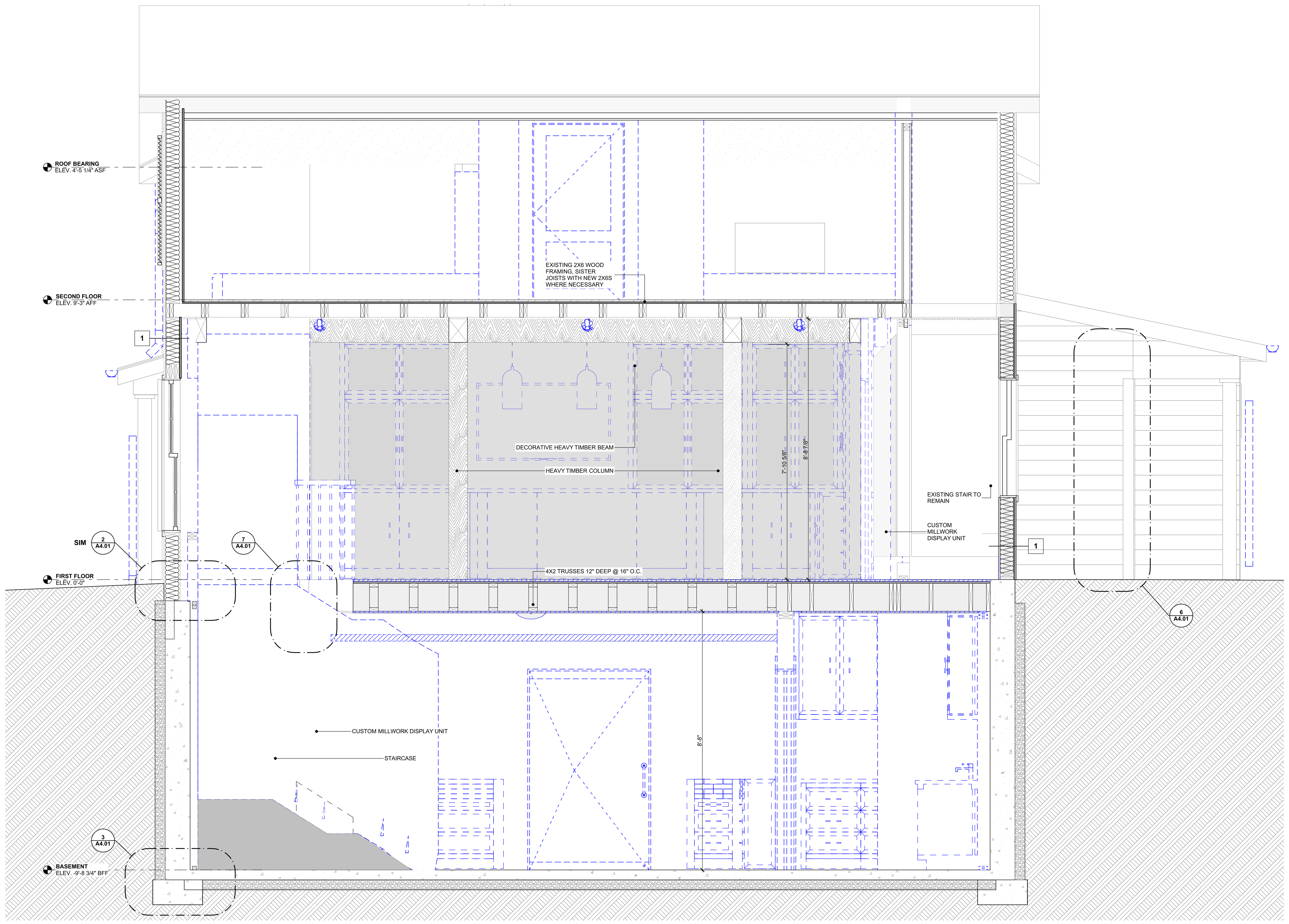
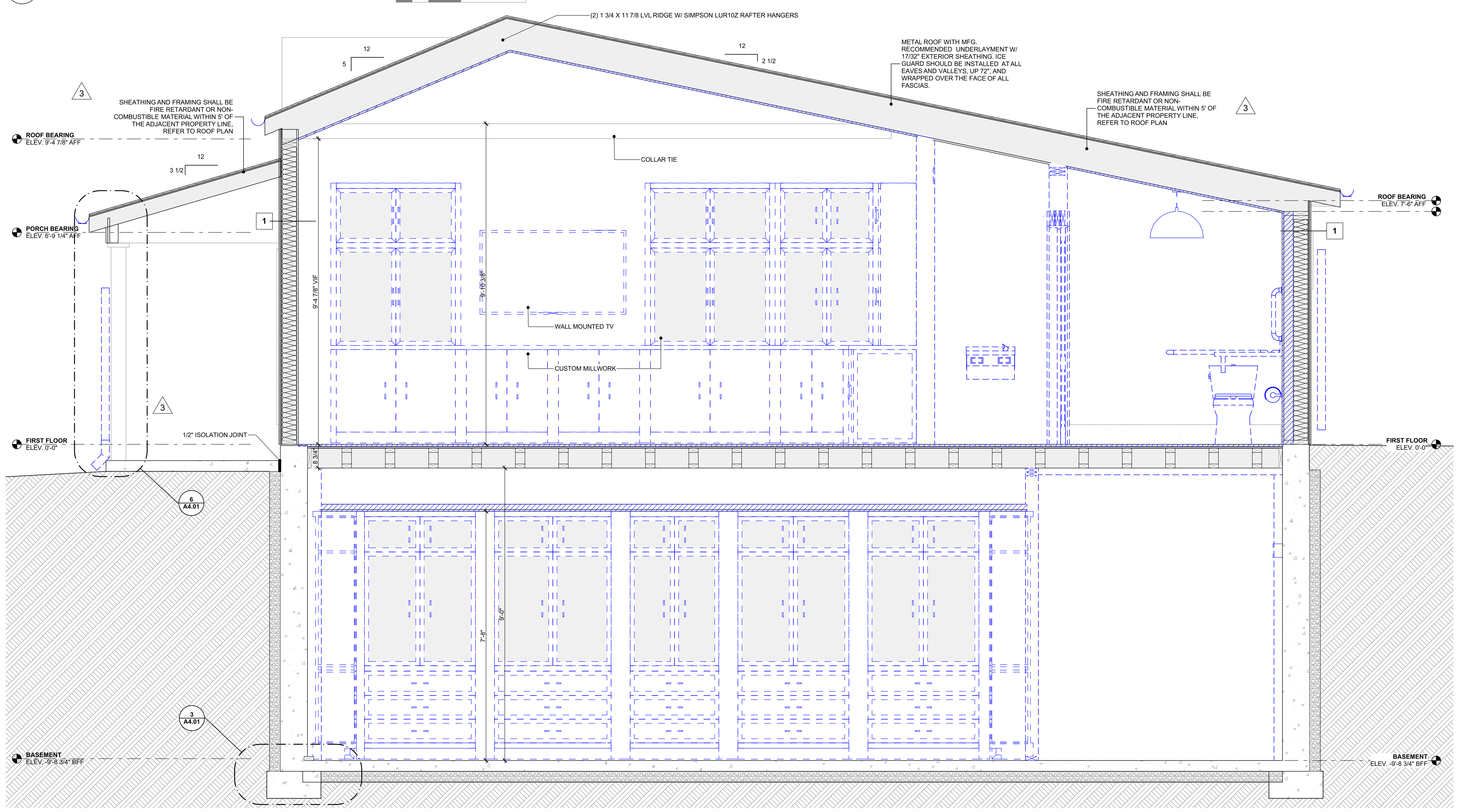
EXTERIOR ELEVATIONS

