



# Stormwater Management Report

CCC – Hudson, OH

750 W Streetsboro St,  
Hudson, OH 44236

Date Prepared: July 7th, 2025

Revised:

On behalf of:

**Christ  
Community  
Chapel**

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7/8/2025

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## Introduction

This report covers the methodology and calculations used in the design of the stormwater management system for the proposed turf field and restroom at the existing Chris Community Chapel campus in Hudson, Ohio.

The stormwater management system is designed in accordance with Chapter 1419 of the City of Hudson's Codified Ordinances. Section 5 of Chapter 1419 has requirements for stormwater quality, stormwater quantity, and major flood path.

- The major flood path is met by directing all runoff towards the proposed detention basin.
- The stormwater quality requirements can be met by adding the water quality volume to the volume of the proposed detention basin and meeting the minimum drawdown time of 48 hours.
- The stormwater quantity requirements for the field's construction and restroom area can be met by the proposed detention basin. According to the City of Hudson Stormwater Code, the 25-year peak rate of runoff in the post-developed condition shall not exceed the 2-year peak rate of runoff in the existing condition. The 100-year post developed condition must also be reduced to the 10-yr peak rate of runoff in the existing condition.

Storm routings for this project were performed using HydroCAD. Time of Concentration was determined by using the TR-55 method, within HydroCAD.

The onsite soils were obtained from USDA NRCS Web Soil Survey and can be found in **Appendix D**.

The storm pipe network was designed using Hydraflow Stormsewers Extension for Autodesk Civil 3D. Section 4 of Chapter 4 of the City of Hudson's Codified Ordinances requires that the pipes be sized so that the HGL does not exceed the crown of the pipe for the 10-year storm. Refer to **Appendix C** for the Storm Pipe Calculations and **Appendix E3** for the associated Tributary Drainage area Map.

## Existing Conditions

The site is a church on 30.07 acres that has frontage along West Streetsboro Street to the north and Terex Road to the west. The site consists of a chapel, parking lot, open field, and stormwater management area. Residential properties border the site to the east and to the south. Existing runoff flows to the stormwater management area via a combination of overland flow and an existing storm sewer system.

The existing runoff consists of one (1) major existing drainage areas as listed below:

- EDA-WEST - This drainage area drains to the north, towards an existing stormwater management area. The Soil Survey indicates this site to have Caneadea Silt Loam, Ellsworth-Urban Land Complex, Geeburg Silt Loam, Sebring Silt Loam, each soil being Hydric Group 'D' type soil. For hydrologic soil group 'D' soils we assumed CN values of 98 for impervious areas and 84 for grass areas in good conditions.

Peak runoff rates from the existing conditions of the site are listed in the following table:

Existing Conditions Peak Runoff Rates							
Drainage Area	1-year Storm	2-year Storm	5-year Storm	10-year Storm	25-year Storm	50-year Storm	100-year Storm
EDA-WEST	5.42 CFS	7.38 CFS	10.22 CFS	12.67 CFS	16.25 CFS	19.29 CFS	22.52 CFS

Refer to **Appendix A** for the Existing Conditions Calculations. The Existing Conditions Drainage Area Map can be found in **Appendix E1**.

## Proposed Conditions

The proposed development of the site will consist of the construction of a 389 SF restroom building, a 6,834 SF office addition, a 44,500 SF turf soccer field, associated site improvements and a stormwater management system. Due to the location of the proposed office addition, an existing parking area to the south of the proposed field addition is being routed to a proposed detention basin to compensate for the limits of disturbance associated with the office addition. See Proposed Drainage Plan in Appendix E2. The office addition will be routed to the existing detention basin. The stormwater management system consists of an extended detention basin, a gravel area with a 6" underdrain, an outlet control structure, and an emergency spillway. The extended detention basin in conjunction with the outlet control structure has been designed to address the water quality and water quantity requirements. The outflow from the proposed detention basin will be routed through the outlet structure and directed into the existing storm water management system on site.

The proposed improvements will create one (1) major drainage area and one (1) detention node

- DA-WEST - This drainage area drains to stormwater management basin. This includes the existing parking lot area that is being routed to the basin, in place of the proposed office addition area.
- POND – Proposed stormwater management basin, which discharges into the existing storm water management system on site.

For hydrologic soil group 'D' soils we assumed CN values of 98 for impervious areas and 84 for grass areas in good conditions.

## Stormwater Quality

To satisfy the water quality requirements, Ohio's water quality BMP Compliance Worksheet and Water Quality Calculator were used. The water quality volume was calculated and was incorporated into the detention pond design. Calculations included the proposed field LOD, as well as the parking lot to the south of this area. The parking lot to the south of the proposed field LOD was added to this calculation to compensate for the office addition LOD, which is not included because it is being routed to the existing detention basin. A water quality orifice was included to satisfy the requirements, as well as a window on the outlet control structure at the water quality elevation. The orifice has been designed to meet the required minimum drawdown time of 48 hours.

Therefore, the stormwater quality requirements have been satisfied. Refer to **Appendix B1** for Stormwater Quality Calculations.

## Stormwater Quantity

The resulting proposed conditions peak runoff rates are listed in the following table:

Proposed Conditions Peak Runoff Rates							
Drainage Area	1-year Storm	2-year Storm	5-year Storm	10-year Storm	25-year Storm	50-year Storm	100-year Storm
PDA-WEST	7.62 CFS	9.89 CFS	13.22 CFS	16.06 CFS	20.16 CFS	23.62 CFS	27.29 CFS

The discharge characteristics for the proposed stormwater management basin are listed in the following table:

Basin		
Storm	Peak Discharge	Water surface Elevation
1-year	2.02 CFS	1010.64 FT
2-year	3.21 CFS	1010.86 FT
5-year	4.52 CFS	1011.24 FT
10-year	5.38 CFS	1011.57 FT
25-year	7.30 CFS	1011.98 FT
50-year	9.86 CFS	1012.21 FT
100-year	11.65 CFS	1012.45 FT

The runoff of the 25-year storm event has been designed to be lower than the existing runoff of the 2-year storm event, as has the 100-year event to the existing 10-year storm event based upon the requirements set forth by the City of Hudson's codified ordinances. Refer to **Appendix B** for storm calculations.

A summary of the existing conditions peak runoff rates, the allowable peak runoff rates and the proposed conditions peak runoff rates are listed in the following table:

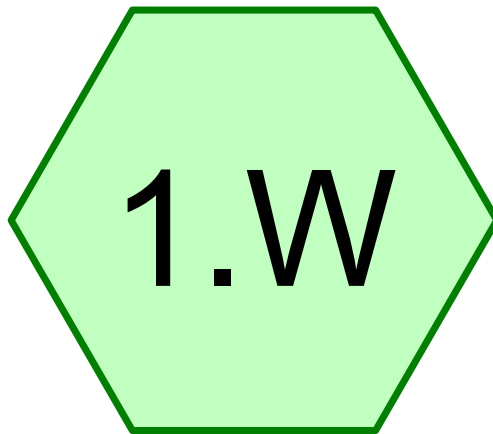
Runoff Reduction Summary			
Storm	Existing	Allowable	Proposed
1-year	5.42 CFS	7.38 CFS	2.02 CFS
2-year	7.38 CFS	7.38 CFS	3.21 CFS
5-year	10.22 CFS	7.38 CFS	4.52 CFS
10-year	12.67 CFS	7.38 CFS	5.38 CFS
25-year	16.25 CFS	7.38 CFS	7.30 CFS
50-year	19.29 CFS	12.67 CFS	9.86 CFS
100-year	22.52 CFS	12.67 CFS	11.65 CFS

Refer to **Appendix B** for the Stormwater Quantity Calculations. The Proposed Conditions Drainage Area Map can be found in **Appendix E2**.

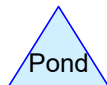
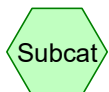
### **Summary**

The proposed stormwater management system has been successfully designed to manage the increased runoff from associated improvements of the project. The stormwater management system has been designed in accordance with the appropriate regulations, as demonstrated in the previous tables and accompanying calculations.

**APPENDIX A:  
EXISTING CONDITIONS CALCULATIONS**



# EDA-WEST





**Summary for Subcatchment 1.W: EDA-WEST**

Runoff = 5.42 cfs @ 12.13 hrs, Volume= 14,389 cf, Depth> 0.92"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-20.00 hrs, dt= 0.05 hrs  
OH-Summit County\_Hudson 24-hr S1 2-yr 1-yr Rainfall=2.04"

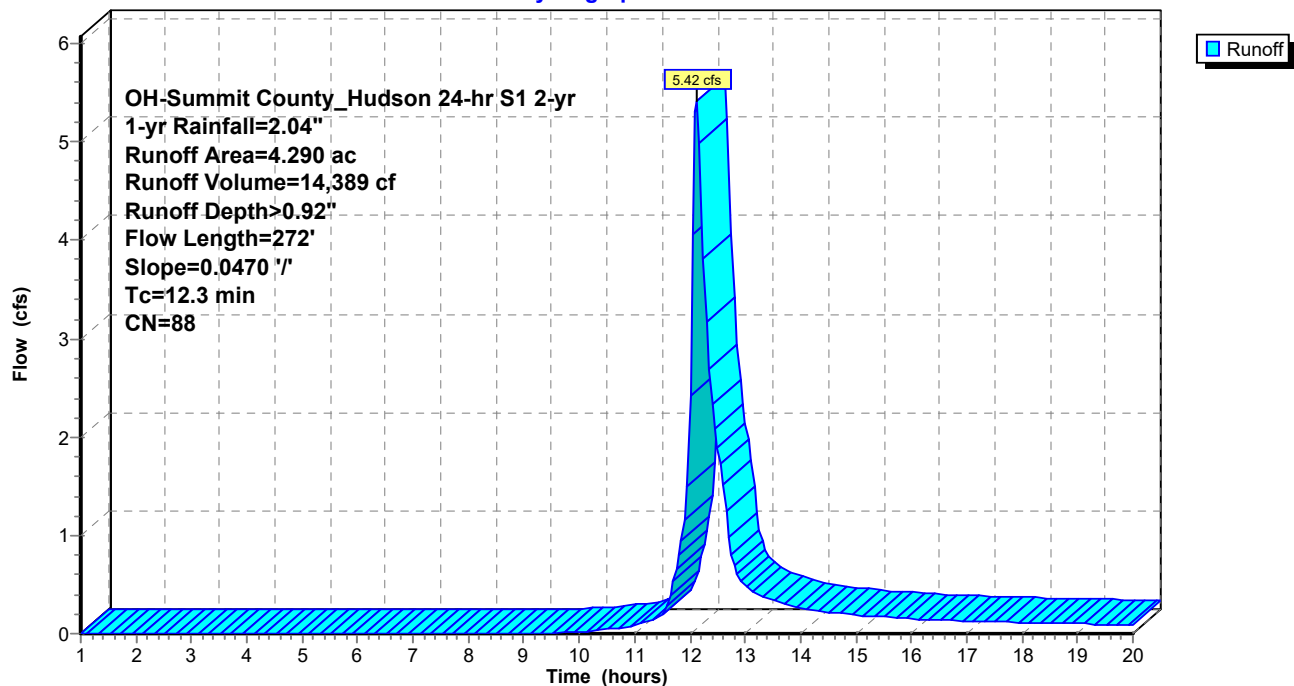
Area (ac)	CN	Description
3.075	84	50-75% Grass cover, Fair, HSG D
1.215	98	Paved parking, HSG D
4.290	88	Weighted Average
3.075		71.68% Pervious Area
1.215		28.32% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
11.0	150	0.0470	0.23		<b>Sheet Flow,</b> Grass: Short n= 0.150 P2= 2.44"
1.3	122	0.0470	1.52		<b>Shallow Concentrated Flow,</b> Short Grass Pasture Kv= 7.0 fps
12.3	272	Total			

**Subcatchment 1.W: EDA-WEST**

Hydrograph



**Summary for Subcatchment 1.W: EDA-WEST**

Runoff = 7.38 cfs @ 12.12 hrs, Volume= 19,275 cf, Depth> 1.24"

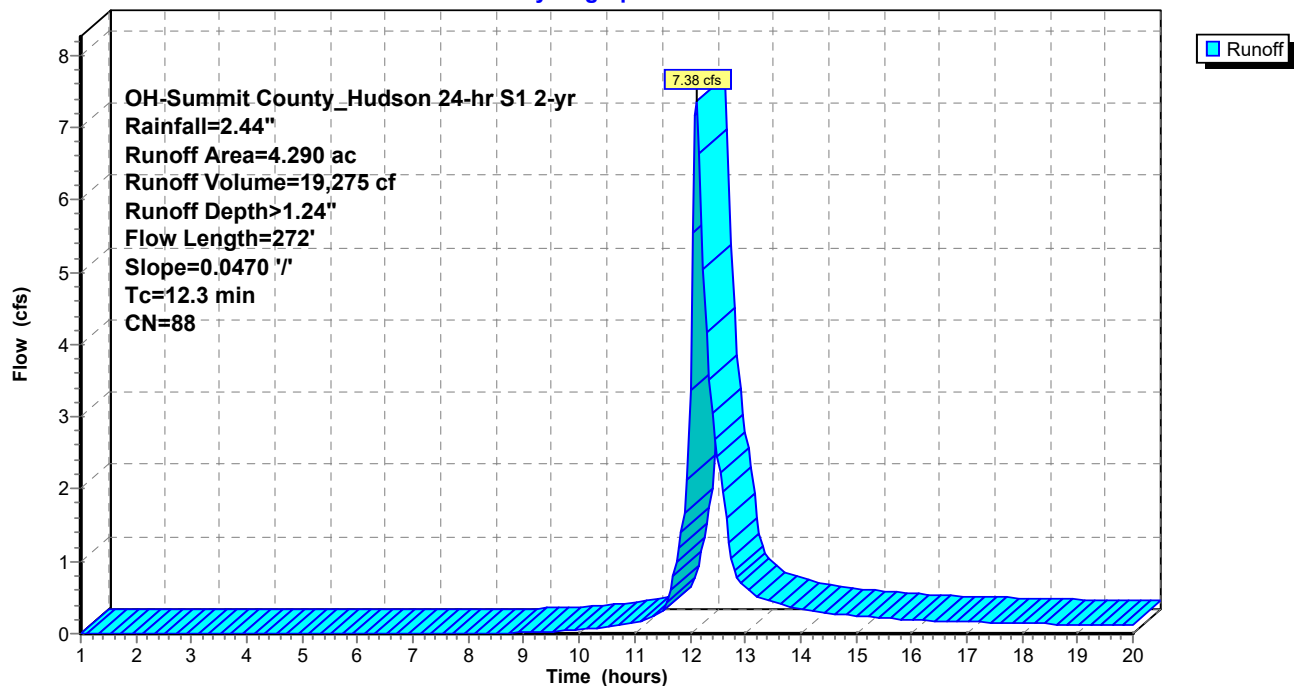
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-20.00 hrs, dt= 0.05 hrs  
OH-Summit County\_Hudson 24-hr S1 2-yr Rainfall=2.44"

Area (ac)	CN	Description
3.075	84	50-75% Grass cover, Fair, HSG D
1.215	98	Paved parking, HSG D
4.290	88	Weighted Average
3.075		71.68% Pervious Area
1.215		28.32% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
11.0	150	0.0470	0.23		<b>Sheet Flow,</b> Grass: Short n= 0.150 P2= 2.44"
1.3	122	0.0470	1.52		<b>Shallow Concentrated Flow,</b> Short Grass Pasture Kv= 7.0 fps
12.3	272	Total			

**Subcatchment 1.W: EDA-WEST**

Hydrograph



**Summary for Subcatchment 1.W: EDA-WEST**

Runoff = 10.22 cfs @ 12.12 hrs, Volume= 26,723 cf, Depth> 1.72"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-20.00 hrs, dt= 0.05 hrs  
OH-Summit County\_Hudson 24-hr S1 2-yr 5-yr Rainfall=3.02"

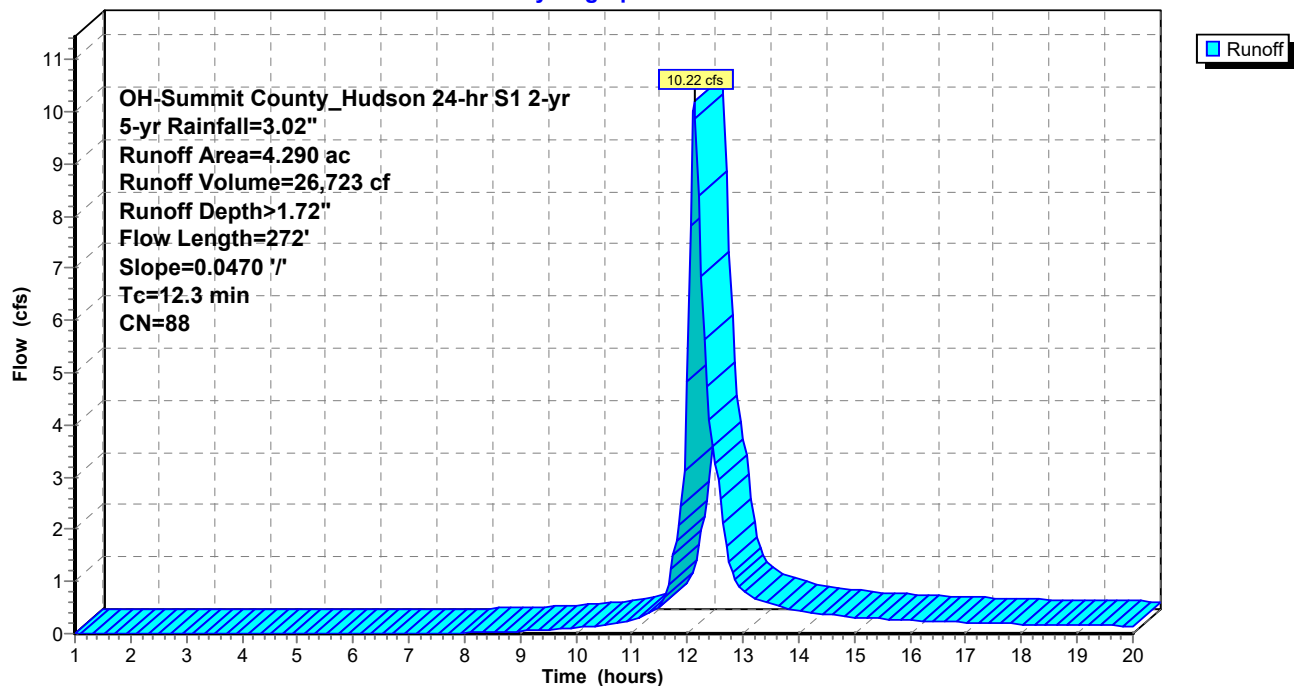
Area (ac)	CN	Description
3.075	84	50-75% Grass cover, Fair, HSG D
1.215	98	Paved parking, HSG D
4.290	88	Weighted Average
3.075		71.68% Pervious Area
1.215		28.32% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
11.0	150	0.0470	0.23		<b>Sheet Flow,</b> Grass: Short n= 0.150 P2= 2.44"
1.3	122	0.0470	1.52		<b>Shallow Concentrated Flow,</b> Short Grass Pasture Kv= 7.0 fps
12.3	272	Total			