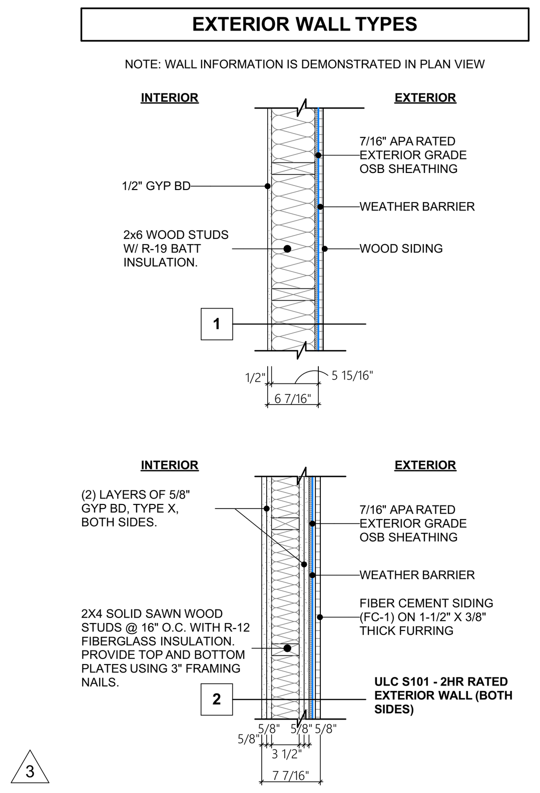
A3.01

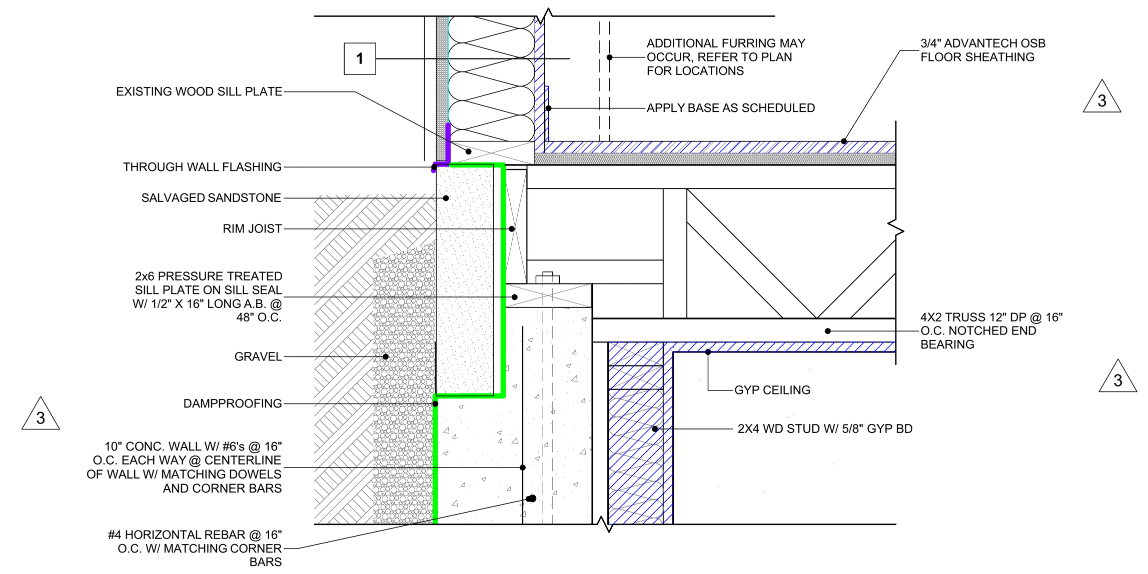


1 BUILDING SECTION
SCALE: 1/2" = 1'-0"

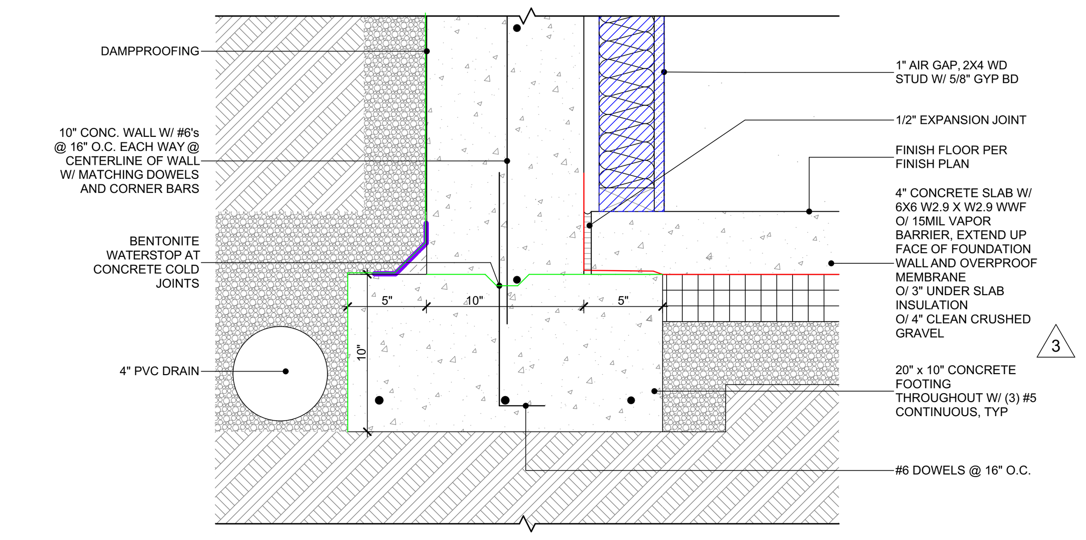


4 BUILDING SECTION
SCALE: 1/2" = 1'-0"

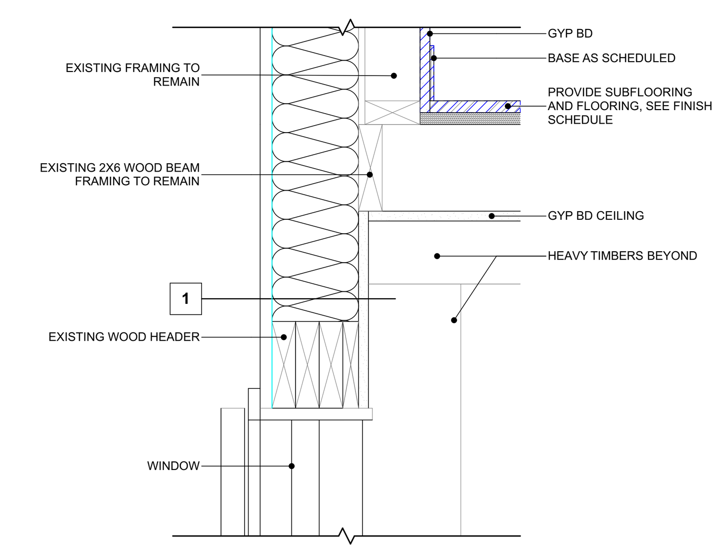




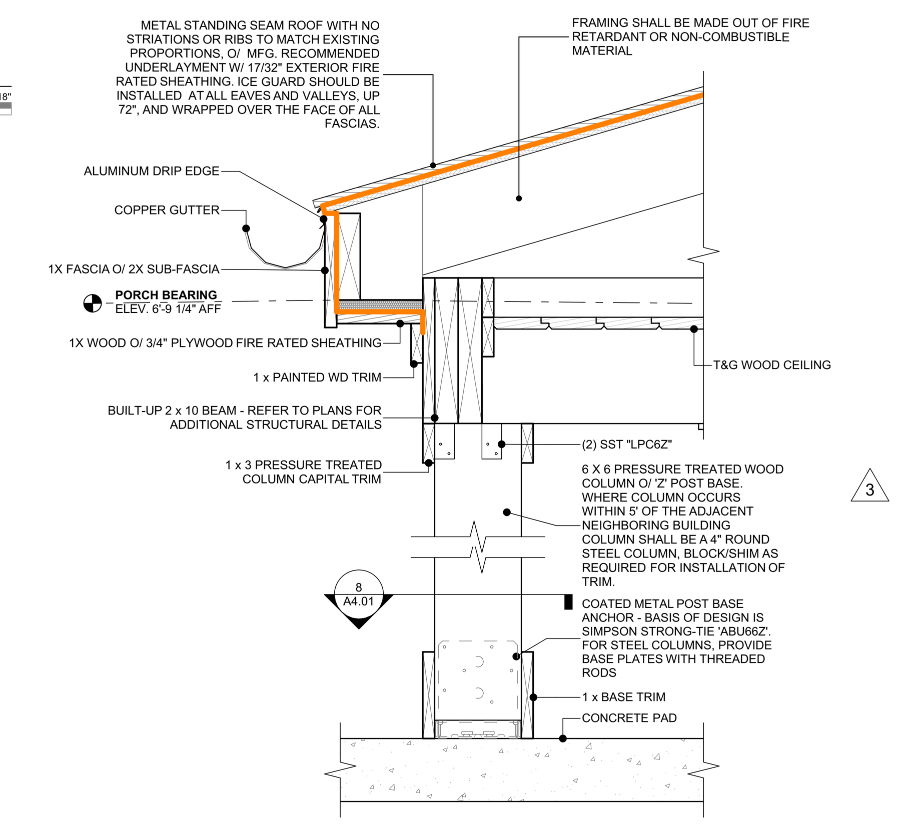
2 DETAIL
SCALE: 1 1/2"= 1'-0"



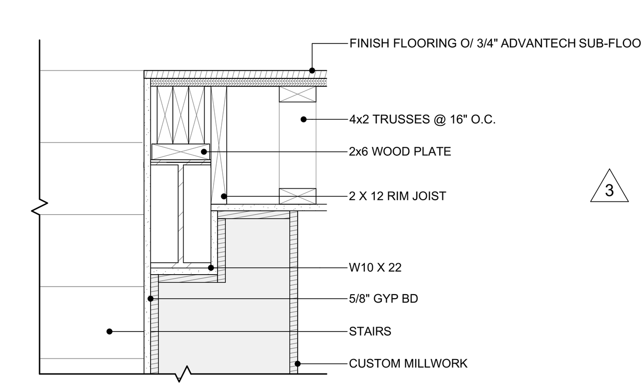
3 FOUNDATION DETAIL
SCALE: 1 1/2"= 1'-0"



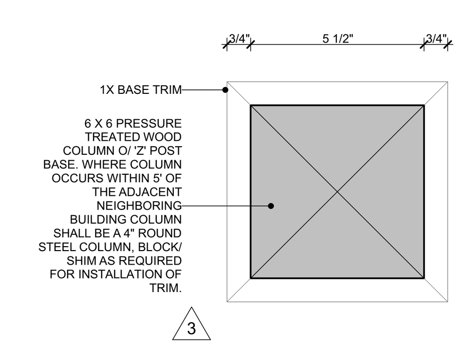
5 DETAIL
SCALE: 1 1/2"= 1'-0"



6 COLUMN DETAIL
SCALE: 1 1/2"= 1'-0"



7 BEAM SECTION DETAIL
SCALE: 1" = 1'-0"



8 COLUMN PLAN DETAIL
SCALE: 3" = 1'-0"

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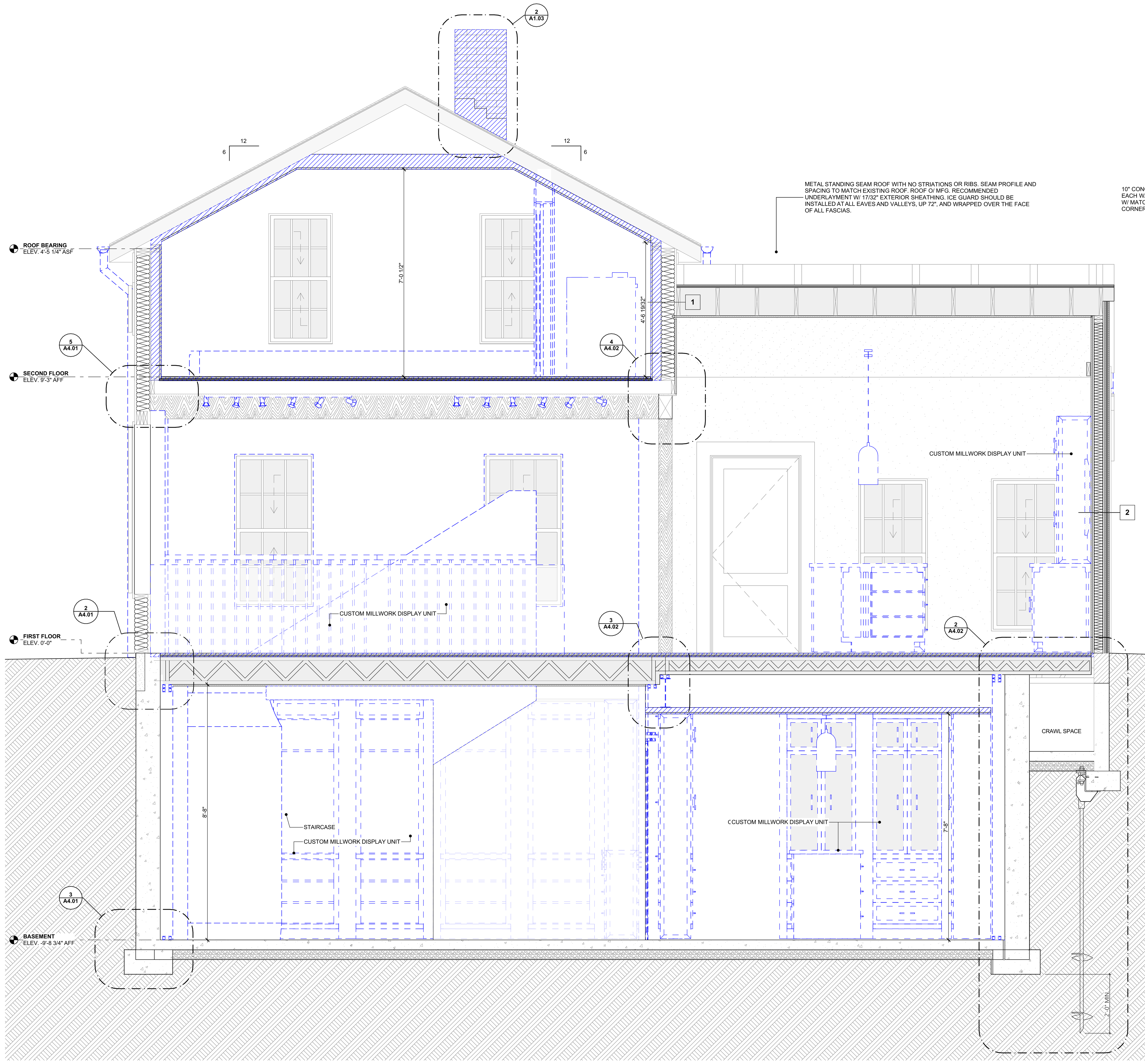
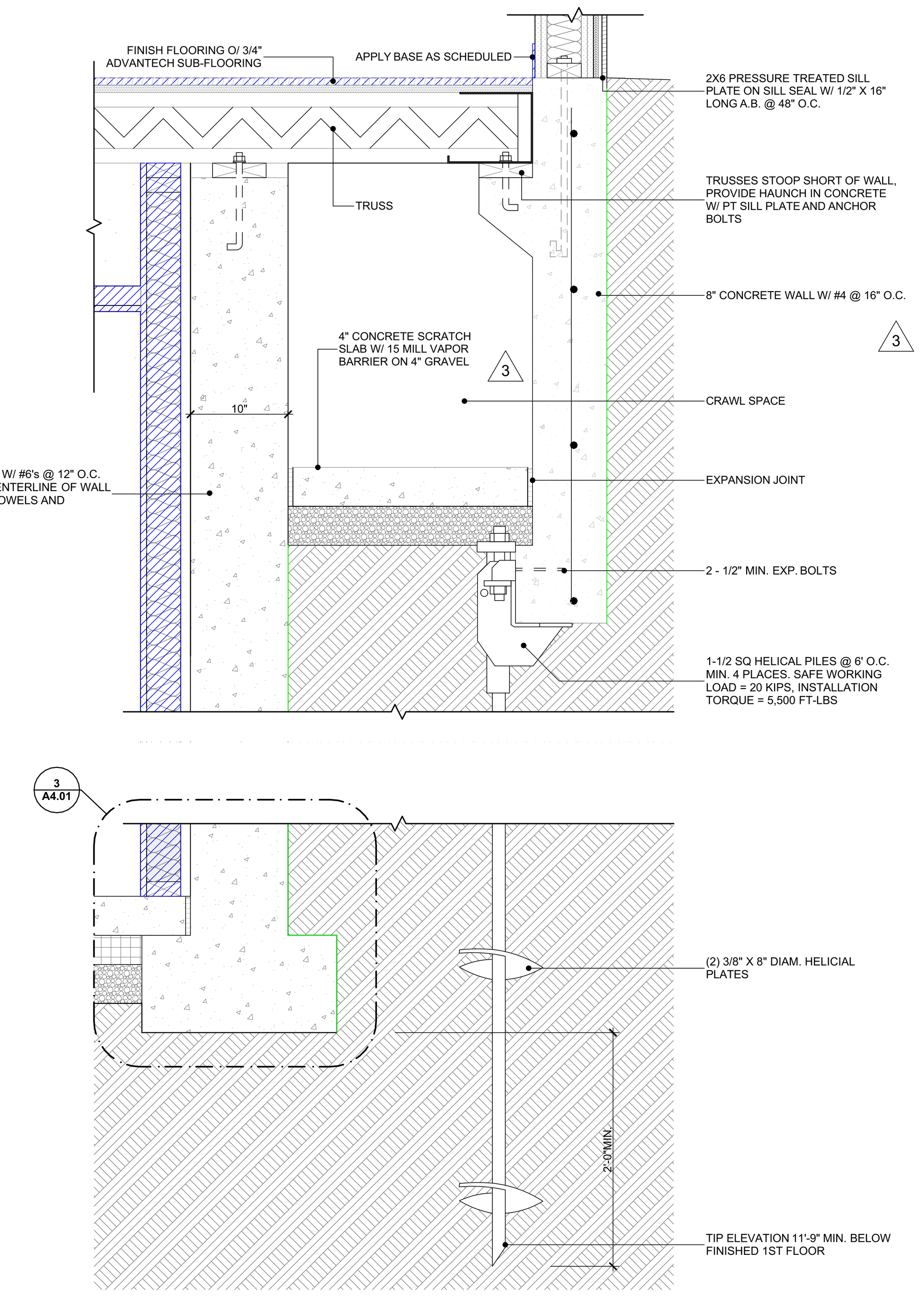
BUILDING SECTIONS