**Subcatchment 1.W: EDA-WEST**

Hydrograph



### Summary for Subcatchment 1.W: EDA-WEST

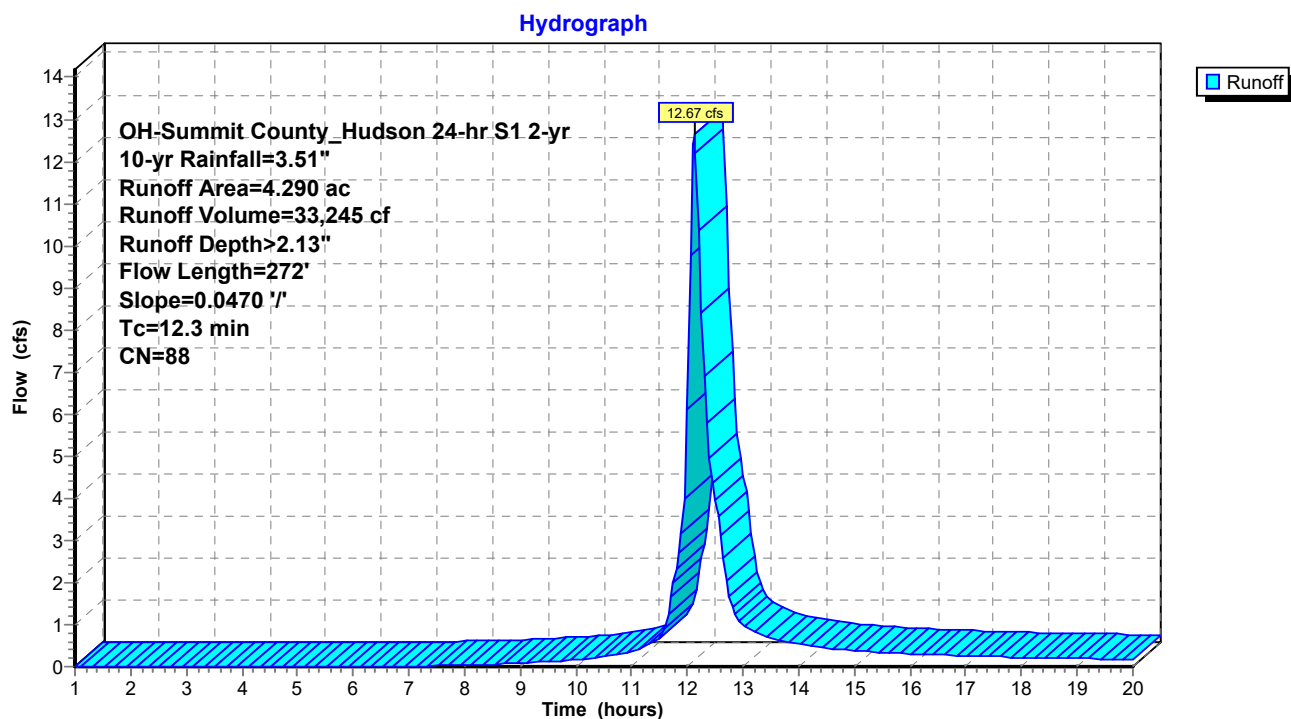
Runoff = 12.67 cfs @ 12.12 hrs, Volume= 33,245 cf, Depth> 2.13"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-20.00 hrs, dt= 0.05 hrs  
 OH-Summit County\_Hudson 24-hr S1 2-yr 10-yr Rainfall=3.51"

Area (ac)	CN	Description
3.075	84	50-75% Grass cover, Fair, HSG D
1.215	98	Paved parking, HSG D
4.290	88	Weighted Average
3.075		71.68% Pervious Area
1.215		28.32% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
11.0	150	0.0470	0.23		<b>Sheet Flow,</b> Grass: Short n= 0.150 P2= 2.44"
1.3	122	0.0470	1.52		<b>Shallow Concentrated Flow,</b> Short Grass Pasture Kv= 7.0 fps
12.3	272	Total			

### Subcatchment 1.W: EDA-WEST



### Summary for Subcatchment 1.W: EDA-WEST

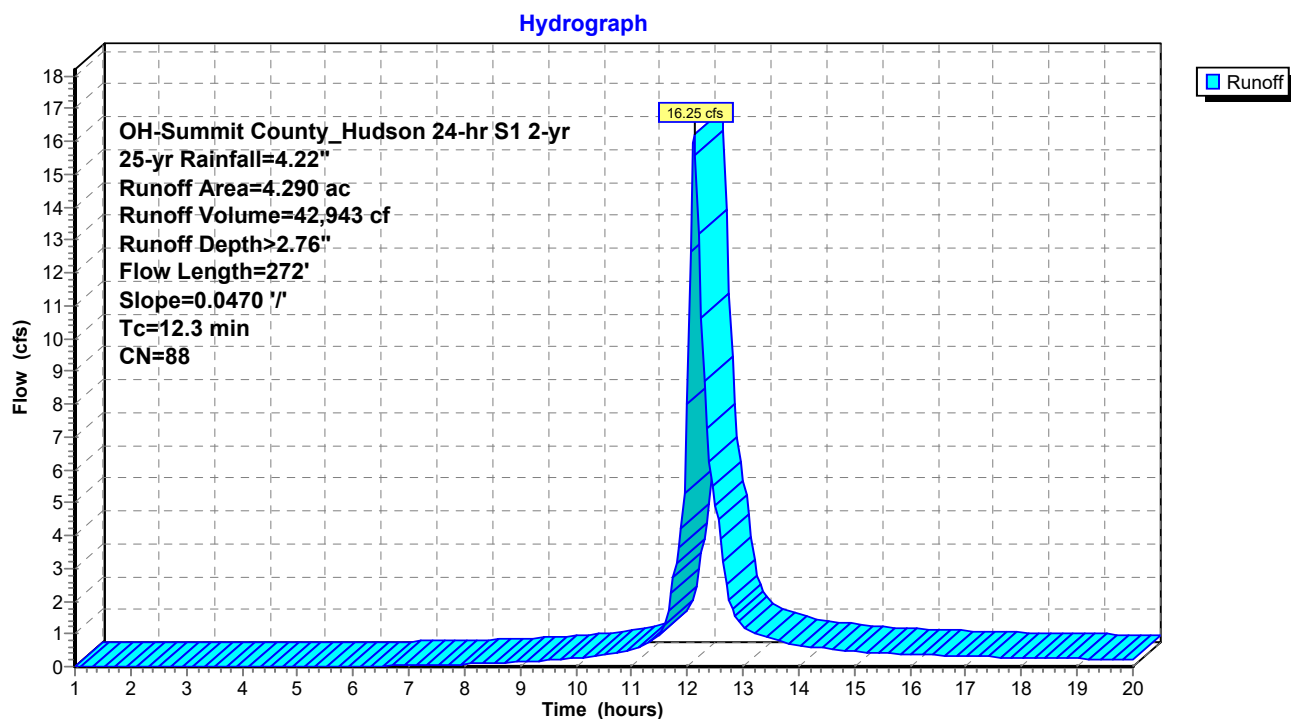
Runoff = 16.25 cfs @ 12.12 hrs, Volume= 42,943 cf, Depth> 2.76"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-20.00 hrs, dt= 0.05 hrs  
 OH-Summit County\_Hudson 24-hr S1 2-yr 25-yr Rainfall=4.22"

Area (ac)	CN	Description
3.075	84	50-75% Grass cover, Fair, HSG D
1.215	98	Paved parking, HSG D
4.290	88	Weighted Average
3.075		71.68% Pervious Area
1.215		28.32% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
11.0	150	0.0470	0.23		<b>Sheet Flow,</b> Grass: Short n= 0.150 P2= 2.44"
1.3	122	0.0470	1.52		<b>Shallow Concentrated Flow,</b> Short Grass Pasture Kv= 7.0 fps
12.3	272	Total			

### Subcatchment 1.W: EDA-WEST



### Summary for Subcatchment 1.W: EDA-WEST

Runoff = 19.29 cfs @ 12.12 hrs, Volume= 51,299 cf, Depth> 3.29"

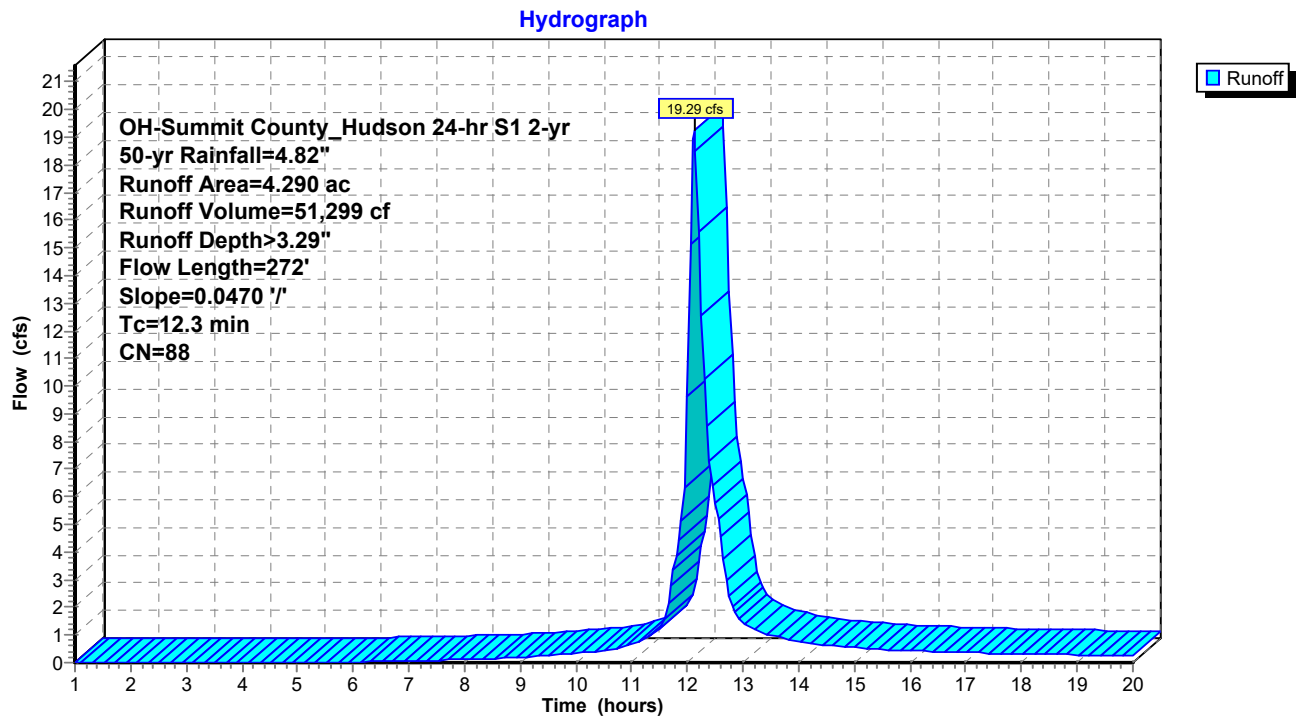
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-20.00 hrs, dt= 0.05 hrs  
 OH-Summit County\_Hudson 24-hr S1 2-yr 50-yr Rainfall=4.82"

Area (ac)	CN	Description
3.075	84	50-75% Grass cover, Fair, HSG D
1.215	98	Paved parking, HSG D
4.290	88	Weighted Average
3.075		71.68% Pervious Area
1.215		28.32% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
11.0	150	0.0470	0.23		<b>Sheet Flow,</b> Grass: Short n= 0.150 P2= 2.44"
1.3	122	0.0470	1.52		<b>Shallow Concentrated Flow,</b> Short Grass Pasture Kv= 7.0 fps
12.3	272	Total			

### Subcatchment 1.W: EDA-WEST



### Summary for Subcatchment 1.W: EDA-WEST

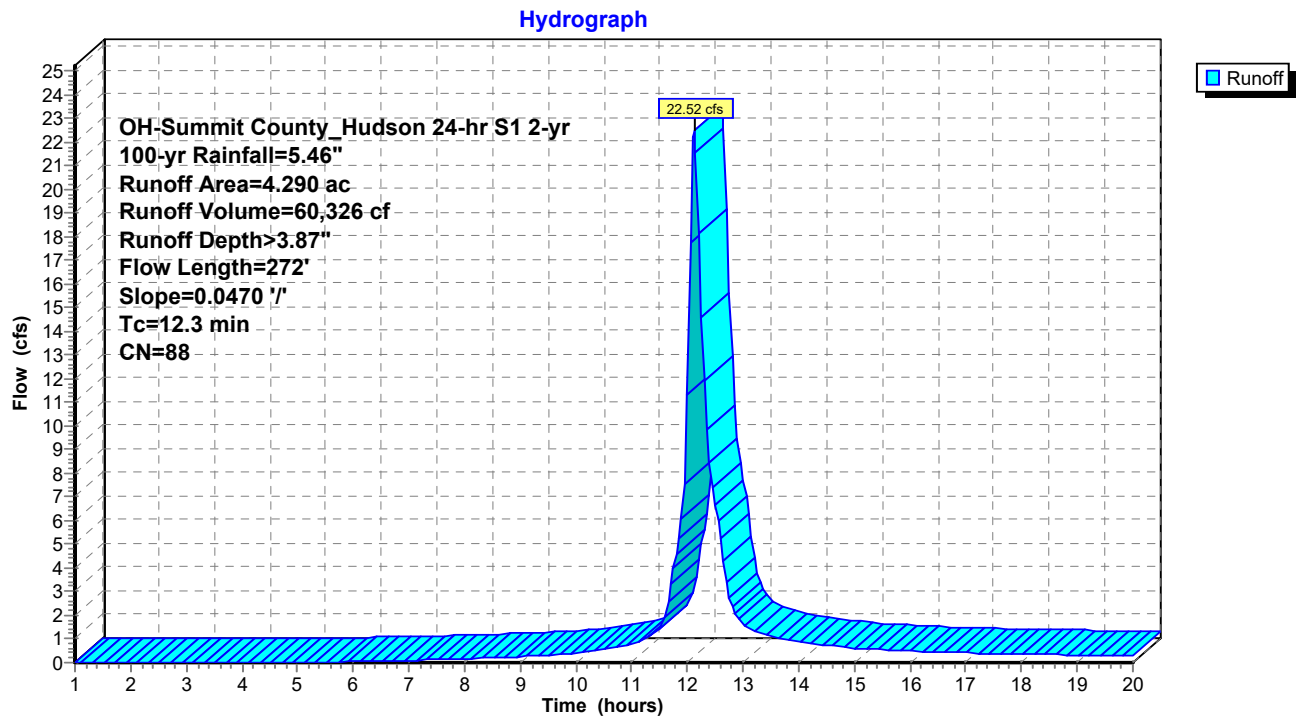
Runoff = 22.52 cfs @ 12.12 hrs, Volume= 60,326 cf, Depth> 3.87"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-20.00 hrs, dt= 0.05 hrs  
 OH-Summit County\_Hudson 24-hr S1 2-yr 100-yr Rainfall=5.46"

Area (ac)	CN	Description
3.075	84	50-75% Grass cover, Fair, HSG D
1.215	98	Paved parking, HSG D
4.290	88	Weighted Average
3.075		71.68% Pervious Area
1.215		28.32% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
11.0	150	0.0470	0.23		<b>Sheet Flow,</b> Grass: Short n= 0.150 P2= 2.44"
1.3	122	0.0470	1.52		<b>Shallow Concentrated Flow,</b> Short Grass Pasture Kv= 7.0 fps
12.3	272	Total			

### Subcatchment 1.W: EDA-WEST

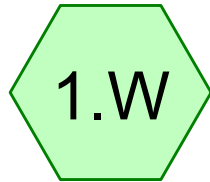


**Events for Subcatchment 1.W: EDA-WEST**

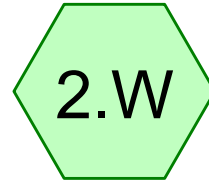
Event	Rainfall (inches)	Runoff (cfs)	Volume (cubic-feet)	Depth (inches)
1-yr	2.04	5.42	14,389	0.92
2-yr	2.44	7.38	19,275	1.24
5-yr	3.02	10.22	26,723	1.72
10-yr	3.51	12.67	33,245	2.13
25-yr	4.22	16.25	42,943	2.76
50-yr	4.82	19.29	51,299	3.29
100-yr	<b>5.46</b>	<b>22.52</b>	<b>60,326</b>	<b>3.87</b>



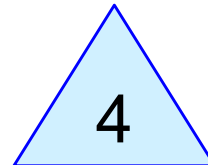
**APPENDIX B:  
PROPOSED CONDITIONS CALCULATIONS**



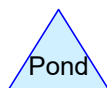
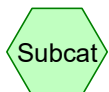
EDA-WEST



PDA-WEST



POND



Routing Diagram for 765295 - HYDROCAD\_REV1

Prepared by CESO, Inc, Printed 7/7/2025

HydroCAD® 10.20-5c s/n 11958 © 2023 HydroCAD Software Solutions LLC

**Summary for Subcatchment 1.W: EDA-WEST**

Runoff = 5.42 cfs @ 12.13 hrs, Volume= 14,389 cf, Depth> 0.92"

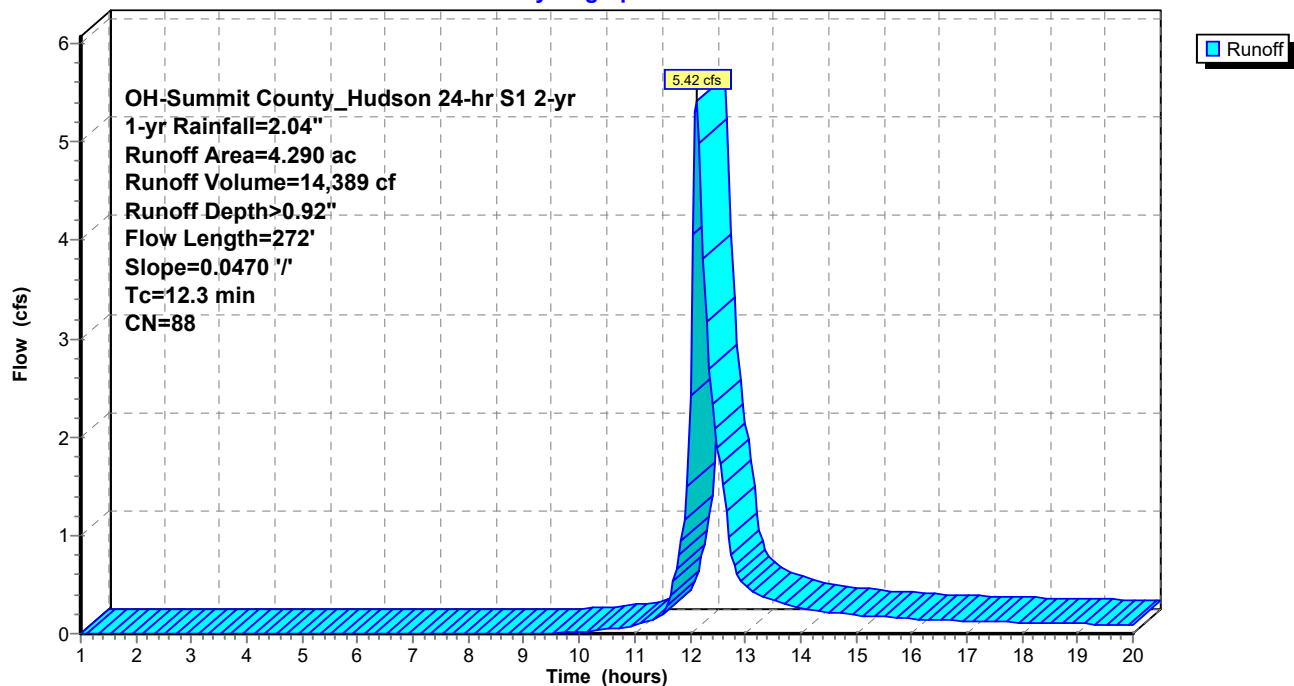
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-20.00 hrs, dt= 0.05 hrs  
OH-Summit County\_Hudson 24-hr S1 2-yr 1-yr Rainfall=2.04"

Area (ac)	CN	Description
3.075	84	50-75% Grass cover, Fair, HSG D
1.215	98	Paved parking, HSG D
4.290	88	Weighted Average
3.075		71.68% Pervious Area
1.215		28.32% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
11.0	150	0.0470	0.23		<b>Sheet Flow,</b> Grass: Short n= 0.150 P2= 2.44"
1.3	122	0.0470	1.52		<b>Shallow Concentrated Flow,</b> Short Grass Pasture Kv= 7.0 fps
12.3	272	Total			

**Subcatchment 1.W: EDA-WEST**

Hydrograph



### Summary for Subcatchment 2.W: PDA-WEST

Runoff = 7.62 cfs @ 12.12 hrs, Volume= 19,900 cf, Depth> 1.12"  
 Routed to Pond 4 : POND

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-20.00 hrs, dt= 0.05 hrs  
 OH-Summit County\_Hudson 24-hr S1 2-yr 1-yr Rainfall=2.04"

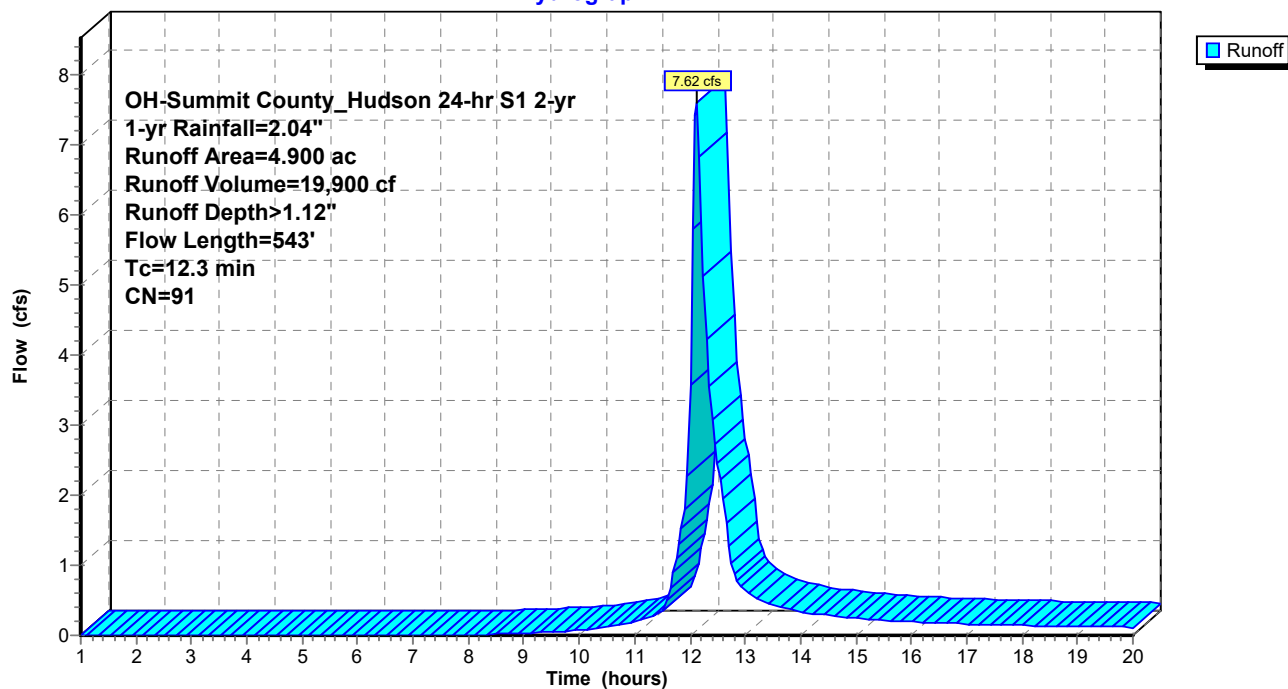
Area (ac)	CN	Description
* 1.020	95	Permeable Turf Field, HSG D
2.080	84	50-75% Grass cover, Fair, HSG D
1.800	98	Paved parking, HSG D
4.900	91	Weighted Average
3.100		63.27% Pervious Area
1.800		36.73% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
9.9	43	0.0050	0.07		<b>Sheet Flow,</b> Grass: Short n= 0.150 P2= 2.44"
1.9	300	0.0050	2.63	0.52	<b>Pipe Channel,</b> 6.00" Round Area= 0.2 sf Perim= 1.6' r= 0.13' n= 0.010
0.5	200	0.0100	6.84	8.40	<b>Pipe Channel,</b> 15.00" Round Area= 1.2 sf Perim= 3.9' r= 0.31' n= 0.010
12.3	543	Total			

## Subcatchment 2.W: PDA-WEST

Hydrograph



**Summary for Pond 4: POND**

Inflow Area = 213,444 sf, 36.73% Impervious, Inflow Depth > 1.12" for 1-yr event  
 Inflow = 7.62 cfs @ 12.12 hrs, Volume= 19,900 cf  
 Outflow = 2.02 cfs @ 12.59 hrs, Volume= 12,175 cf, Atten= 73%, Lag= 28.0 min  
 Primary = 2.02 cfs @ 12.59 hrs, Volume= 12,175 cf  
     Routed to nonexistent node 5L  
 Secondary = 0.00 cfs @ 1.00 hrs, Volume= 0 cf  
     Routed to nonexistent node 5L

Routing by Stor-Ind method, Time Span= 1.00-20.00 hrs, dt= 0.05 hrs  
 Peak Elev= 1,010.64' @ 12.59 hrs Surf.Area= 8,152 sf Storage= 10,510 cf

Plug-Flow detention time= 132.6 min calculated for 12,175 cf (61% of inflow)  
 Center-of-Mass det. time= 68.3 min ( 852.2 - 783.9 )

Volume	Invert	Avail.Storage	Storage Description
#1	1,008.00'	44,483 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Voids (%)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
1,008.00	2,848	0.0	0	0
1,009.00	2,848	40.0	1,139	1,139
1,010.00	6,536	100.0	4,692	5,831
1,011.00	9,073	100.0	7,805	13,636
1,012.00	11,663	100.0	10,368	24,004
1,013.00	14,311	100.0	12,987	36,991
1,013.50	15,658	100.0	7,492	44,483

Device	Routing	Invert	Outlet Devices
#1	Device 4	1,008.00'	<b>1.44" Vert. Water Quality Orifice</b> C= 0.600 Limited to weir flow at low heads
#2	Secondary	1,013.00'	<b>10.0' long + 3.0 ' SideZ x 4.0' breadth Spillway</b> Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.38 2.54 2.69 2.68 2.67 2.67 2.65 2.66 2.66 2.68 2.72 2.73 2.76 2.79 2.88 3.07 3.32
#3	Device 4	1,012.80'	<b>27.50" x 27.50" Horiz. Rim</b> C= 0.600 Limited to weir flow at low heads
#4	Primary	1,008.00'	<b>18.00" Vert. Outlet</b> C= 0.600 Limited to weir flow at low heads
#5	Device 4	1,010.25'	<b>30.00" W x 5.00" H Vert. WQV Window</b> C= 0.600 Limited to weir flow at low heads
#6	Device 4	1,011.80'	<b>48.00" W x 4.00" H Vert. Orifice/Grate</b> C= 0.600 Limited to weir flow at low heads

**Primary OutFlow** Max=2.02 cfs @ 12.59 hrs HW=1,010.64' (Free Discharge)

↳ **4=Outlet** (Passes 2.02 cfs of 11.69 cfs potential flow)

↳ **1=Water Quality Orifice** (Orifice Controls 0.09 cfs @ 7.73 fps)

↳ **3=Rim** ( Controls 0.00 cfs)

↳ **5=WQV Window** (Orifice Controls 1.93 cfs @ 2.00 fps)

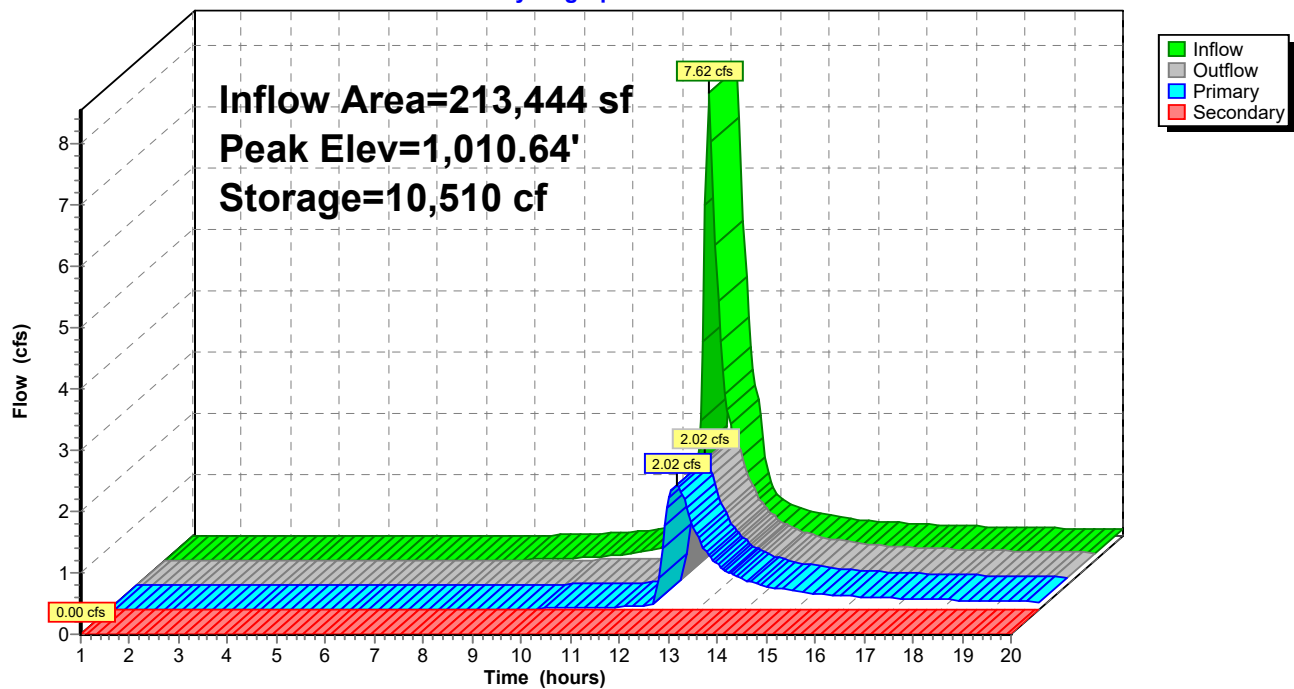
↳ **6=Orifice/Grate** ( Controls 0.00 cfs)

**Secondary OutFlow** Max=0.00 cfs @ 1.00 hrs HW=1,008.00' (Free Discharge)

↳ **2=Spillway** ( Controls 0.00 cfs)

### Pond 4: POND

#### Hydrograph



**Summary for Subcatchment 1.W: EDA-WEST**

Runoff = 7.38 cfs @ 12.12 hrs, Volume= 19,275 cf, Depth> 1.24"

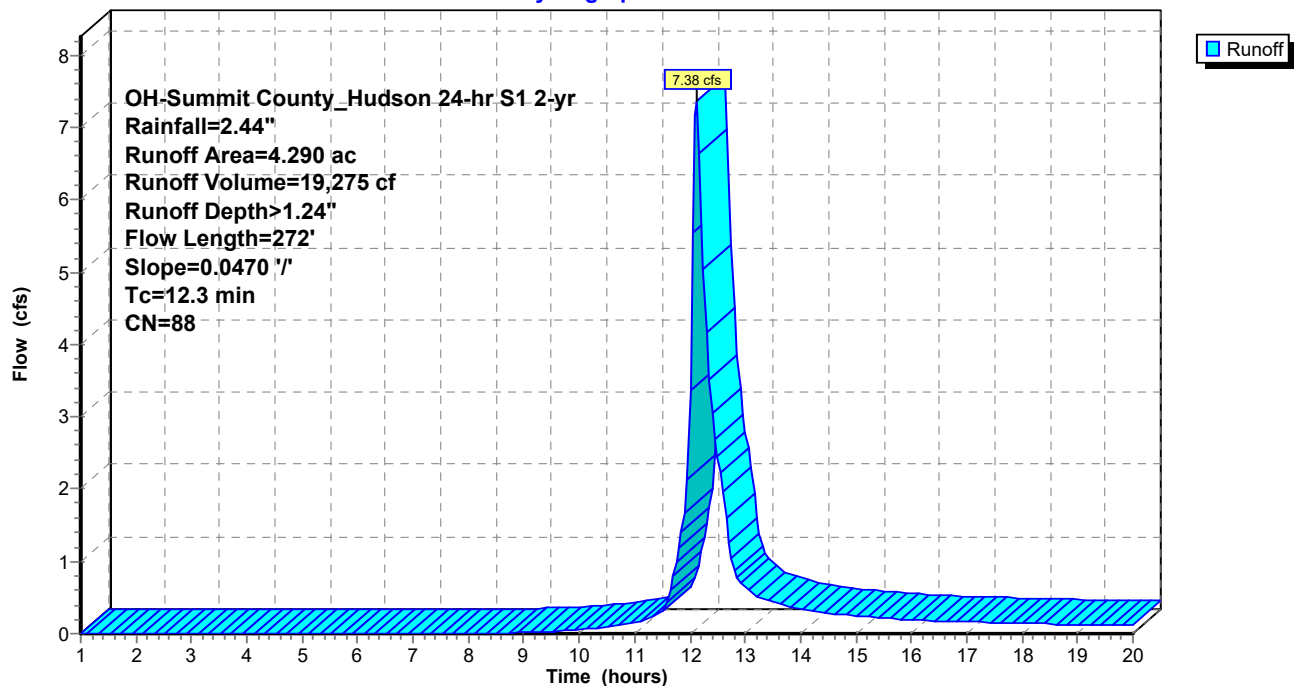
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-20.00 hrs, dt= 0.05 hrs  
OH-Summit County\_Hudson 24-hr S1 2-yr Rainfall=2.44"