A4.01



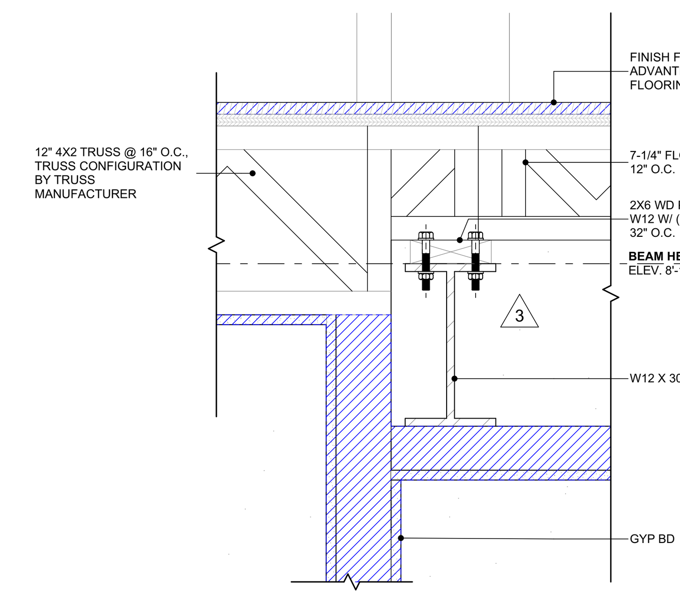
1 BUILDING SECTION

SCALE: 1/2" = 1'-0"



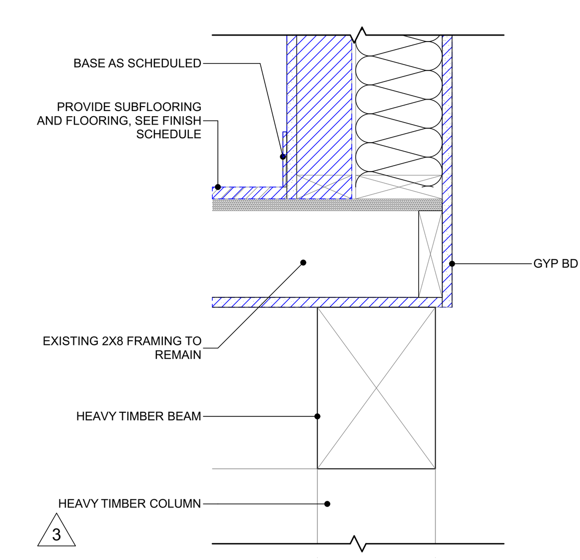
2 DETAIL - EXISTING FOUNDATION

SCALE: 1" = 1'-0"



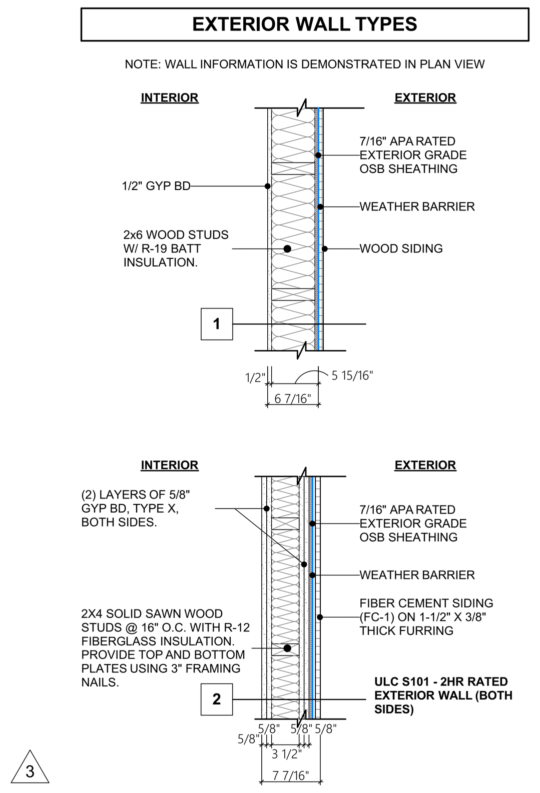
3 DETAIL - DROPPED CEILING

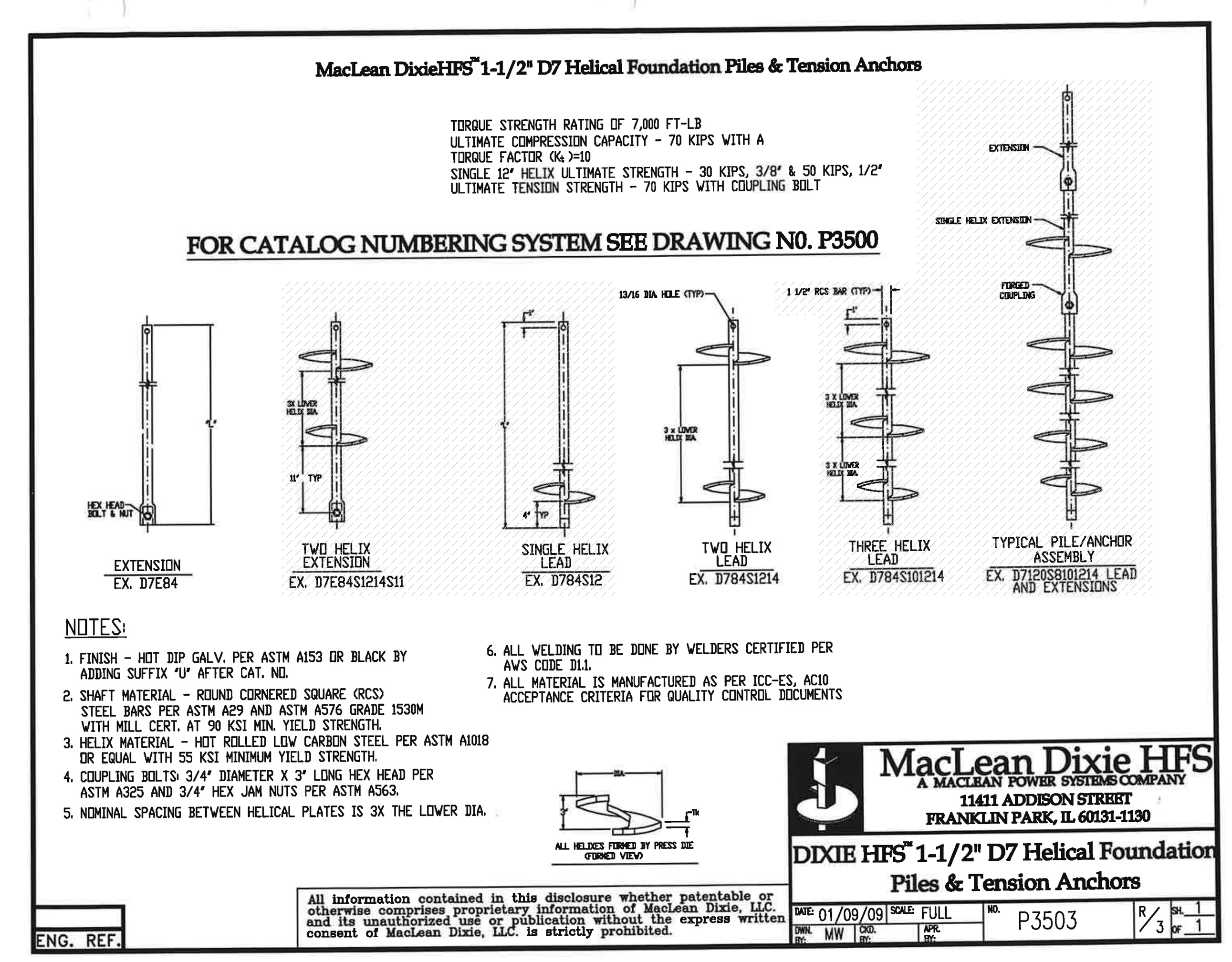
SCALE: 1 1/2"= 1'-0"