Area (ac)	CN	Description
3.075	84	50-75% Grass cover, Fair, HSG D
1.215	98	Paved parking, HSG D
4.290	88	Weighted Average
3.075		71.68% Pervious Area
1.215		28.32% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
11.0	150	0.0470	0.23		<b>Sheet Flow,</b> Grass: Short n= 0.150 P2= 2.44"
1.3	122	0.0470	1.52		<b>Shallow Concentrated Flow,</b> Short Grass Pasture Kv= 7.0 fps
12.3	272	Total			

**Subcatchment 1.W: EDA-WEST**

Hydrograph





**Summary for Subcatchment 2.W: PDA-WEST**

Runoff = 9.89 cfs @ 12.12 hrs, Volume= 25,919 cf, Depth> 1.46"  
 Routed to Pond 4 : POND

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-20.00 hrs, dt= 0.05 hrs  
 OH-Summit County\_Hudson 24-hr S1 2-yr Rainfall=2.44"

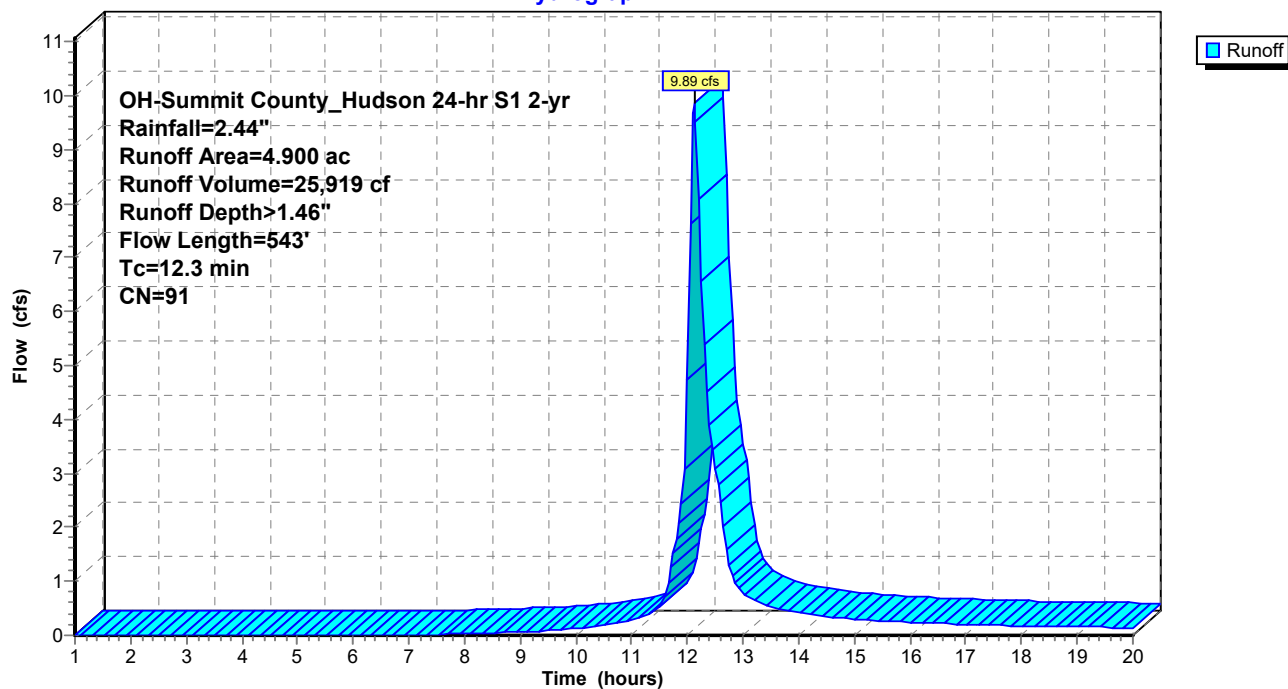
Area (ac)	CN	Description
* 1.020	95	Permeable Turf Field, HSG D
2.080	84	50-75% Grass cover, Fair, HSG D
1.800	98	Paved parking, HSG D
4.900	91	Weighted Average
3.100		63.27% Pervious Area
1.800		36.73% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
9.9	43	0.0050	0.07		<b>Sheet Flow,</b> Grass: Short n= 0.150 P2= 2.44"
1.9	300	0.0050	2.63	0.52	<b>Pipe Channel,</b> 6.00" Round Area= 0.2 sf Perim= 1.6' r= 0.13' n= 0.010
0.5	200	0.0100	6.84	8.40	<b>Pipe Channel,</b> 15.00" Round Area= 1.2 sf Perim= 3.9' r= 0.31' n= 0.010
12.3	543	Total			

## Subcatchment 2.W: PDA-WEST

Hydrograph



**Summary for Pond 4: POND**

Inflow Area = 213,444 sf, 36.73% Impervious, Inflow Depth > 1.46" for 2-yr event  
 Inflow = 9.89 cfs @ 12.12 hrs, Volume= 25,919 cf  
 Outflow = 3.21 cfs @ 12.48 hrs, Volume= 18,094 cf, Atten= 68%, Lag= 21.7 min  
 Primary = 3.21 cfs @ 12.48 hrs, Volume= 18,094 cf  
 Routed to nonexistent node 5L  
 Secondary = 0.00 cfs @ 1.00 hrs, Volume= 0 cf  
 Routed to nonexistent node 5L

Routing by Stor-Ind method, Time Span= 1.00-20.00 hrs, dt= 0.05 hrs  
 Peak Elev= 1,010.86' @ 12.48 hrs Surf.Area= 8,706 sf Storage= 12,348 cf

Plug-Flow detention time= 112.5 min calculated for 18,047 cf (70% of inflow)  
 Center-of-Mass det. time= 54.8 min ( 833.4 - 778.7 )

Volume	Invert	Avail.Storage	Storage Description
#1	1,008.00'	44,483 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Voids (%)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
1,008.00	2,848	0.0	0	0
1,009.00	2,848	40.0	1,139	1,139
1,010.00	6,536	100.0	4,692	5,831
1,011.00	9,073	100.0	7,805	13,636
1,012.00	11,663	100.0	10,368	24,004
1,013.00	14,311	100.0	12,987	36,991
1,013.50	15,658	100.0	7,492	44,483

Device	Routing	Invert	Outlet Devices
#1	Device 4	1,008.00'	<b>1.44" Vert. Water Quality Orifice</b> C= 0.600 Limited to weir flow at low heads
#2	Secondary	1,013.00'	<b>10.0' long + 3.0 ' SideZ x 4.0' breadth Spillway</b> Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.38 2.54 2.69 2.68 2.67 2.67 2.65 2.66 2.66 2.68 2.72 2.73 2.76 2.79 2.88 3.07 3.32
#3	Device 4	1,012.80'	<b>27.50" x 27.50" Horiz. Rim</b> C= 0.600 Limited to weir flow at low heads
#4	Primary	1,008.00'	<b>18.00" Vert. Outlet</b> C= 0.600 Limited to weir flow at low heads
#5	Device 4	1,010.25'	<b>30.00" W x 5.00" H Vert. WQV Window</b> C= 0.600 Limited to weir flow at low heads
#6	Device 4	1,011.80'	<b>48.00" W x 4.00" H Vert. Orifice/Grate</b> C= 0.600 Limited to weir flow at low heads

**Primary OutFlow** Max=3.21 cfs @ 12.48 hrs HW=1,010.85' (Free Discharge)

↳ **4=Outlet** (Passes 3.21 cfs of 12.34 cfs potential flow)

↳ **1=Water Quality Orifice** (Orifice Controls 0.09 cfs @ 8.05 fps)

↳ **3=Rim** ( Controls 0.00 cfs)

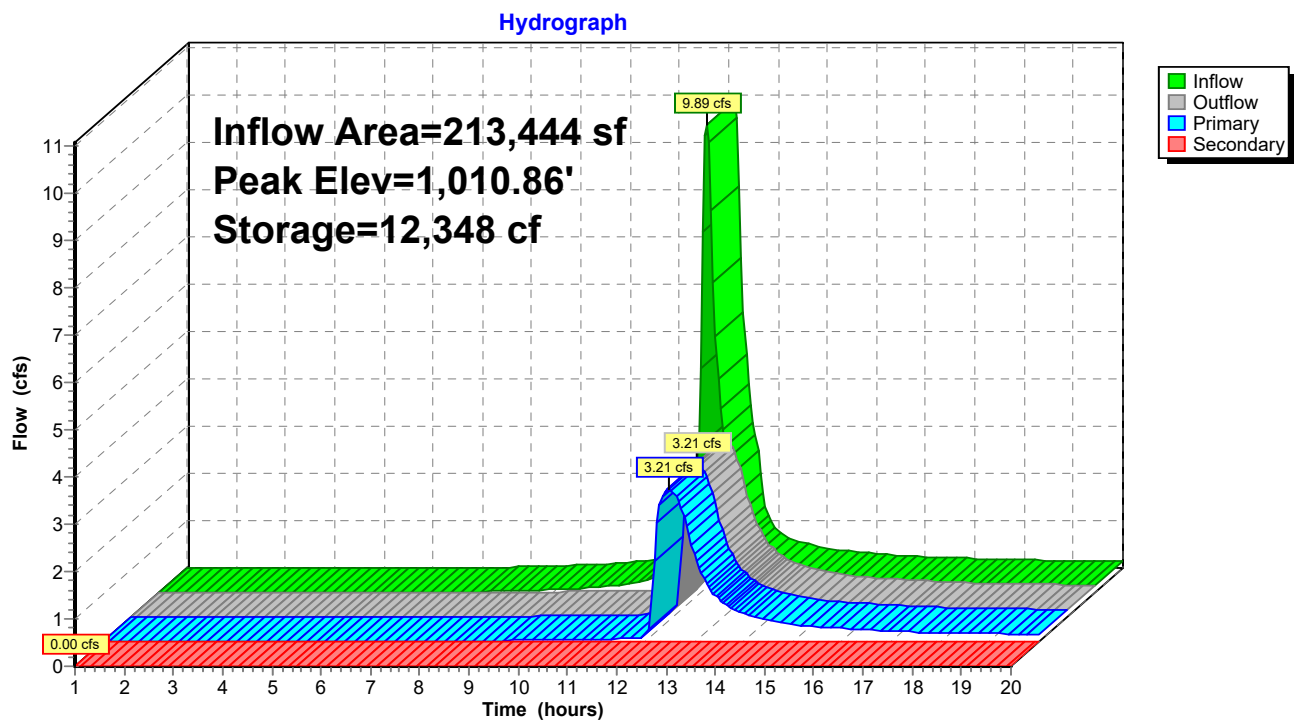
↳ **5=WQV Window** (Orifice Controls 3.12 cfs @ 2.99 fps)

↳ **6=Orifice/Grate** ( Controls 0.00 cfs)

**Secondary OutFlow** Max=0.00 cfs @ 1.00 hrs HW=1,008.00' (Free Discharge)

↳ **2=Spillway** ( Controls 0.00 cfs)

### Pond 4: POND



**Summary for Subcatchment 1.W: EDA-WEST**

Runoff = 10.22 cfs @ 12.12 hrs, Volume= 26,723 cf, Depth> 1.72"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-20.00 hrs, dt= 0.05 hrs  
OH-Summit County\_Hudson 24-hr S1 2-yr 5-yr Rainfall=3.02"

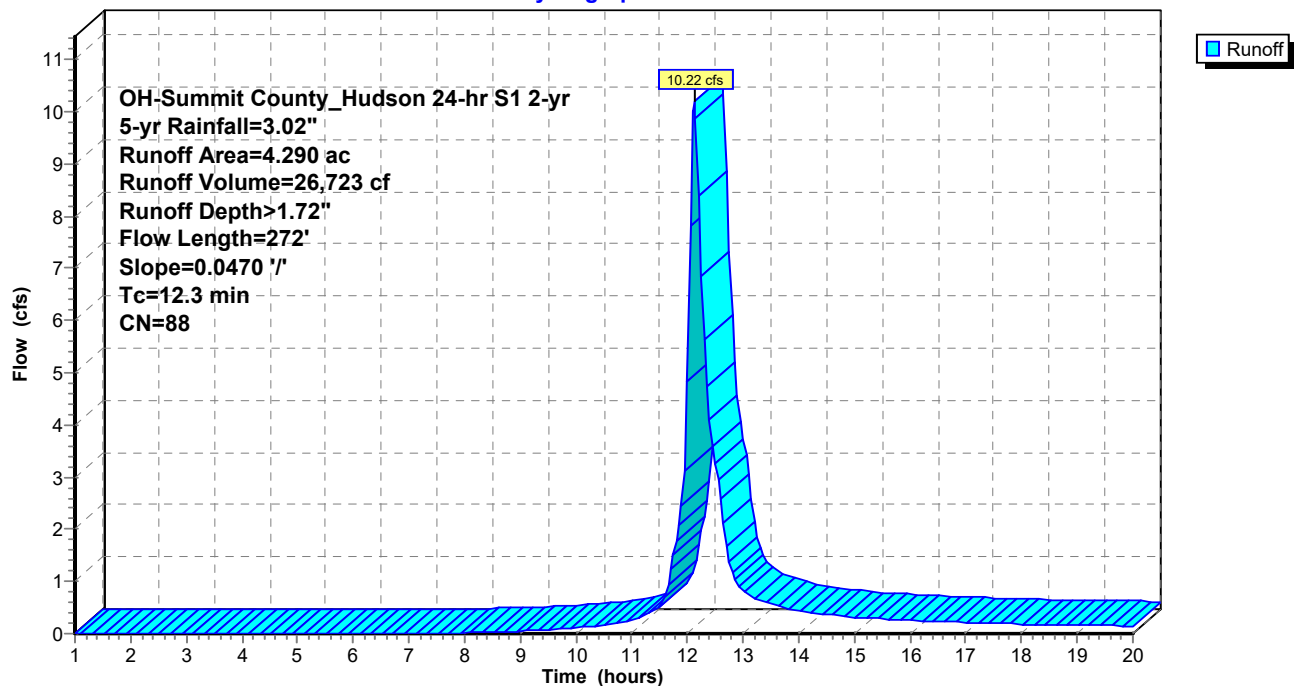
Area (ac)	CN	Description
3.075	84	50-75% Grass cover, Fair, HSG D
1.215	98	Paved parking, HSG D
4.290	88	Weighted Average
3.075		71.68% Pervious Area
1.215		28.32% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
11.0	150	0.0470	0.23		<b>Sheet Flow,</b> Grass: Short n= 0.150 P2= 2.44"
1.3	122	0.0470	1.52		<b>Shallow Concentrated Flow,</b> Short Grass Pasture Kv= 7.0 fps
12.3	272	Total			

**Subcatchment 1.W: EDA-WEST**

Hydrograph



**Summary for Subcatchment 2.W: PDA-WEST**

Runoff = 13.22 cfs @ 12.12 hrs, Volume= 34,933 cf, Depth> 1.96"  
 Routed to Pond 4 : POND

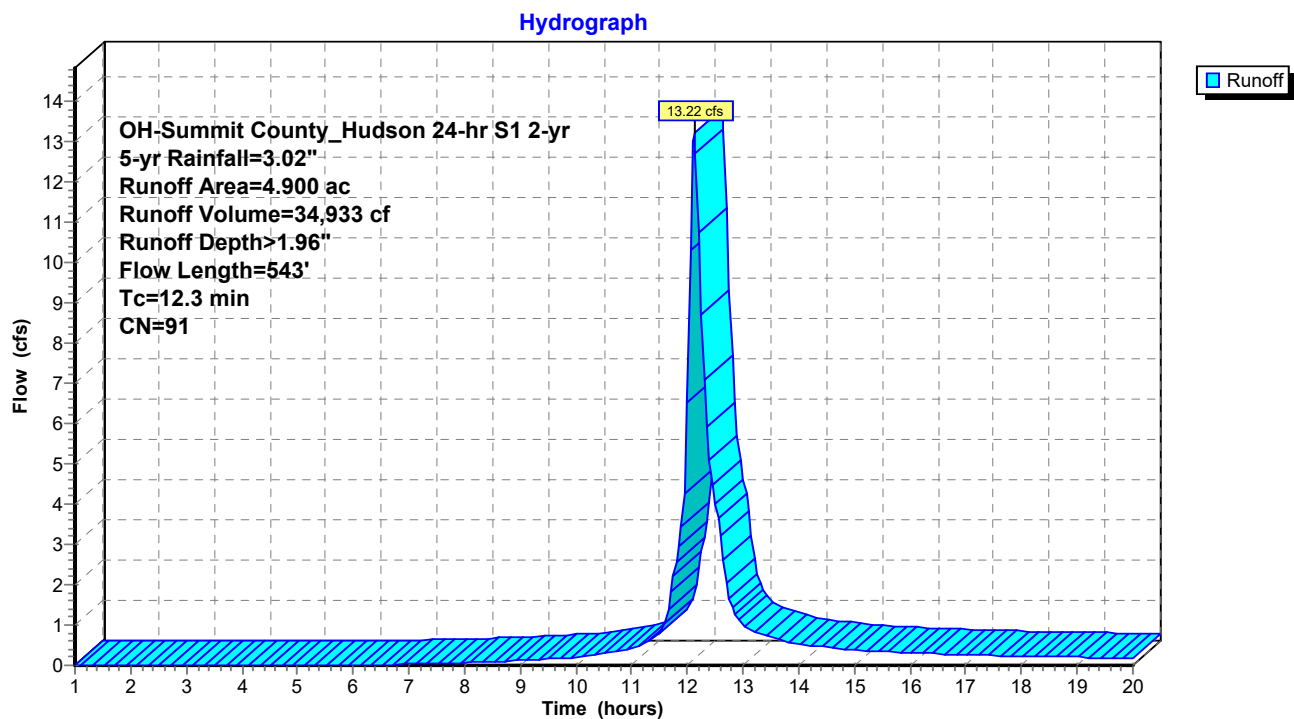
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-20.00 hrs, dt= 0.05 hrs  
 OH-Summit County\_Hudson 24-hr S1 2-yr 5-yr Rainfall=3.02"

Area (ac)	CN	Description
* 1.020	95	Permeable Turf Field, HSG D
2.080	84	50-75% Grass cover, Fair, HSG D
1.800	98	Paved parking, HSG D
4.900	91	Weighted Average
3.100		63.27% Pervious Area
1.800		36.73% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
9.9	43	0.0050	0.07		<b>Sheet Flow,</b> Grass: Short n= 0.150 P2= 2.44"
1.9	300	0.0050	2.63	0.52	<b>Pipe Channel,</b> 6.00" Round Area= 0.2 sf Perim= 1.6' r= 0.13' n= 0.010
0.5	200	0.0100	6.84	8.40	<b>Pipe Channel,</b> 15.00" Round Area= 1.2 sf Perim= 3.9' r= 0.31' n= 0.010
12.3	543	Total			

## Subcatchment 2.W: PDA-WEST



**Summary for Pond 4: POND**

Inflow Area = 213,444 sf, 36.73% Impervious, Inflow Depth > 1.96" for 5-yr event  
 Inflow = 13.22 cfs @ 12.12 hrs, Volume= 34,933 cf  
 Outflow = 4.52 cfs @ 12.45 hrs, Volume= 26,980 cf, Atten= 66%, Lag= 19.8 min  
 Primary = 4.52 cfs @ 12.45 hrs, Volume= 26,980 cf  
 Routed to nonexistent node 5L  
 Secondary = 0.00 cfs @ 1.00 hrs, Volume= 0 cf  
 Routed to nonexistent node 5L

Routing by Stor-Ind method, Time Span= 1.00-20.00 hrs, dt= 0.05 hrs  
 Peak Elev= 1,011.24' @ 12.45 hrs Surf.Area= 9,692 sf Storage= 15,879 cf

Plug-Flow detention time= 100.3 min calculated for 26,909 cf (77% of inflow)  
 Center-of-Mass det. time= 48.9 min ( 821.4 - 772.6 )

Volume	Invert	Avail.Storage	Storage Description
#1	1,008.00'	44,483 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Voids (%)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
1,008.00	2,848	0.0	0	0
1,009.00	2,848	40.0	1,139	1,139
1,010.00	6,536	100.0	4,692	5,831
1,011.00	9,073	100.0	7,805	13,636
1,012.00	11,663	100.0	10,368	24,004
1,013.00	14,311	100.0	12,987	36,991
1,013.50	15,658	100.0	7,492	44,483

Device	Routing	Invert	Outlet Devices
#1	Device 4	1,008.00'	<b>1.44" Vert. Water Quality Orifice</b> C= 0.600 Limited to weir flow at low heads
#2	Secondary	1,013.00'	<b>10.0' long + 3.0 ' SideZ x 4.0' breadth Spillway</b> Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.38 2.54 2.69 2.68 2.67 2.67 2.65 2.66 2.66 2.68 2.72 2.73 2.76 2.79 2.88 3.07 3.32
#3	Device 4	1,012.80'	<b>27.50" x 27.50" Horiz. Rim</b> C= 0.600 Limited to weir flow at low heads
#4	Primary	1,008.00'	<b>18.00" Vert. Outlet</b> C= 0.600 Limited to weir flow at low heads
#5	Device 4	1,010.25'	<b>30.00" W x 5.00" H Vert. WQV Window</b> C= 0.600 Limited to weir flow at low heads
#6	Device 4	1,011.80'	<b>48.00" W x 4.00" H Vert. Orifice/Grate</b> C= 0.600 Limited to weir flow at low heads



**Primary OutFlow** Max=4.52 cfs @ 12.45 hrs HW=1,011.24' (Free Discharge)

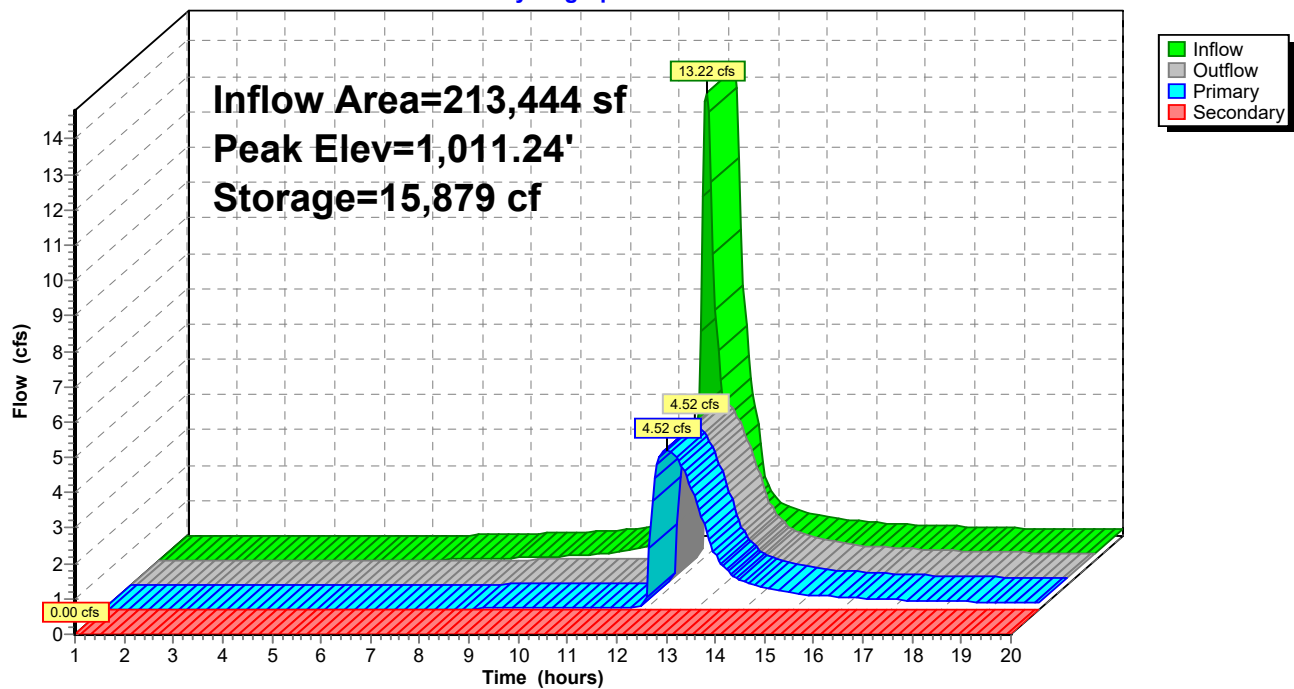
- 4=Outlet (Passes 4.52 cfs of 13.42 cfs potential flow)
- 1=Water Quality Orifice (Orifice Controls 0.10 cfs @ 8.59 fps)
- 3=Rim ( Controls 0.00 cfs)
- 5=WQV Window (Orifice Controls 4.42 cfs @ 4.24 fps)
- 6=Orifice/Grate ( Controls 0.00 cfs)

**Secondary OutFlow** Max=0.00 cfs @ 1.00 hrs HW=1,008.00' (Free Discharge)

- 2=Spillway ( Controls 0.00 cfs)

### Pond 4: POND

#### Hydrograph



### Summary for Subcatchment 1.W: EDA-WEST

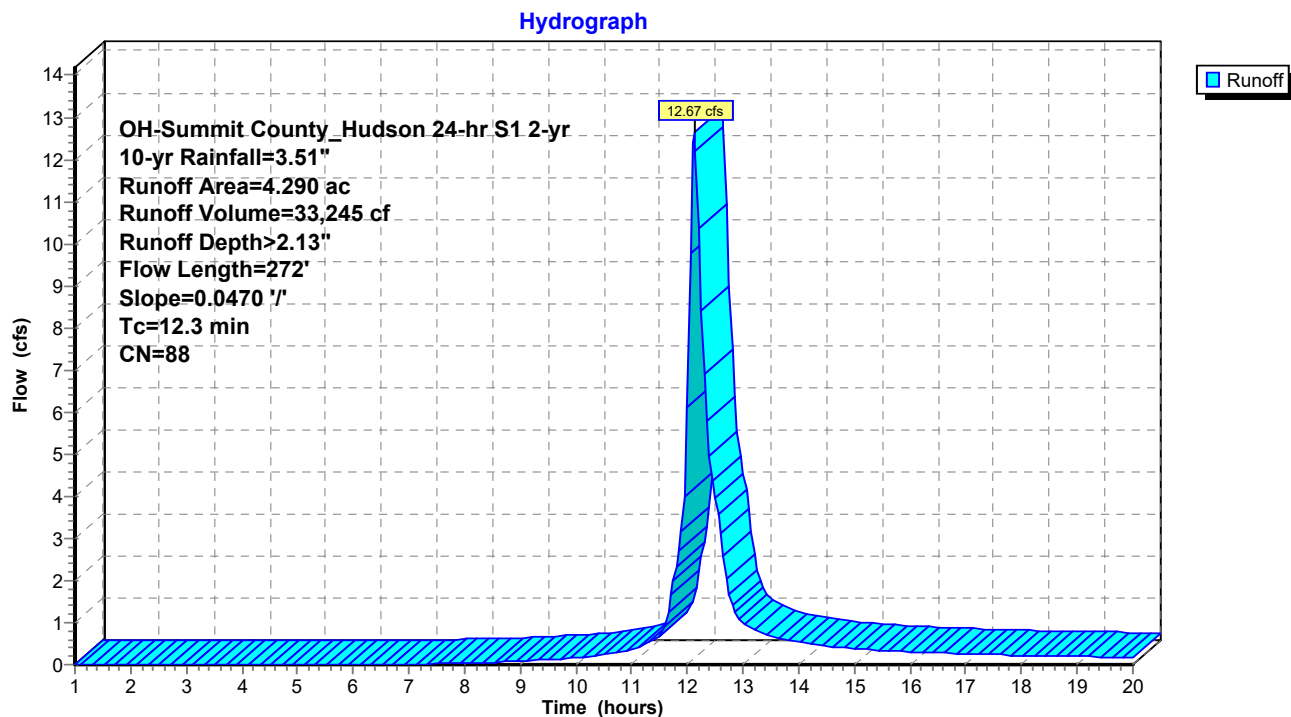
Runoff = 12.67 cfs @ 12.12 hrs, Volume= 33,245 cf, Depth> 2.13"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-20.00 hrs, dt= 0.05 hrs  
 OH-Summit County\_Hudson 24-hr S1 2-yr 10-yr Rainfall=3.51"

Area (ac)	CN	Description
3.075	84	50-75% Grass cover, Fair, HSG D
1.215	98	Paved parking, HSG D
4.290	88	Weighted Average
3.075		71.68% Pervious Area
1.215		28.32% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
11.0	150	0.0470	0.23		<b>Sheet Flow,</b> Grass: Short n= 0.150 P2= 2.44"
1.3	122	0.0470	1.52		<b>Shallow Concentrated Flow,</b> Short Grass Pasture Kv= 7.0 fps
12.3	272	Total			

### Subcatchment 1.W: EDA-WEST



### Summary for Subcatchment 2.W: PDA-WEST

Runoff = 16.06 cfs @ 12.12 hrs, Volume= 42,723 cf, Depth> 2.40"  
 Routed to Pond 4 : POND

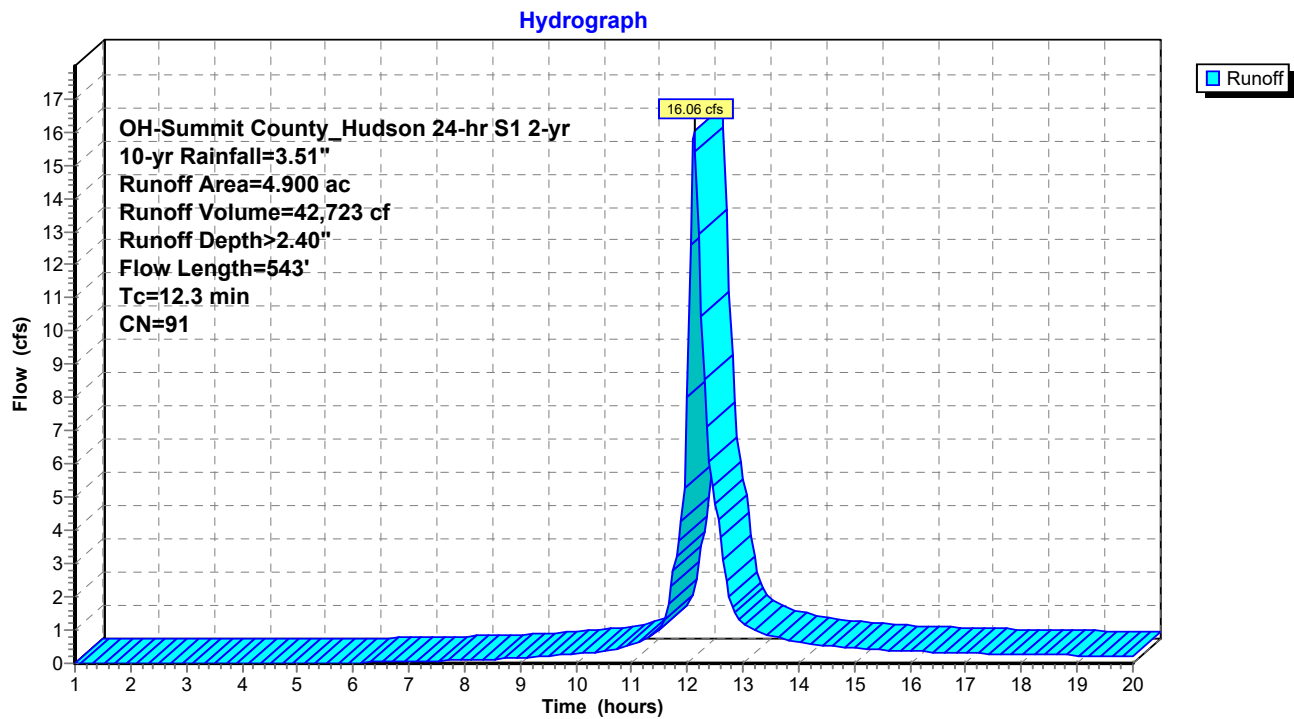
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-20.00 hrs, dt= 0.05 hrs  
 OH-Summit County\_Hudson 24-hr S1 2-yr 10-yr Rainfall=3.51"

Area (ac)	CN	Description
* 1.020	95	Permeable Turf Field, HSG D
2.080	84	50-75% Grass cover, Fair, HSG D
1.800	98	Paved parking, HSG D
4.900	91	Weighted Average
3.100		63.27% Pervious Area
1.800		36.73% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
9.9	43	0.0050	0.07		<b>Sheet Flow,</b> Grass: Short n= 0.150 P2= 2.44"
1.9	300	0.0050	2.63	0.52	<b>Pipe Channel,</b> 6.00" Round Area= 0.2 sf Perim= 1.6' r= 0.13' n= 0.010
0.5	200	0.0100	6.84	8.40	<b>Pipe Channel,</b> 15.00" Round Area= 1.2 sf Perim= 3.9' r= 0.31' n= 0.010
12.3	543	Total			

## Subcatchment 2.W: PDA-WEST



### Summary for Pond 4: POND

Inflow Area = 213,444 sf, 36.73% Impervious, Inflow Depth > 2.40" for 10-yr event  
 Inflow = 16.06 cfs @ 12.12 hrs, Volume= 42,723 cf  
 Outflow = 5.38 cfs @ 12.45 hrs, Volume= 34,696 cf, Atten= 66%, Lag= 19.9 min  
 Primary = 5.38 cfs @ 12.45 hrs, Volume= 34,696 cf  
     Routed to nonexistent node 5L  
 Secondary = 0.00 cfs @ 1.00 hrs, Volume= 0 cf  
     Routed to nonexistent node 5L

Routing by Stor-Ind method, Time Span= 1.00-20.00 hrs, dt= 0.05 hrs  
 Peak Elev= 1,011.57' @ 12.45 hrs Surf.Area= 10,550 sf Storage= 19,233 cf

Plug-Flow detention time= 95.6 min calculated for 34,605 cf (81% of inflow)  
 Center-of-Mass det. time= 48.7 min ( 817.1 - 768.3 )

Volume	Invert	Avail.Storage	Storage Description
#1	1,008.00'	44,483 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Voids (%)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
1,008.00	2,848	0.0	0	0
1,009.00	2,848	40.0	1,139	1,139
1,010.00	6,536	100.0	4,692	5,831
1,011.00	9,073	100.0	7,805	13,636
1,012.00	11,663	100.0	10,368	24,004
1,013.00	14,311	100.0	12,987	36,991
1,013.50	15,658	100.0	7,492	44,483

Device	Routing	Invert	Outlet Devices
#1	Device 4	1,008.00'	<b>1.44" Vert. Water Quality Orifice</b> C= 0.600 Limited to weir flow at low heads
#2	Secondary	1,013.00'	<b>10.0' long + 3.0 ' SideZ x 4.0' breadth Spillway</b> Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.38 2.54 2.69 2.68 2.67 2.67 2.65 2.66 2.66 2.68 2.72 2.73 2.76 2.79 2.88 3.07 3.32
#3	Device 4	1,012.80'	<b>27.50" x 27.50" Horiz. Rim</b> C= 0.600 Limited to weir flow at low heads
#4	Primary	1,008.00'	<b>18.00" Vert. Outlet</b> C= 0.600 Limited to weir flow at low heads
#5	Device 4	1,010.25'	<b>30.00" W x 5.00" H Vert. WQV Window</b> C= 0.600 Limited to weir flow at low heads
#6	Device 4	1,011.80'	<b>48.00" W x 4.00" H Vert. Orifice/Grate</b> C= 0.600 Limited to weir flow at low heads