4 DETAIL

SCALE: 1 1/2"= 1'-0"





NOTE: MACLEAN DIXIE SHOWN AS BASIS OF DESIGN - CHANCE, RAM-JACK OR OTHER ALTERNATES ARE ACCEPTABLE. SUBMIT DATA PRIOR TO INSTALLATION FOR APPROVAL

SECTION 12

The values of K_t shown below are intended to assist the designer in selecting the proper shaft size for his particular application. Larger values of K_t can provide greater geotechnical pile capacities, but Design Loads should not induce: (1) stresses into the shaft which exceed one-half of the Yield Strength shown in Table 12.1 or (2) Helix loads exceeding the values shown in Table 12.3.

Table 12.2
RATED MAXIMUM DESIGN LOADS (ALLOWABLE LOADS) – BASED ON INSTALLATION TORQUE *

Shaft Size	Designation	K_t	Rated Torsional Capacity (ft-lbs)	Ultimate Capacity		Maximum Design Load	
				Tension (lbs)	Compression (lbs)	Tension (lbs)	Compression (lbs)
1.25" Square Shaft	D3	10	3,400	34,000	34,000	17,000	17,000
1.50" Square Shaft	D6	10	5,500	60,000	55,000	30,000	27,500
1.50" Square Shaft (high str.)	D7	10	7,000	70,000	70,000	35,000	35,000
1.75" Square Shaft	D10	10	10,000	100,000	100,000	50,000	50,000
2.00" Square Shaft	D15	10	15,000	150,000	150,000	75,000	75,000
2.875" O.D. (0.203" wall)	P28	8	7,500	80,000	60,000	40,000	30,000
2.875" O.D. (0.276" wall)	P28H	8	9,000	100,000	72,000	50,000	36,000
3.500" O.D. (0.216" wall)	P35	7	11,400	100,000	80,000	50,000	40,000
3.500" O.D. (0.300" wall)	P35H	7	15,000	140,000	105,000	70,000	52,500
4.500" O.D. (0.237" wall)	P45	6	20,000	140,000	120,000	70,000	60,000
4.500" O.D. (0.337" wall)	P45H	6	26,000	200,000	156,000	100,000	78,000
8.625" O.D. X 0.1875" wall	P8	4.5	44,500	240,000	200,000	120,000	100,000**

* Maximum Design Loads (Allowable Loads) are based on a Factor of Safety of 2 (See Section 5). Column buckling considerations are not addressed (See Section 9).
* Loads are axial.
* Ultimate tension is based on mechanical strength of pipe and coupling.
* Ultimate Compression Capacity (lbs) = ultimate helix/soil bearing capacity (lbs) = K_t x Torque (ft-lbs), (See Sec.13).
* Rated maximum design loads assume that the torsional capacity of the pile has been achieved.
* If the soil's bearing capacity beneath the helices is greater than that above the helices, the geotechnical compression capacity of the pile will be greater, but design loads should not induce: (1) stresses into the shaft which exceed one-half of the Yield Strength shown in Table 12.1 or (2) Helix loads exceeding the values shown in Table 12.3.

** Depending on pile depth and type of soil, skin friction (after soil remolds around the pipe) will increase the total capacity of the P45 and P8 Pipe Piles.
** A concrete or grout filled P8 Pipe Pile may significantly increase its capacity. P8 Pipe Piles have been successfully load tested to 180,000 lbs. Unless otherwise noted, the P8 Pipe Pile should be filled with suitably reinforced concrete or grout of sufficient strength after installation.

B. Square Shaft or Round Pipe Shaft?

Square shafts are more efficient than round pipe. When installed to the same torque, the capacity of a square shaft anchor will be greater than that of a round shaft anchor. (This is discussed in Section 13.) In addition, because of the small cross sectional area, square shafts can penetrate into denser soils (higher Standard Penetration Test "N" values) than round pipe sections. This is especially true in dense sands and gravel.

Square shafts are generally the best choice for new and existing construction unless:

1. Significant lateral (shear) loads are expected. (i.e. bending stresses which exceed the shaft's allowable yield stress or lateral loads, which exceed the pile/soil capacity.) See Section 10.

Helical Foundations Systems Engineering Manual
Issued: April 1, 2010
Supercedes: June 1, 2006

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BUILDING SECTIONS

A4.02

201 N Main Street Retail

<u>Project Narrative</u>

The historic building at 201 North Main Street in Hudson, Ohio was built in 1840 and has served many functions over the years ranging from a family dwelling to most recently a dental office. With each use, the building was augmented to serve the needs of the previous owners. We are proposing to renovate and restore the building to existing historic footprint and create a new retail space with ADA accessibility.

A historic analysis was performed by Dr. Elwin Robison and he determined that the original building only consisted of the two story portion. Everything else surrounding the structure was added at later points in time. A structural analysis of the building determined that the foundation walls are unstable and termites have left first floor framing in unusable condition.

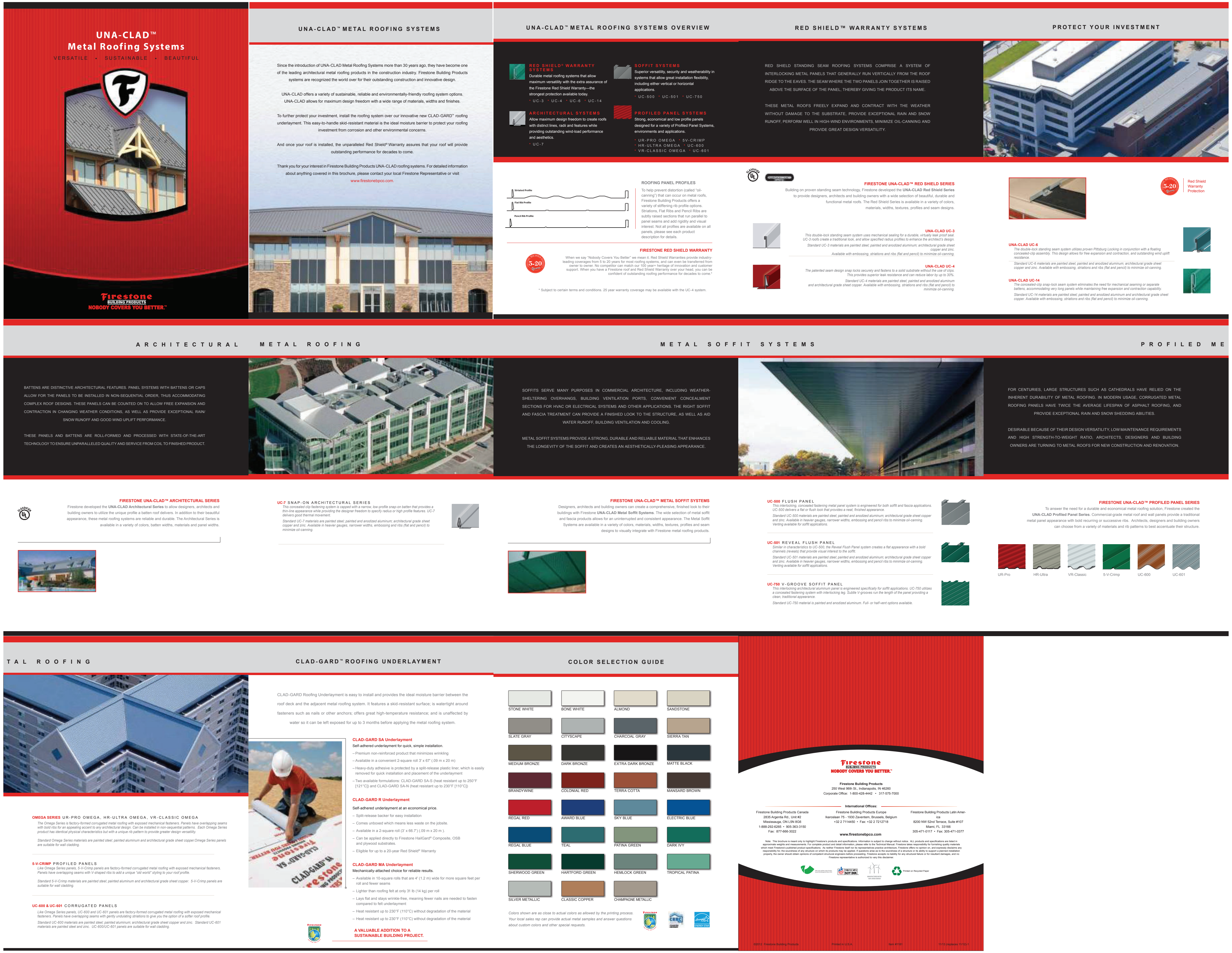
We are proposing to remove all non-historic portions of the building including the single story "ell" and later additions on the rear. The historic two-story building will be stabilized and lifted off the foundations to allow for new foundation and first floor deck. Upon completion, the historic two-story building will be lifted and reinstalled on the new foundation. Prior to lifting the building, the exact location of the building foundation will be field measured for reinstallation accuracy.