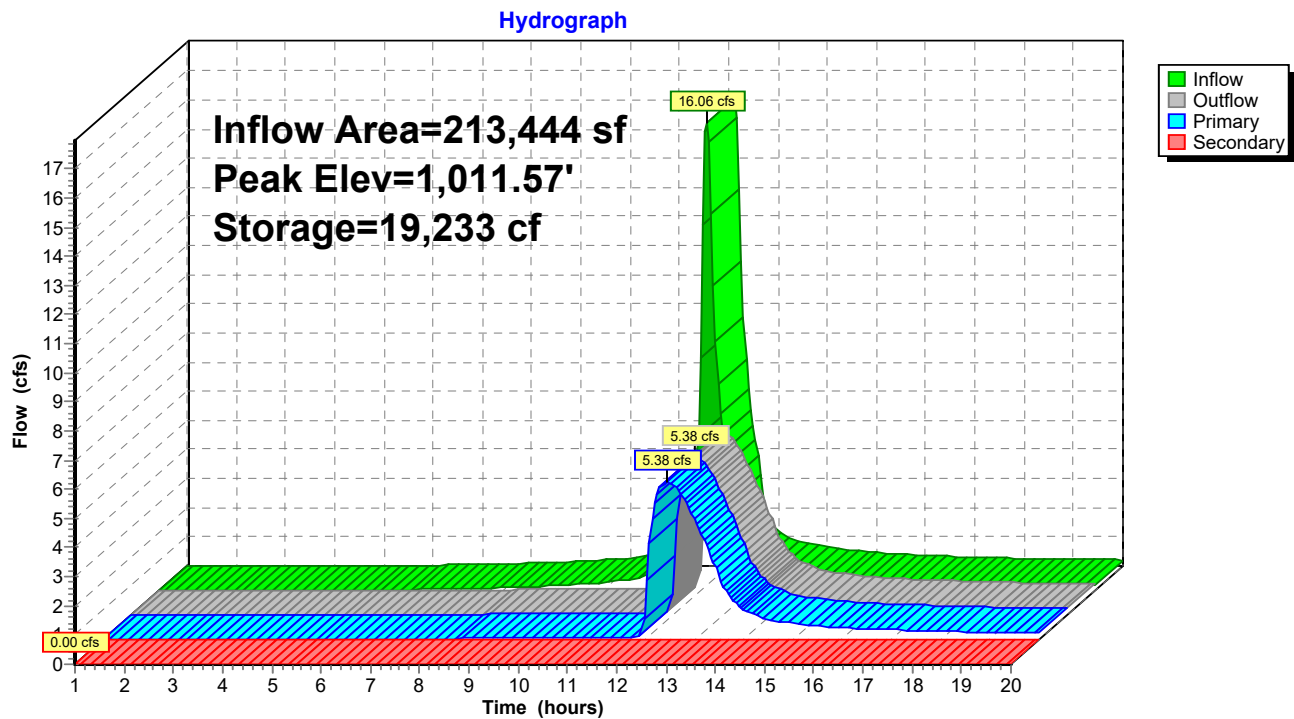
**Primary OutFlow** Max=5.38 cfs @ 12.45 hrs HW=1,011.57' (Free Discharge)

- ↳ **4=Outlet** (Passes 5.38 cfs of 14.29 cfs potential flow)
  - ↳ **1=Water Quality Orifice** (Orifice Controls 0.10 cfs @ 9.02 fps)
  - ↳ **3=Rim** ( Controls 0.00 cfs)
  - ↳ **5=WQV Window** (Orifice Controls 5.28 cfs @ 5.07 fps)
  - ↳ **6=Orifice/Grate** ( Controls 0.00 cfs)

**Secondary OutFlow** Max=0.00 cfs @ 1.00 hrs HW=1,008.00' (Free Discharge)

- ↳ **2=Spillway** ( Controls 0.00 cfs)

### Pond 4: POND



### Summary for Subcatchment 1.W: EDA-WEST

Runoff = 16.25 cfs @ 12.12 hrs, Volume= 42,943 cf, Depth> 2.76"

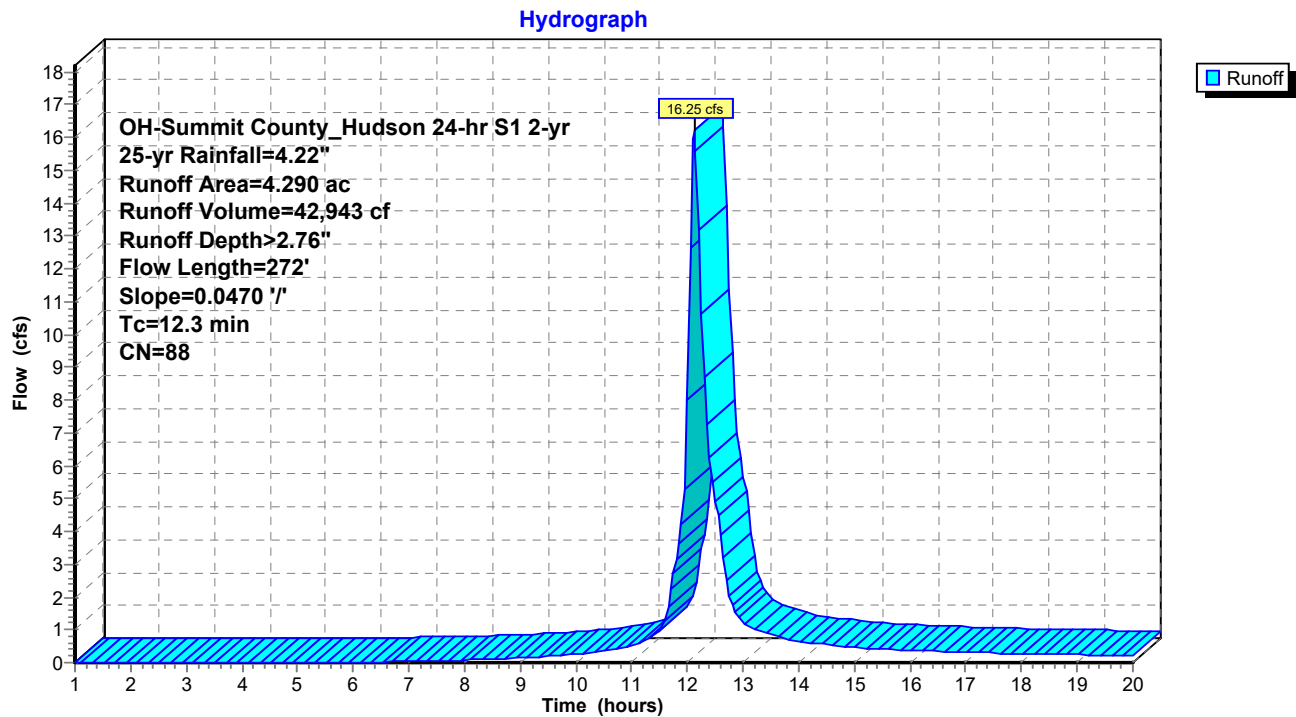
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-20.00 hrs, dt= 0.05 hrs  
 OH-Summit County\_Hudson 24-hr S1 2-yr 25-yr Rainfall=4.22"

Area (ac)	CN	Description
3.075	84	50-75% Grass cover, Fair, HSG D
1.215	98	Paved parking, HSG D
4.290	88	Weighted Average
3.075		71.68% Pervious Area
1.215		28.32% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
11.0	150	0.0470	0.23		<b>Sheet Flow,</b> Grass: Short n= 0.150 P2= 2.44"
1.3	122	0.0470	1.52		<b>Shallow Concentrated Flow,</b> Short Grass Pasture Kv= 7.0 fps
12.3	272	Total			

### Subcatchment 1.W: EDA-WEST



### Summary for Subcatchment 2.W: PDA-WEST

Runoff = 20.16 cfs @ 12.12 hrs, Volume= 54,196 cf, Depth> 3.05"  
 Routed to Pond 4 : POND

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-20.00 hrs, dt= 0.05 hrs  
 OH-Summit County\_Hudson 24-hr S1 2-yr 25-yr Rainfall=4.22"

Area (ac)	CN	Description
* 1.020	95	Permeable Turf Field, HSG D
2.080	84	50-75% Grass cover, Fair, HSG D
1.800	98	Paved parking, HSG D
4.900	91	Weighted Average
3.100		63.27% Pervious Area
1.800		36.73% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
9.9	43	0.0050	0.07		<b>Sheet Flow,</b> Grass: Short n= 0.150 P2= 2.44"
1.9	300	0.0050	2.63	0.52	<b>Pipe Channel,</b> 6.00" Round Area= 0.2 sf Perim= 1.6' r= 0.13' n= 0.010
0.5	200	0.0100	6.84	8.40	<b>Pipe Channel,</b> 15.00" Round Area= 1.2 sf Perim= 3.9' r= 0.31' n= 0.010
12.3	543	Total			



## Subcatchment 2.W: PDA-WEST



### Summary for Pond 4: POND

Inflow Area = 213,444 sf, 36.73% Impervious, Inflow Depth > 3.05" for 25-yr event  
 Inflow = 20.16 cfs @ 12.12 hrs, Volume= 54,196 cf  
 Outflow = 7.30 cfs @ 12.42 hrs, Volume= 46,066 cf, Atten= 64%, Lag= 17.9 min  
 Primary = 7.30 cfs @ 12.42 hrs, Volume= 46,066 cf  
     Routed to nonexistent node 5L  
 Secondary = 0.00 cfs @ 1.00 hrs, Volume= 0 cf  
     Routed to nonexistent node 5L

Routing by Stor-Ind method, Time Span= 1.00-20.00 hrs, dt= 0.05 hrs  
 Peak Elev= 1,011.98' @ 12.42 hrs Surf.Area= 11,618 sf Storage= 23,800 cf

Plug-Flow detention time= 91.0 min calculated for 45,945 cf (85% of inflow)  
 Center-of-Mass det. time= 49.6 min ( 812.8 - 763.3 )

Volume	Invert	Avail.Storage	Storage Description
#1	1,008.00'	44,483 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Voids (%)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
1,008.00	2,848	0.0	0	0
1,009.00	2,848	40.0	1,139	1,139
1,010.00	6,536	100.0	4,692	5,831
1,011.00	9,073	100.0	7,805	13,636
1,012.00	11,663	100.0	10,368	24,004
1,013.00	14,311	100.0	12,987	36,991
1,013.50	15,658	100.0	7,492	44,483

Device	Routing	Invert	Outlet Devices
#1	Device 4	1,008.00'	<b>1.44" Vert. Water Quality Orifice</b> C= 0.600 Limited to weir flow at low heads
#2	Secondary	1,013.00'	<b>10.0' long + 3.0 ' SideZ x 4.0' breadth Spillway</b> Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.38 2.54 2.69 2.68 2.67 2.67 2.65 2.66 2.66 2.68 2.72 2.73 2.76 2.79 2.88 3.07 3.32
#3	Device 4	1,012.80'	<b>27.50" x 27.50" Horiz. Rim</b> C= 0.600 Limited to weir flow at low heads
#4	Primary	1,008.00'	<b>18.00" Vert. Outlet</b> C= 0.600 Limited to weir flow at low heads
#5	Device 4	1,010.25'	<b>30.00" W x 5.00" H Vert. WQV Window</b> C= 0.600 Limited to weir flow at low heads
#6	Device 4	1,011.80'	<b>48.00" W x 4.00" H Vert. Orifice/Grate</b> C= 0.600 Limited to weir flow at low heads

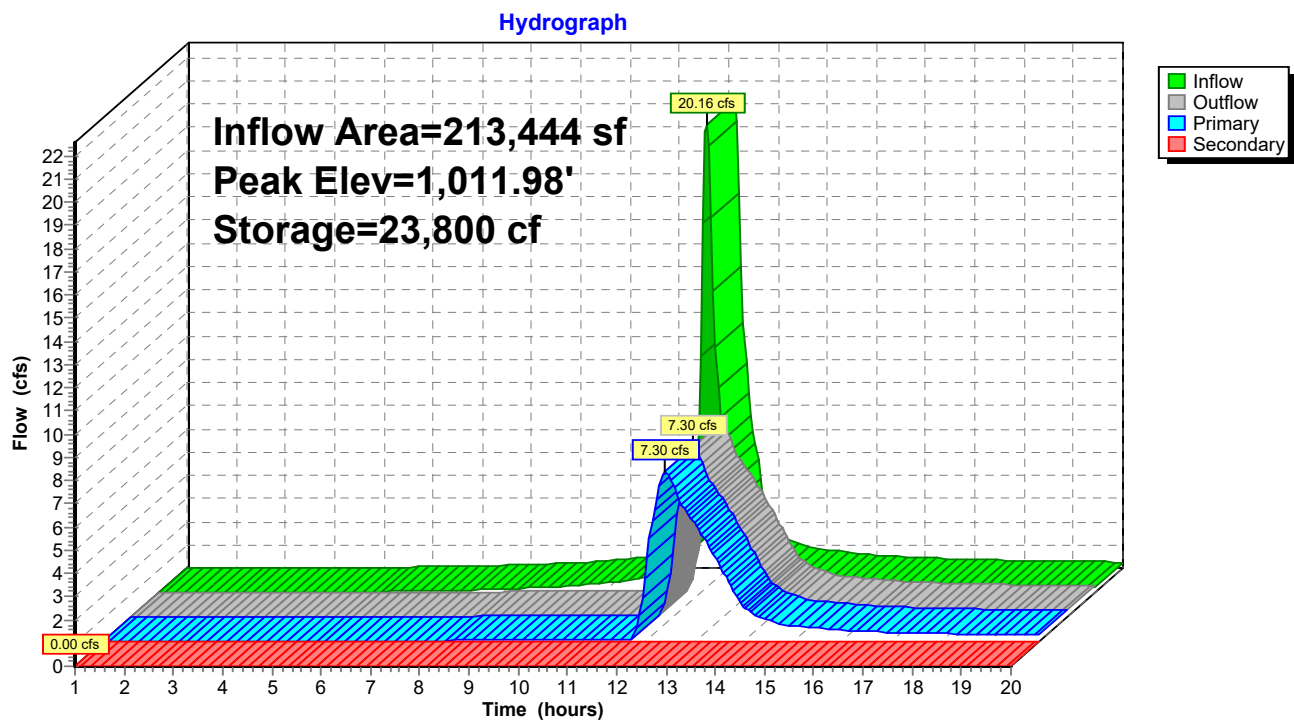
**Primary OutFlow** Max=7.28 cfs @ 12.42 hrs HW=1,011.98' (Free Discharge)

- ↳ **4=Outlet** (Passes 7.28 cfs of 15.29 cfs potential flow)
- ↳ **1=Water Quality Orifice** (Orifice Controls 0.11 cfs @ 9.53 fps)
- ↳ **3=Rim** ( Controls 0.00 cfs)
- ↳ **5=WQV Window** (Orifice Controls 6.18 cfs @ 5.94 fps)
- ↳ **6=Orifice/Grate** (Orifice Controls 0.98 cfs @ 1.36 fps)

**Secondary OutFlow** Max=0.00 cfs @ 1.00 hrs HW=1,008.00' (Free Discharge)

- ↳ **2=Spillway** ( Controls 0.00 cfs)

### Pond 4: POND



### Summary for Subcatchment 1.W: EDA-WEST

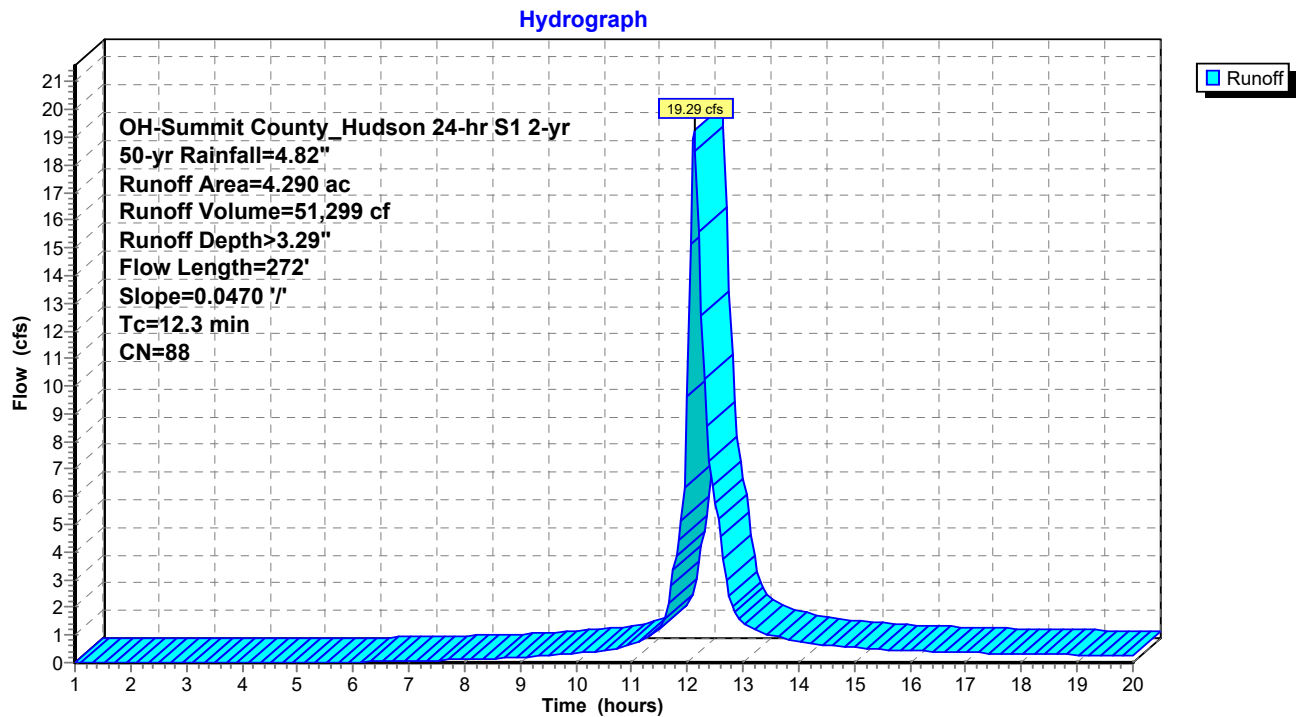
Runoff = 19.29 cfs @ 12.12 hrs, Volume= 51,299 cf, Depth> 3.29"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-20.00 hrs, dt= 0.05 hrs  
 OH-Summit County\_Hudson 24-hr S1 2-yr 50-yr Rainfall=4.82"