After the building is reinstalled in its original location, the single story "ell" located on the North of the building will be rebuilt to match the existing conditions on the west façade. Although this addition was added later to the historic building, we feel that this part of the facade is integral to the historic fabric of North Main Street.

The existing exterior siding will be removed on the entirety of the building and be replaced with new wood (western red cedar) siding, trim, and corner boards that match the historic siding exposure and dimensions. The existing windows and shutters are also to be replaced with wood frames with 6 over 6, double hung windows that match the existing openings and restore the original style of window. The existing slate roof on the two-story portion of the building is to remain and a new metal standing seam roof over the west porch will be installed. New gutters and downspouts will be half round copper to match the existing.

A new accessible entry and sitting area will be installed on the east side. The first floor will be renovated into a new retail space with an accessible restroom. The second floor will provide an office space. A small patio will be created on the east side of the building and the existing parking lot will be repaved and restriped.

Material Specifications



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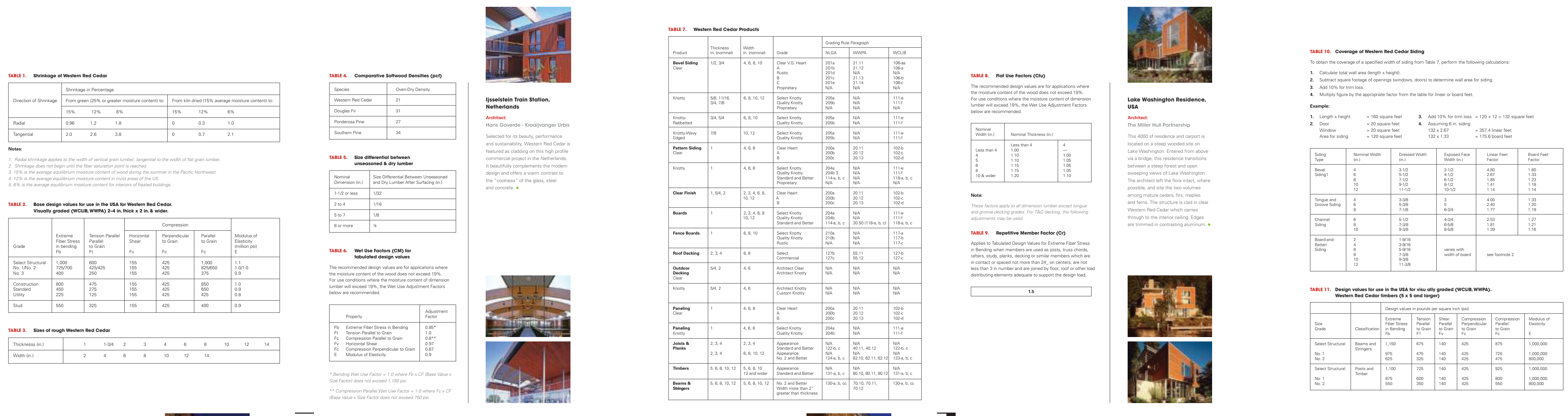

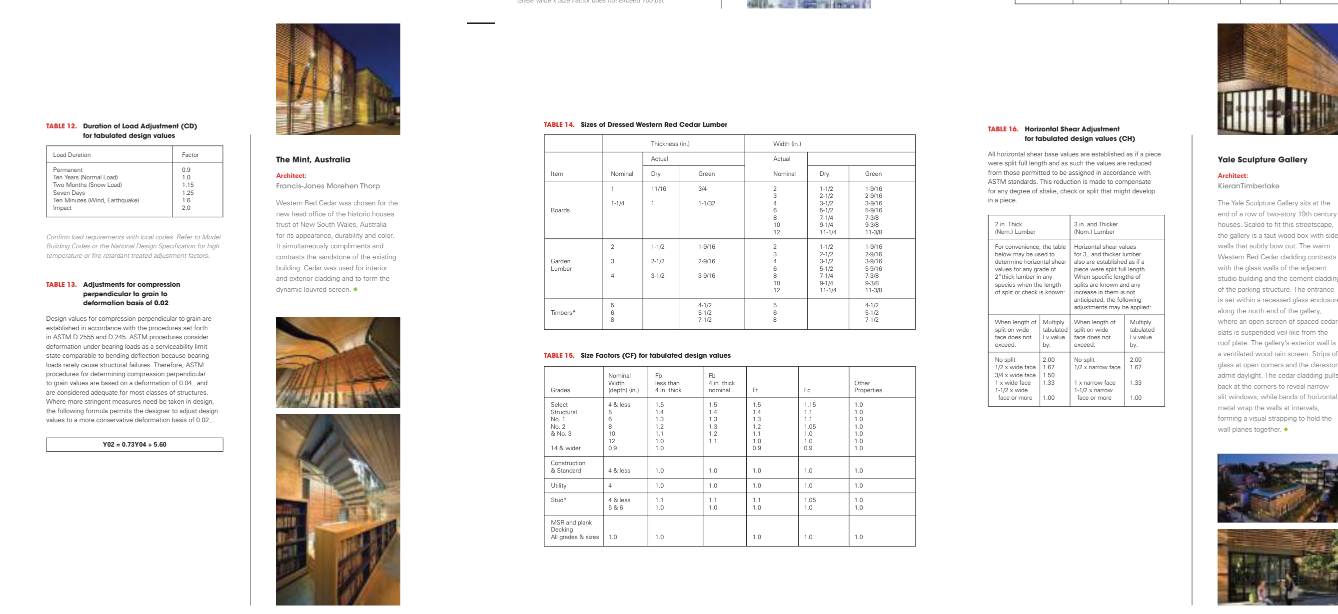

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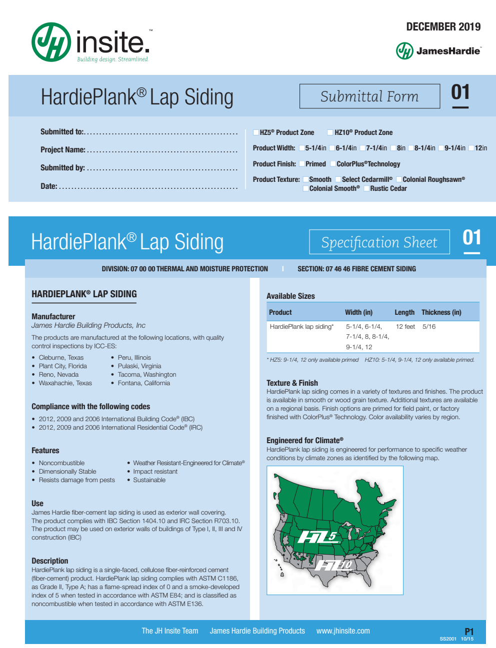
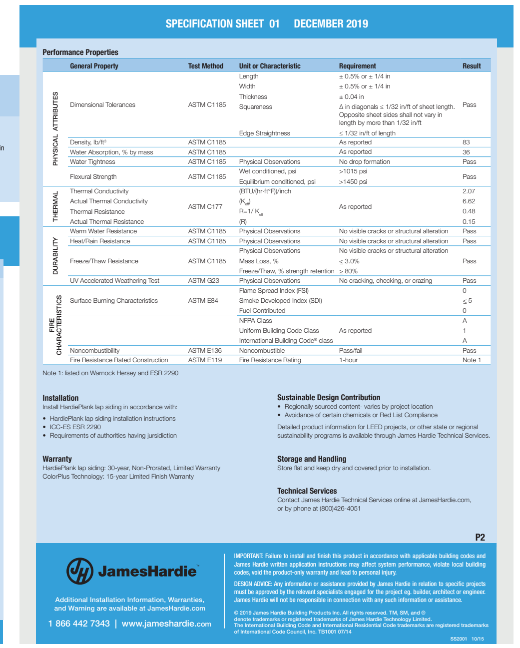

DECEMBER 2019

insite.

JamesHardie

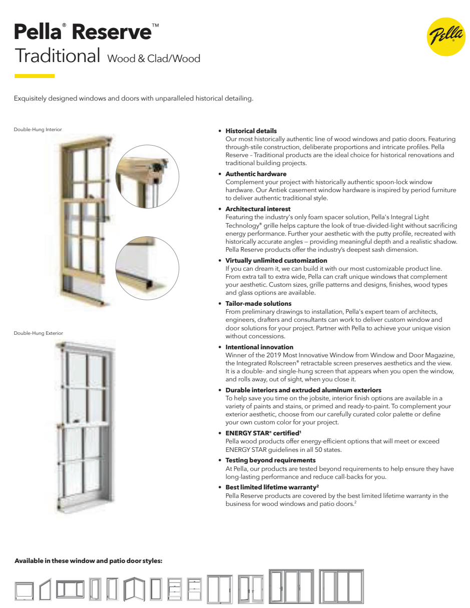
HardiePlank® Lap Siding Submittal Form 01

HardiePlank® Lap Siding Specification Sheet 01

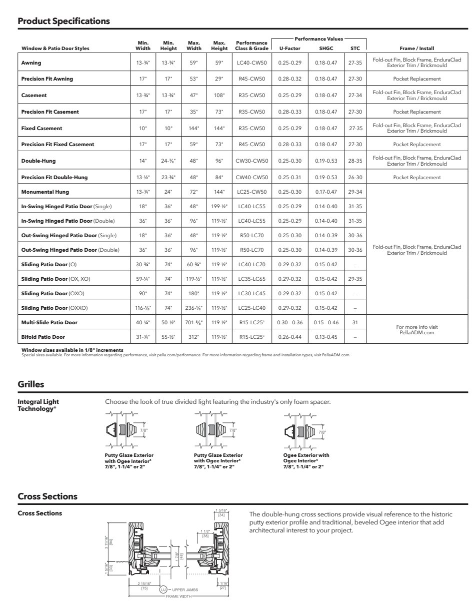

SPECIFICATION SHEET 01 DECEMBER 2019

Performance Properties

JamesHardie

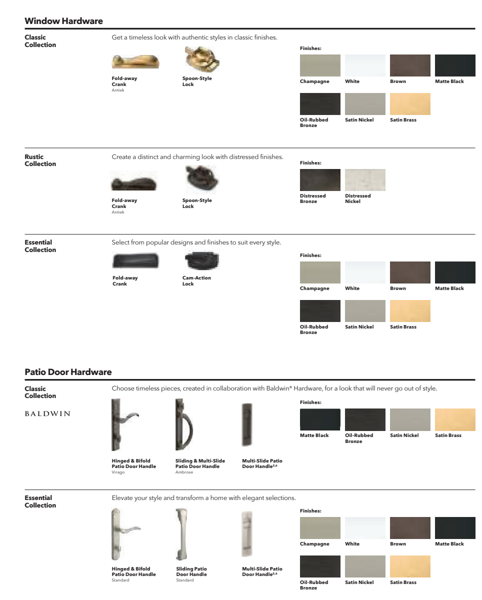

Pella® Reserve™

Traditional Wood & Clad/Wood


Product Specifications

Grilles

Cross Sections


Window Hardware

Patio Door Hardware


Colors

Added Peace of Mind

EXISTING FACADE PHOTOGRAPHS

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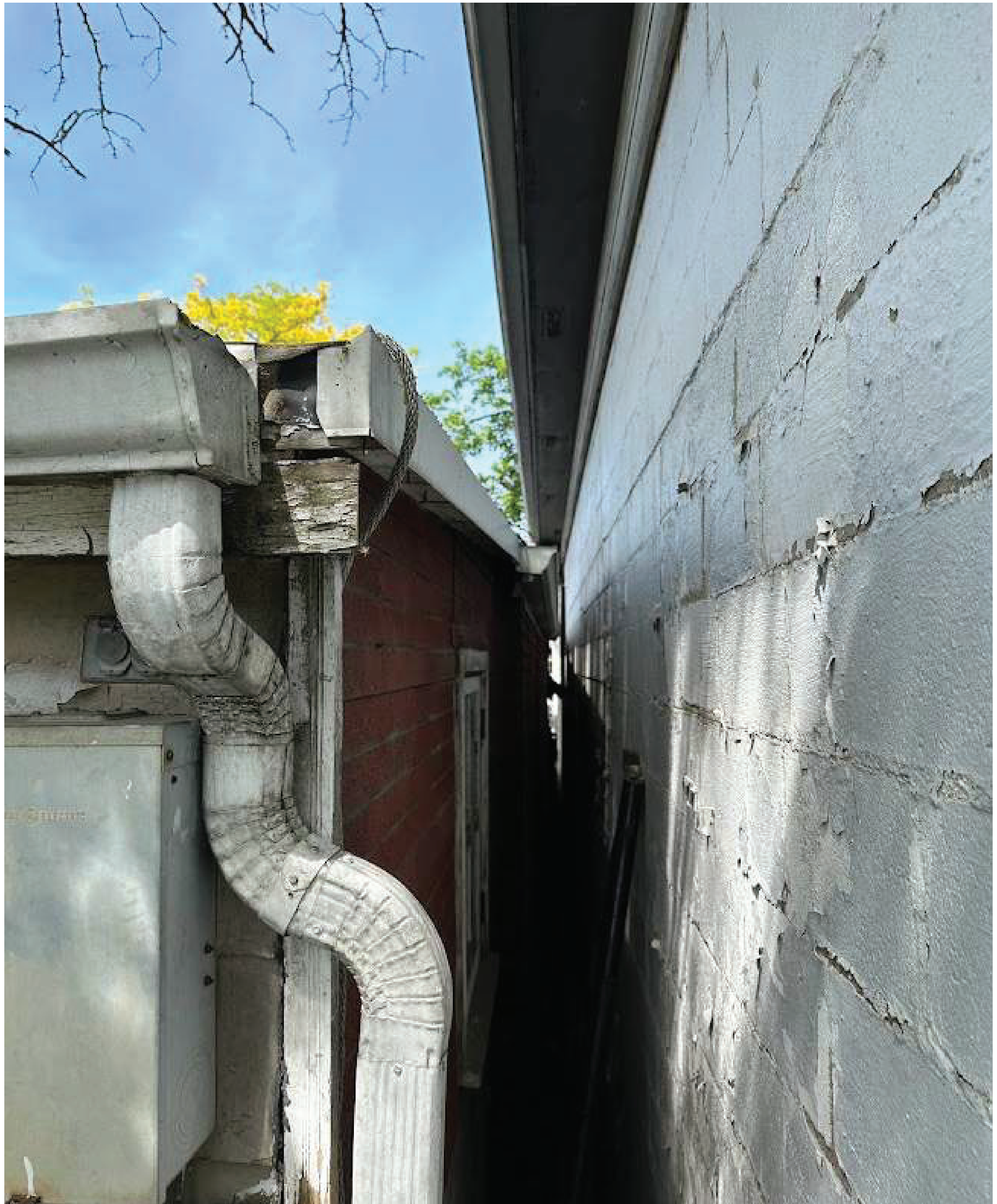