Area (ac)	CN	Description
3.075	84	50-75% Grass cover, Fair, HSG D
1.215	98	Paved parking, HSG D
4.290	88	Weighted Average
3.075		71.68% Pervious Area
1.215		28.32% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
11.0	150	0.0470	0.23		<b>Sheet Flow,</b> Grass: Short n= 0.150 P2= 2.44"
1.3	122	0.0470	1.52		<b>Shallow Concentrated Flow,</b> Short Grass Pasture Kv= 7.0 fps
12.3	272	Total			

### Subcatchment 1.W: EDA-WEST



### Summary for Subcatchment 2.W: PDA-WEST

Runoff = 23.62 cfs @ 12.12 hrs, Volume= 64,006 cf, Depth> 3.60"  
 Routed to Pond 4 : POND

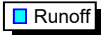
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-20.00 hrs, dt= 0.05 hrs  
 OH-Summit County\_Hudson 24-hr S1 2-yr 50-yr Rainfall=4.82"

Area (ac)	CN	Description
* 1.020	95	Permeable Turf Field, HSG D
2.080	84	50-75% Grass cover, Fair, HSG D
1.800	98	Paved parking, HSG D
4.900	91	Weighted Average
3.100		63.27% Pervious Area
1.800		36.73% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
9.9	43	0.0050	0.07		<b>Sheet Flow,</b> Grass: Short n= 0.150 P2= 2.44"
1.9	300	0.0050	2.63	0.52	<b>Pipe Channel,</b> 6.00" Round Area= 0.2 sf Perim= 1.6' r= 0.13' n= 0.010
0.5	200	0.0100	6.84	8.40	<b>Pipe Channel,</b> 15.00" Round Area= 1.2 sf Perim= 3.9' r= 0.31' n= 0.010
12.3	543	Total			

## Subcatchment 2.W: PDA-WEST



### Summary for Pond 4: POND

Inflow Area = 213,444 sf, 36.73% Impervious, Inflow Depth > 3.60" for 50-yr event  
 Inflow = 23.62 cfs @ 12.12 hrs, Volume= 64,006 cf  
 Outflow = 9.86 cfs @ 12.37 hrs, Volume= 55,791 cf, Atten= 58%, Lag= 14.9 min  
 Primary = 9.86 cfs @ 12.37 hrs, Volume= 55,791 cf  
     Routed to nonexistent node 5L  
 Secondary = 0.00 cfs @ 1.00 hrs, Volume= 0 cf  
     Routed to nonexistent node 5L

Routing by Stor-Ind method, Time Span= 1.00-20.00 hrs, dt= 0.05 hrs  
 Peak Elev= 1,012.21' @ 12.37 hrs Surf.Area= 12,227 sf Storage= 26,549 cf

Plug-Flow detention time= 86.8 min calculated for 55,791 cf (87% of inflow)  
 Center-of-Mass det. time= 48.4 min ( 808.1 - 759.7 )

Volume	Invert	Avail.Storage	Storage Description
#1	1,008.00'	44,483 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Voids (%)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
1,008.00	2,848	0.0	0	0
1,009.00	2,848	40.0	1,139	1,139
1,010.00	6,536	100.0	4,692	5,831
1,011.00	9,073	100.0	7,805	13,636
1,012.00	11,663	100.0	10,368	24,004
1,013.00	14,311	100.0	12,987	36,991
1,013.50	15,658	100.0	7,492	44,483

Device	Routing	Invert	Outlet Devices
#1	Device 4	1,008.00'	<b>1.44" Vert. Water Quality Orifice</b> C= 0.600 Limited to weir flow at low heads
#2	Secondary	1,013.00'	<b>10.0' long + 3.0 ' SideZ x 4.0' breadth Spillway</b> Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.38 2.54 2.69 2.68 2.67 2.67 2.65 2.66 2.66 2.68 2.72 2.73 2.76 2.79 2.88 3.07 3.32
#3	Device 4	1,012.80'	<b>27.50" x 27.50" Horiz. Rim</b> C= 0.600 Limited to weir flow at low heads
#4	Primary	1,008.00'	<b>18.00" Vert. Outlet</b> C= 0.600 Limited to weir flow at low heads
#5	Device 4	1,010.25'	<b>30.00" W x 5.00" H Vert. WQV Window</b> C= 0.600 Limited to weir flow at low heads
#6	Device 4	1,011.80'	<b>48.00" W x 4.00" H Vert. Orifice/Grate</b> C= 0.600 Limited to weir flow at low heads

**Primary OutFlow** Max=9.84 cfs @ 12.37 hrs HW=1,012.21' (Free Discharge)

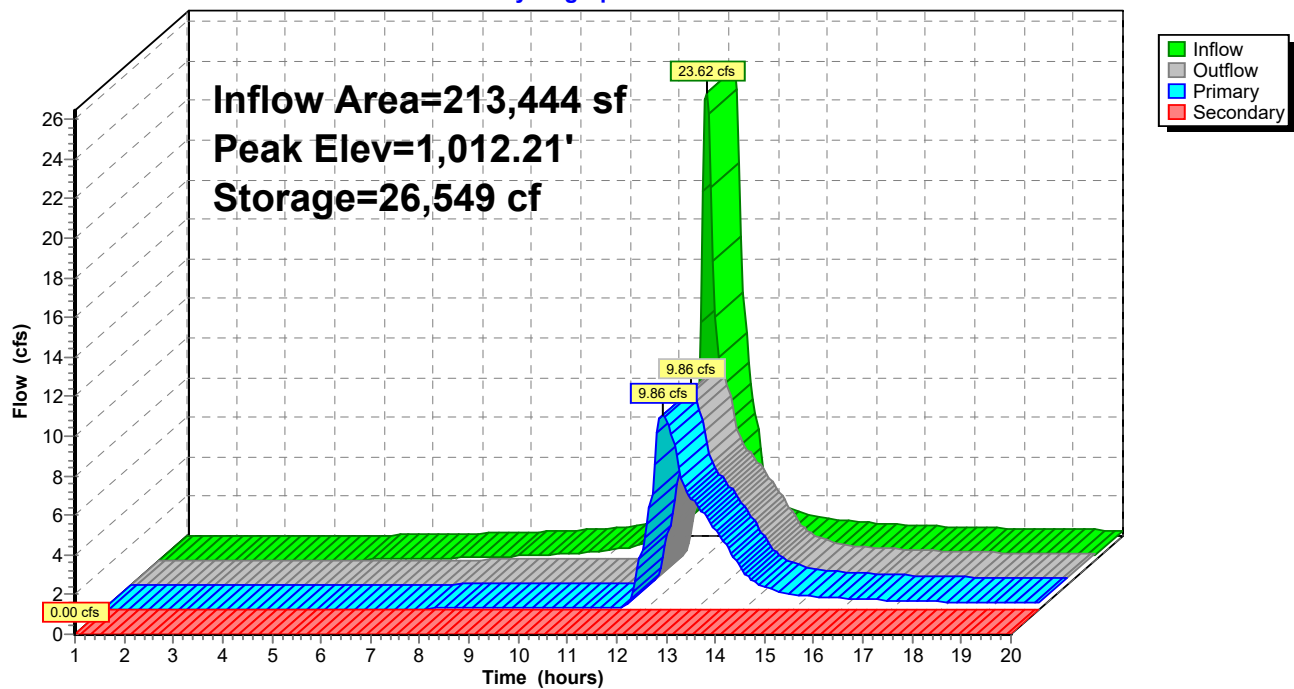
- ↑ **4=Outlet** (Passes 9.84 cfs of 15.83 cfs potential flow)
  - ↑ **1=Water Quality Orifice** (Orifice Controls 0.11 cfs @ 9.81 fps)
  - ↑ **3=Rim** ( Controls 0.00 cfs)
  - ↑ **5=WQV Window** (Orifice Controls 6.63 cfs @ 6.37 fps)
  - ↑ **6=Orifice/Grate** (Orifice Controls 3.10 cfs @ 2.32 fps)

**Secondary OutFlow** Max=0.00 cfs @ 1.00 hrs HW=1,008.00' (Free Discharge)

- ↑ **2=Spillway** ( Controls 0.00 cfs)

### Pond 4: POND

Hydrograph





### Summary for Subcatchment 1.W: EDA-WEST

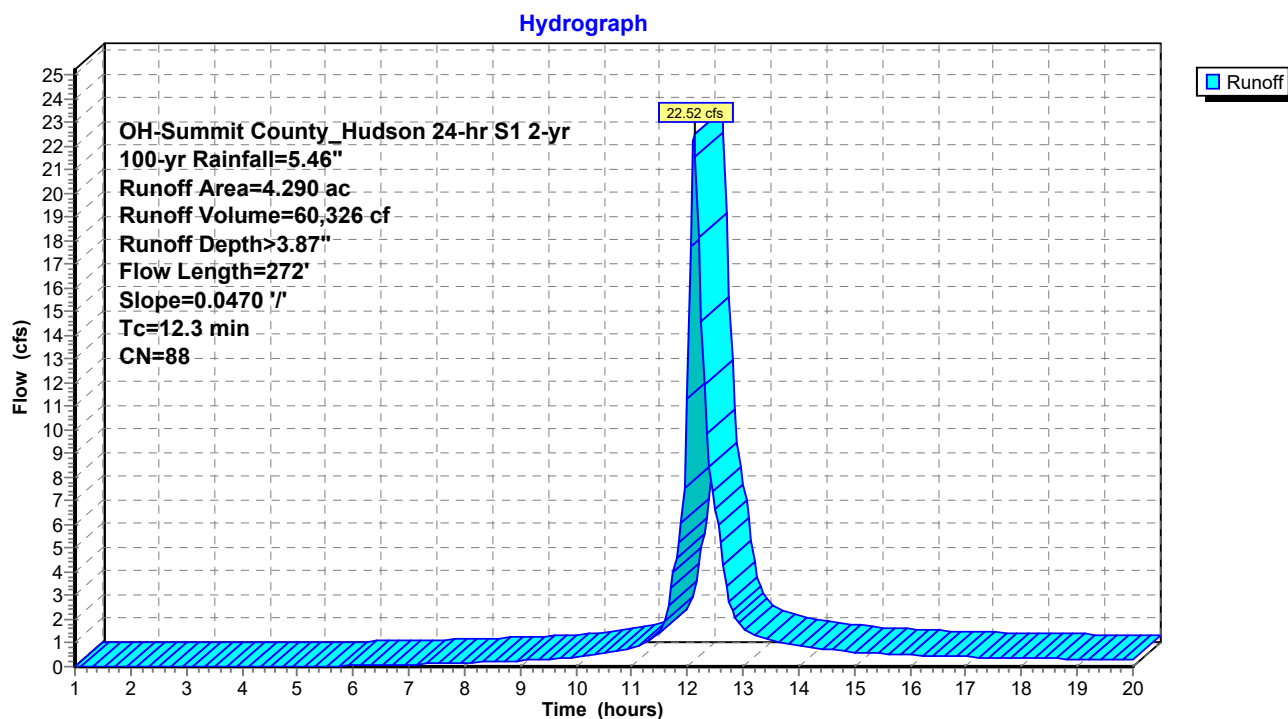
Runoff = 22.52 cfs @ 12.12 hrs, Volume= 60,326 cf, Depth> 3.87"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-20.00 hrs, dt= 0.05 hrs  
 OH-Summit County\_Hudson 24-hr S1 2-yr 100-yr Rainfall=5.46"

Area (ac)	CN	Description
3.075	84	50-75% Grass cover, Fair, HSG D
1.215	98	Paved parking, HSG D
4.290	88	Weighted Average
3.075		71.68% Pervious Area
1.215		28.32% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
11.0	150	0.0470	0.23		<b>Sheet Flow,</b> Grass: Short n= 0.150 P2= 2.44"
1.3	122	0.0470	1.52		<b>Shallow Concentrated Flow,</b> Short Grass Pasture Kv= 7.0 fps
12.3	272	Total			

### Subcatchment 1.W: EDA-WEST



### Summary for Subcatchment 2.W: PDA-WEST

Runoff = 27.29 cfs @ 12.12 hrs, Volume= 74,552 cf, Depth> 4.19"  
 Routed to Pond 4 : POND

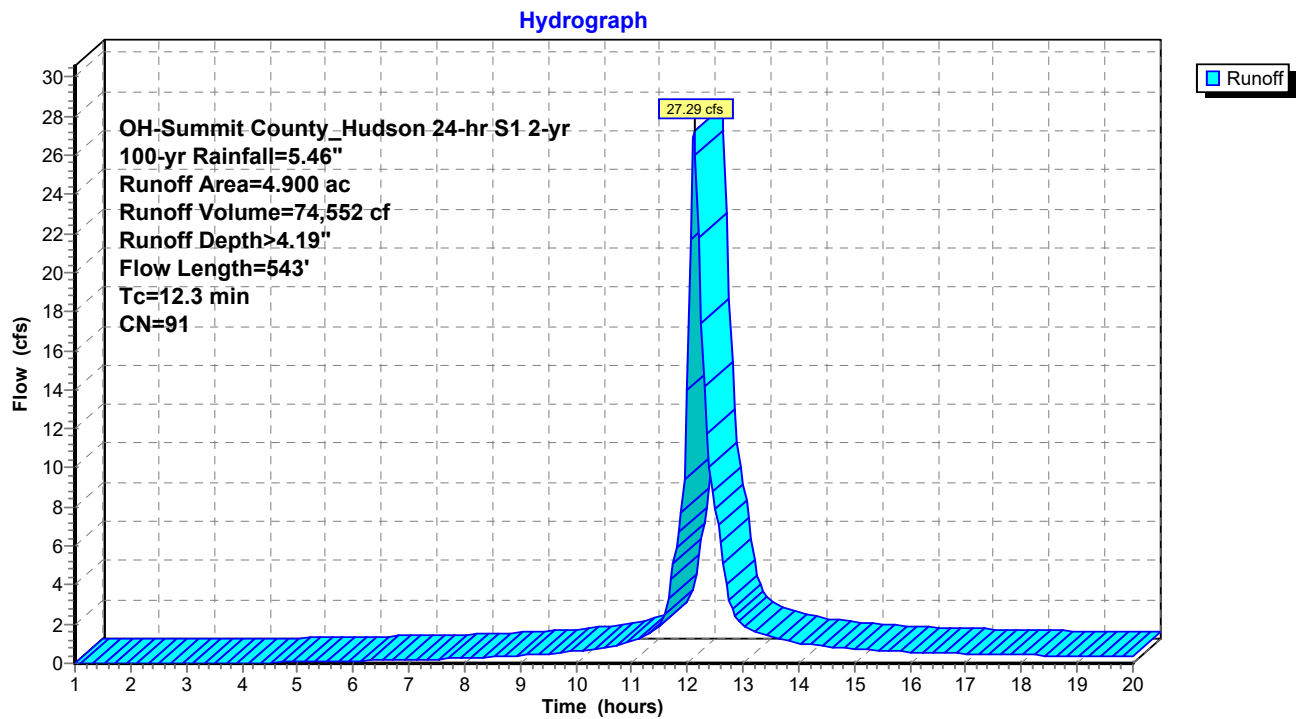
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-20.00 hrs, dt= 0.05 hrs  
 OH-Summit County\_Hudson 24-hr S1 2-yr 100-yr Rainfall=5.46"

Area (ac)	CN	Description
* 1.020	95	Permeable Turf Field, HSG D
2.080	84	50-75% Grass cover, Fair, HSG D
1.800	98	Paved parking, HSG D
4.900	91	Weighted Average
3.100		63.27% Pervious Area
1.800		36.73% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
9.9	43	0.0050	0.07		<b>Sheet Flow,</b> Grass: Short n= 0.150 P2= 2.44"
1.9	300	0.0050	2.63	0.52	<b>Pipe Channel,</b> 6.00" Round Area= 0.2 sf Perim= 1.6' r= 0.13' n= 0.010
0.5	200	0.0100	6.84	8.40	<b>Pipe Channel,</b> 15.00" Round Area= 1.2 sf Perim= 3.9' r= 0.31' n= 0.010
12.3	543	Total			

## Subcatchment 2.W: PDA-WEST



### Summary for Pond 4: POND

Inflow Area = 213,444 sf, 36.73% Impervious, Inflow Depth > 4.19" for 100-yr event  
 Inflow = 27.29 cfs @ 12.12 hrs, Volume= 74,552 cf  
 Outflow = 11.65 cfs @ 12.36 hrs, Volume= 66,246 cf, Atten= 57%, Lag= 14.4 min  
 Primary = 11.65 cfs @ 12.36 hrs, Volume= 66,246 cf  
 Routed to nonexistent node 5L  
 Secondary = 0.00 cfs @ 1.00 hrs, Volume= 0 cf  
 Routed to nonexistent node 5L

Routing by Stor-Ind method, Time Span= 1.00-20.00 hrs, dt= 0.05 hrs  
 Peak Elev= 1,012.45' @ 12.36 hrs Surf.Area= 12,864 sf Storage= 29,567 cf

Plug-Flow detention time= 82.8 min calculated for 66,246 cf (89% of inflow)  
 Center-of-Mass det. time= 47.8 min ( 804.2 - 756.4 )