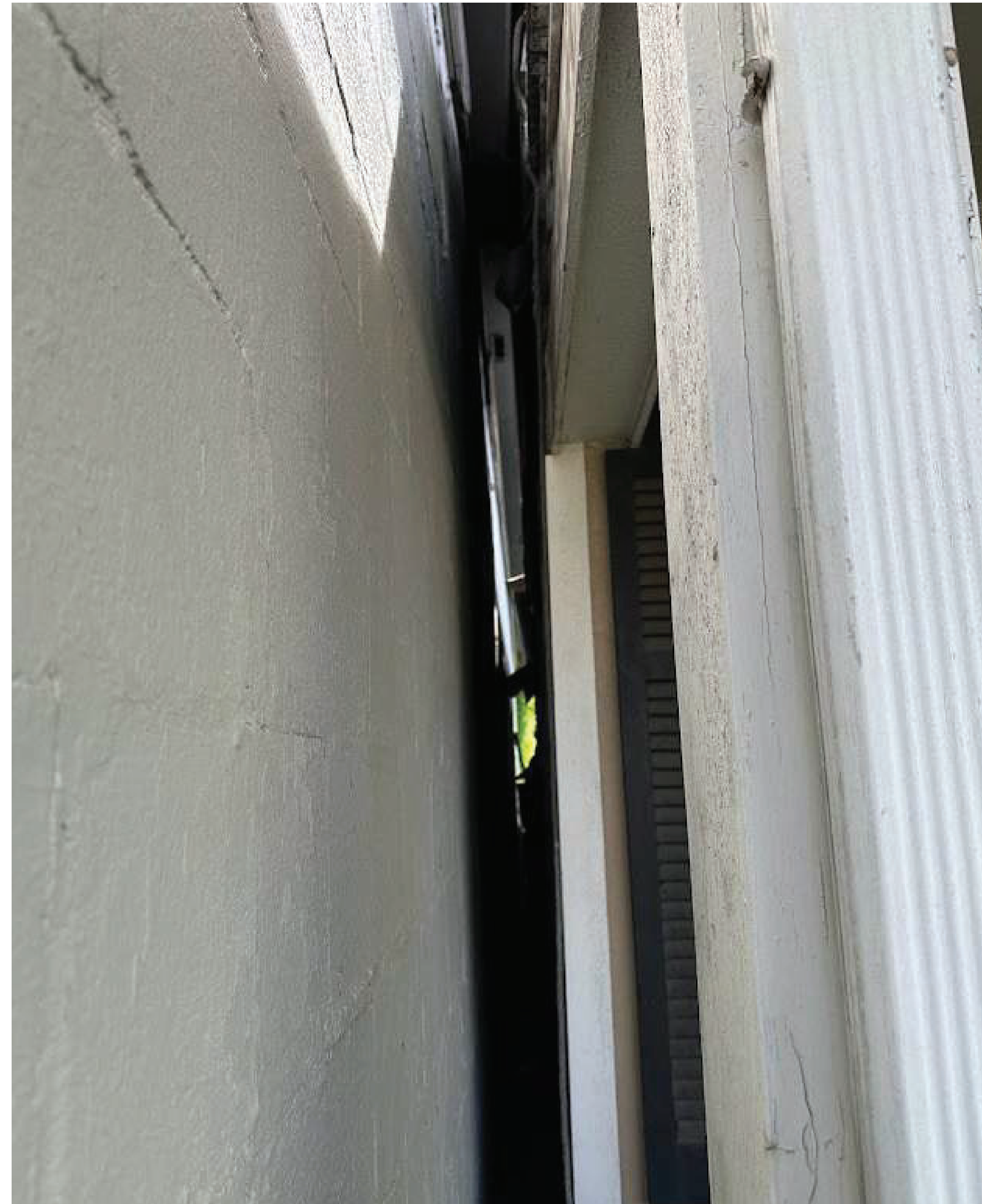

EXISTING FACADE PHOTOGRAPHS

EXISTING FACADE PHOTOGRAPHS

Peninsula

DATE: 10/03/2023

North Main Retail
201 North Main Street
Hudson, OH 44236

1) Contractor information. Additionally it would be helpful to state any experience with historic properties.
The General Contractor will be Jordan Snedeker of Villa Custom Builders. Jordan has been in the construction business for over 20 years having previously worked on large scale residential remodels of century homes and historical buildings/homes. Jordan has extensive experience with maintaining the integrity of the original design and craftsmanship. Two local projects that Jordan was the primary contractor on included the revitalization of 5 Aurora Street (Bank Building) and 233 College Street, the John William Creswell Corbusier House.

2) Construction timeline detailing the process of moving the building.
It should take about 8-9 weeks to move the building and replace it on the new foundation.

3) Protection plan for historic materials through the process
The only historic material remaining is framing and the slate roof. All historical framing will be protected and kept intact during the building move process. No slate nor framing is slated for demolition.

4) An explanation on how you would document historic materials through process, especially after major events such as moving the structure.
Pictures will be taken to document the moving of the building as well as any historic framing that will be covered by finished walls.

As a related case study, a few photos of a previously completed project in the Cuyahoga Valley National Park are attached. This project is a very similar scale building, of the same building typology, and used the same building mover. The effort to lift the building in order to accommodate a new foundation was successful.


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Peninsula

201 North Main Street

DATE: 10/25/2023

201 North Main Street
Hudson, Ohio 44236

Response Letter for
Architectural and Historic Board of Review

Change #1 is not appropriate because it will remove what appears to be historic materials from the site. The Secretary of the Interior Standards (SIS), #4, states that "changes to a property that have acquired historic significance in their own right will be retained and preserved." Applicant to submit evidence that the addition was constructed outside the period of significance, which ends in 1963. Further, evidence needs to be provided demonstrating why the demolition of what appears to be a historic portion and historic fabric of the building needs to be removed and cannot be lifted, moved, and reset along with the two-story portion.
Response: The existing foundation and first floor framing need to be replaced due to extensive termite damage.

Per the attached email from the building lifting company, Dingey Movers Inc, they cannot safely salvage the single story portion when they lift the building.

We propose demolishing the single story portion, lifting the two story building to accommodate the new foundation and framing work, and rebuilding the single story portion and porch to the original proportions.

Change #2: The submitted drawings do not indicate an exterior finish for the new foundation walls, but brick veneer was discussed on-site. A sandstone veneer that matches the substantial scale as it appears in the c1900 image (see Image 1) would be appropriate. SIS #3 states that "Each property will be recognized as a physical record of its time, place, and use. Changes that create a false sense of historical development, such as adding conjectural features or elements from other historic properties, will not be undertaken."
Response: Acknowledged, we have no issue with a sandstone veneer in lieu of brick veneer.

Change #3: The lap exposure of the siding is not indicated on the drawings, it should match the lap exposure as it appears in the c1900 image (see image 1).
Response: Acknowledged, we will match the lap siding exposure per the image.

Change #4: The roof detail 4 on Sheet A1.03 does not specify a material for the "low slope above return." This material should be copper to match existing.
Response: Acknowledged, we have no issue with the material being copper.

Change #5: The proportions of the proposed shutters as drawn does not appear appropriate for the windows. Each shutter should be correctly sized to only cover half the window when in the closed position.
Response: The shutters are drawn correctly. Each shutter covers half the window when in the closed position.

Change #6: The location of the faux chimney should be based on the c.1900 image (see Image 1). Note the chimney in the c.1900 image appears to be brick and the foundation appears to be stone.
Response: Acknowledged, the chimney will match the location per the image.

Signage and lighting locations are not indicated on the drawings but will be reviewed by the AHBR per Codified Ordinances Appendix D – Architectural Standards Section III-1. The owner and AHBR will work together to determine appropriate locations for these items.
Response: Acknowledged, we will work with AHBR on signage and lighting.

Dingey Movers, Inc.
544 Harvard Street
Zanesville, Ohio 43701
609-618-9263
karadingey@dingeymovers.com

Dear Sir: October 25, 2023

In the matter of the Hudson Historical building project, it is beneficial to all involved that the 1-story addition be removed prior to starting the project.

Dingey Movers, Inc. is not inclined to do further damage to the adjacent building where this current 1-room addition exists. The addition is too close to the current neighbor.

If you have further questions/or concerns, please do not hesitate to contact me.

Thank you,

Bart Dingey

Direct/ 740-624-1748
Office/ 740-453-6724

karadingey@dingeymovers.com
info@dingeymovers.com
www.dingeymovers.com

Image 1: Date c.1900. Courtesy of the Hudson Historical Society. Front (west) and side (north) elevation. Note the Gable-Front-and-Wing configuration of the house, the 6 over 6 double hung windows on the first floor, the exposure width of the siding, the two chimneys appear to be brick, and the foundation appears to be stone. The door appears to be under the porch on the north return wall.