Volume	Invert	Avail.Storage	Storage Description
#1	1,008.00'	44,483 cf	<b>Custom Stage Data (Prismatic)</b> Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Voids (%)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
1,008.00	2,848	0.0	0	0
1,009.00	2,848	40.0	1,139	1,139
1,010.00	6,536	100.0	4,692	5,831
1,011.00	9,073	100.0	7,805	13,636
1,012.00	11,663	100.0	10,368	24,004
1,013.00	14,311	100.0	12,987	36,991
1,013.50	15,658	100.0	7,492	44,483

Device	Routing	Invert	Outlet Devices
#1	Device 4	1,008.00'	<b>1.44" Vert. Water Quality Orifice</b> C= 0.600 Limited to weir flow at low heads
#2	Secondary	1,013.00'	<b>10.0' long + 3.0 ' SideZ x 4.0' breadth Spillway</b> Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.38 2.54 2.69 2.68 2.67 2.67 2.65 2.66 2.66 2.68 2.72 2.73 2.76 2.79 2.88 3.07 3.32
#3	Device 4	1,012.80'	<b>27.50" x 27.50" Horiz. Rim</b> C= 0.600 Limited to weir flow at low heads
#4	Primary	1,008.00'	<b>18.00" Vert. Outlet</b> C= 0.600 Limited to weir flow at low heads
#5	Device 4	1,010.25'	<b>30.00" W x 5.00" H Vert. WQV Window</b> C= 0.600 Limited to weir flow at low heads
#6	Device 4	1,011.80'	<b>48.00" W x 4.00" H Vert. Orifice/Grate</b> C= 0.600 Limited to weir flow at low heads

**Primary OutFlow** Max=11.64 cfs @ 12.36 hrs HW=1,012.45' (Free Discharge)

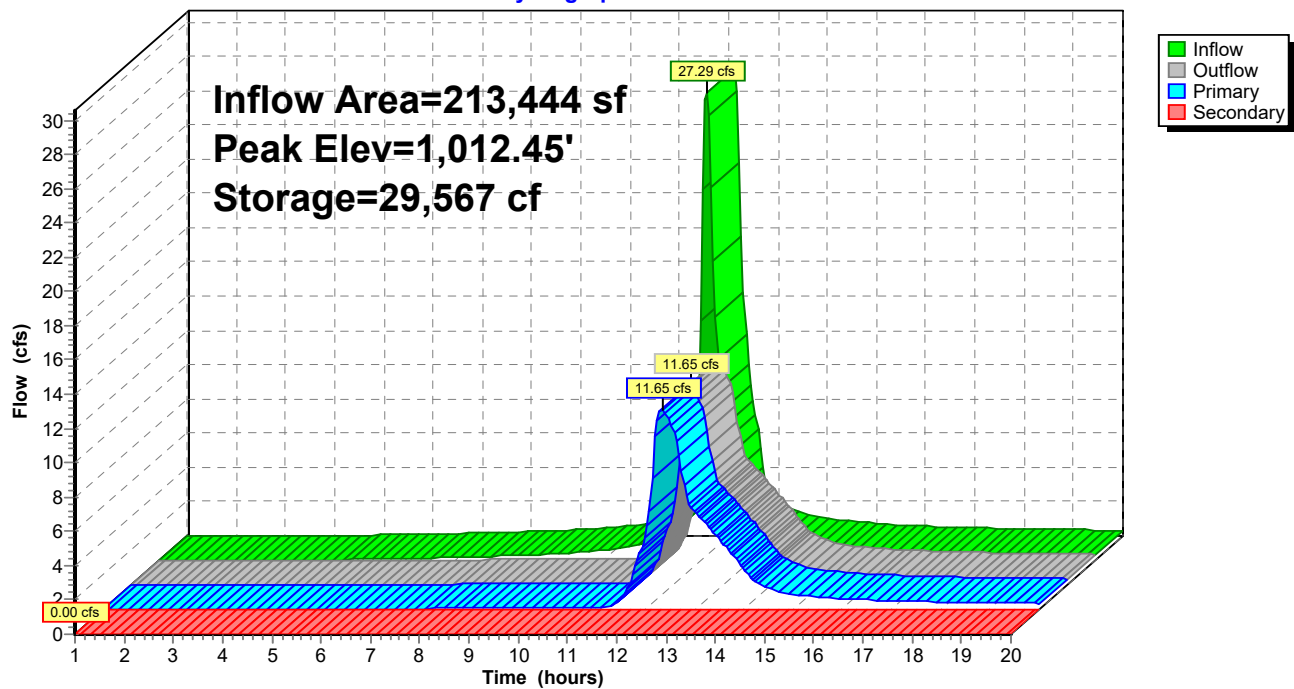
- ↳ **4=Outlet** (Passes 11.64 cfs of 16.37 cfs potential flow)
  - ↳ **1=Water Quality Orifice** (Orifice Controls 0.11 cfs @ 10.09 fps)
  - ↳ **3=Rim** ( Controls 0.00 cfs)
  - ↳ **5=WQV Window** (Orifice Controls 7.08 cfs @ 6.79 fps)
  - ↳ **6=Orifice/Grate** (Orifice Controls 4.45 cfs @ 3.34 fps)

**Secondary OutFlow** Max=0.00 cfs @ 1.00 hrs HW=1,008.00' (Free Discharge)

- ↳ **2=Spillway** ( Controls 0.00 cfs)

### Pond 4: POND

#### Hydrograph



**Events for Subcatchment 1.W: EDA-WEST**

Event	Rainfall (inches)	Runoff (cfs)	Volume (cubic-feet)	Depth (inches)
1-yr	2.04	5.42	14,389	0.92
2-yr	2.44	7.38	19,275	1.24
5-yr	3.02	10.22	26,723	1.72
10-yr	3.51	12.67	33,245	2.13
25-yr	4.22	16.25	42,943	2.76
50-yr	4.82	19.29	51,299	3.29
100-yr	<b>5.46</b>	<b>22.52</b>	<b>60,326</b>	<b>3.87</b>

**Events for Subcatchment 2.W: PDA-WEST**

Event	Rainfall (inches)	Runoff (cfs)	Volume (cubic-feet)	Depth (inches)
1-yr	2.04	7.62	19,900	1.12
2-yr	2.44	9.89	25,919	1.46
5-yr	3.02	13.22	34,933	1.96
10-yr	3.51	16.06	42,723	2.40
25-yr	4.22	20.16	54,196	3.05
50-yr	4.82	23.62	64,006	3.60
100-yr	<b>5.46</b>	<b>27.29</b>	<b>74,552</b>	<b>4.19</b>

**Events for Pond 4: POND**

Event	Inflow (cfs)	Outflow (cfs)	Primary (cfs)	Secondary (cfs)	Elevation (feet)	Storage (cubic-feet)
1-yr	7.62	2.02	2.02	<b>0.00</b>	1,010.64	10,510
2-yr	9.89	3.21	3.21	0.00	1,010.86	12,348
5-yr	13.22	4.52	4.52	0.00	1,011.24	15,879
10-yr	16.06	5.38	5.38	0.00	1,011.57	19,233
25-yr	20.16	7.30	7.30	0.00	1,011.98	23,800
50-yr	23.62	9.86	9.86	0.00	1,012.21	26,549
100-yr	<b>27.29</b>	<b>11.65</b>	<b>11.65</b>	0.00	<b>1,012.45</b>	<b>29,567</b>



**APPENDIX B1:  
STORMWATER QUALITY CALCULATIONS**

## Post-Construction Water Quality Volume As Required Under Ohio NPDES Construction General Permit No. OHC000006

version 1.2 2023-5-15

This spreadsheet calculates the Water Quality Volume required for both new development and redevelopment projects. Green boxes indicate user input for 1) the total area disturbed, 2) planned total impervious surface and, if redevelopment, 3) total existing impervious surface, each in acres. The user must select new or redevelopment from the dropdown menu to apply the proper equation. Use the separate BMP Compliance Spreadsheets to verify a designed practice or combination of practices meets the applicable requirements including the required Water Quality Volume calculated here. This spreadsheet does not account for factors that may affect the final practice design, including offsite run-on or sediment storage volume.

### Project Details

Project Name:	Christ Community Chapel		
Project ID:			
Project Location:	750 W Streetsboro St Hudson, OH 44236		
Project Latitude:	41.23116	Longitude:	-81.48405
NPDES Permit Applicant:			
Submitted By:	5/19/2025		
Date:	5/14/2025		

### Required Water Quality Volume Calculation

Total Disturbed Area, A = 3.930 acres

Type of Development: Redevelopment ▼

Water Quality Volume Equation:  $WQvr = 0.90 \text{ in.} * A * [(Rv1 * 0.2) + (Rv2 - Rv1)] / 12$  [Equation 3]  
where,  $Rv = 0.05 + 0.9(i)$

#### PRE-CONSTRUCTION CONDITIONS

Ex. Impervious Surface = 0.100 acres  
Ex. Impervious Fraction, i = 0.025  
Rv1 = 0.073

#### PROPOSED POST-CONSTRUCTION CONDITIONS

Total Impervious Surface Area = 1.910 acres  
Impervious Fraction, i = 0.486  
Volumetric Runoff Coefficient, Rv2 = 0.487  
 $\Delta Rv = 569 \%$

Water Quality Volume, WQv = 0.126 ac-ft = 5,509 cu. ft.

### Message Center:

*The minimum impervious area to treat with a practice is 1.775 acres*

## version 3.2 2020-07-07

Subwatershed Drainage Area, $A_{\text{total}}$ =	5.00	acres	=	217,800	ft <sup>2</sup>
Subwatershed Impervious Area, $A_{\text{imp}}$ =	2.82	acres	=	122,839	ft <sup>2</sup>
Imperviousness fraction, $i$ =	0.56			56	%
Water Quality Volume, $WQ_v$ =	9,108	ft <sup>3</sup>	=	0.21	ac-ft

Soil Series		HSG	D
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Extended Detention Volume, EDv =	9108	ft <sup>3</sup>
Minimum Sediment Storage Volume, V <sub>sediment</sub> =	1822	ft <sup>3</sup>
Minimum Forebay Volume, V <sub>forebay</sub> =	911	ft <sup>3</sup>
Minimum Permanent Micropool Volume, V <sub>micropool</sub> =	911	ft <sup>3</sup>

[illegible]

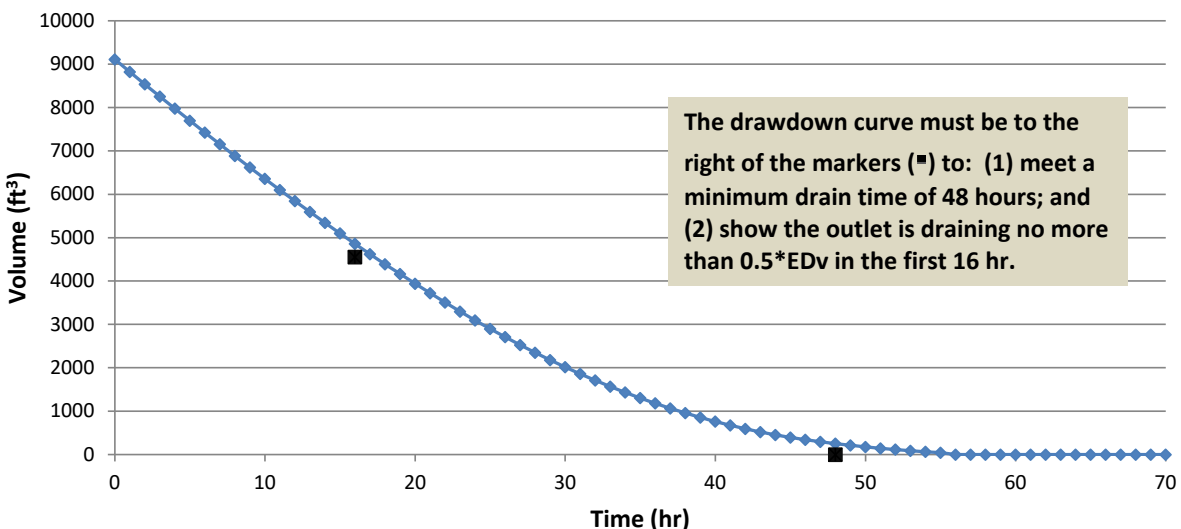
#### Step 4 - Outlet Elevations and Storage Volumes

WQ Orifice Invert Elevation =	1008.00		
Elevation of Top of EDv =	1010.25		
Secondary Outlet Invert Elevation =	1010.25		OKAY
WQ Treatment Volume Provided, $V_{\text{treatment}}$ =	9,122	ft <sup>3</sup>	
Treatment Vol Provided Relative to EDv, $V_{\text{treatment}}/\text{EDv}$ =	1.00		= 100% OKAY
Permanent Pool Volume Provided, PPV =	2,848	ft <sup>3</sup>	
Forebay Volume Provided, $V_{\text{forebay}}$ =	1,139	ft <sup>3</sup>	= 1.25
Is forebay volume below WQ outlet? (Yes or No)	Yes		= 125% OKAY
Permanent Micropool Volume Provided, $V_{\text{micropool}}$ =	1,709	ft <sup>3</sup>	
Ratio $V_{\text{micropool}}$ Provided to $V_{\text{micropool}}$ Required =	1.88		= 188% OKAY
Sediment Storage Volume Provided, $V_{\text{sediment}}$ =	2,848	ft <sup>3</sup>	
Ratio $V_{\text{sediment}}$ Provided to $V_{\text{sediment}}$ Required =	1.56		= 156% OKAY

#### Step 5 - Outlet (Orifice) Sizing

Maximum Hydraulic Head, $H_{\text{max}}$ =	2.25	ft	
Orifice Coefficient, $C$ =	0.6		
Target (Minimum) Draw-down Time, $T_d$ =	48	hr	
Target Average Discharge, $Q_{\text{avg}}$ =	0.05	cfs	
Average Hydraulic Head, $H_{\text{avg}}$ =	1.13	ft	
Estimated Orifice Area, $A_{\text{orifice}}$ =	1.49	in <sup>2</sup>	= 0.010 ft <sup>2</sup>
Estimated Orifice Diameter, $D_{\text{orifice}}$ =	1.38	in	= 0.11 ft
Design Orifice Diameter, $D_{\text{orifice}}$ =	1.44	in	= 0.12 ft
Design Orifice Area, $A_{\text{orifice}}$ =	1.62	in <sup>2</sup>	= 0.011 ft <sup>2</sup>
Time to Completely Drain EDv, $T_d$ =	56	hr	must be $\geq 48$ hr OKAY
Volume Drained in First 16 hr =	4,251	ft <sup>3</sup>	
% of EDv =	46.7	%	must be $\leq 50\%$ OKAY

#### Dry Basin - EDv Drawdown vs Time



## **APPENDIX C: STORMWATER PIPE CALCULATIONS**

Line No.	Line ID	Line Length (ft)	Line Size (in)	Line Slope (%)	Drng Area (ac)	Total Area (ac)	Flow Rate (cfs)	Capac Full (cfs)	Invert Dn (ft)	Invert Up (ft)	HGL Dn (ft)	Gnd/Rim El Dn (ft)	HGL Up (ft)	Gnd/Rim El Up (ft)	Junct Type	Inlet Depth (ft)	Vel Ave (ft/s)	Cover Up (ft)	Tc (min)	
1	35	33.236	18	1.50	0.52	1.32	4.03	12.88	1009.50	1010.00	1010.08	1011.71	1010.77	1014.95	Comb.	0.30	5.43	3.45	7.4	
2	104	58.165	15	1.00	0.06	0.80	2.35	6.45	1010.00	1010.58	1010.77	1014.95	1011.19 j	1014.25	Comb.	0.10	3.46	2.42	6.9	
3	24	55.426	15	1.01	0.06	0.74	2.29	6.49	1010.58	1011.14	1011.19	1014.25	1011.75 j	1014.44	Comb.	0.10	3.86	2.05	6.5	
4	25	52.202	15	1.00	0.55	0.68	2.23	6.44	1011.14	1011.66	1011.75	1014.44	1012.26 j	1015.25	Comb.	0.30	3.83	2.34	6.0	
5	26	25.304	12	0.99	0.13	0.13	0.33	3.54	1011.66	1011.91	1012.26	1015.25	1012.15	1015.75	Comb.	0.13	1.50	2.84	5.0	
6	27	29.000	24	0.52	0.10	0.57	11.61	16.27	1010.00	1010.15	1011.25	1012.71	1011.40	1013.89	Comb.	0.11	5.62	1.74	6.2	
7	28	164.000	24	0.50	0.13	0.47	11.50	15.99	1010.14	1010.96	1011.65	1013.89	1012.18	1014.88	Comb.	0.12	5.13	1.92	5.4	
8	148	95.991	24	0.50	0.34	0.34	11.24	15.99	1010.96	1011.44	1012.94	1014.88	1013.11	1014.69	Comb.	0.22	3.80	1.25	5.0	
9	150	7.012	18	12.26	0.00	0.00	10.50	36.77	1011.44	1012.30	1013.48	1014.69	1013.54	1016.41	MH	....	6.32	2.61	0.0	
10	153	34.919	15	9.99	0.10	0.10	0.14	20.41	1006.02	1009.51	1006.10	1017.08	1009.66	1019.50	Comb.	0.11	3.30	8.74	5.0	

Project File: stm.stm

Number of lines: 10

Date: 5/9/2025

NOTES: \*\* Critical depth

**APPENDIX D:**  
**USDA NRCS Web Soil Survey**



# Soil Map—Summit County, Ohio (CCC Hudson, OH)





## MAP LEGEND

### Area of Interest (AOI)

 Area of Interest (AOI)

### Soils

 Soil Map Unit Polygons

 Soil Map Unit Lines

 Soil Map Unit Points

### Special Point Features



Blowout



Borrow Pit



Clay Spot



Closed Depression



Gravel Pit



Gravelly Spot



Landfill



Lava Flow



Marsh or swamp



Mine or Quarry



Miscellaneous Water



Perennial Water



Rock Outcrop



Saline Spot



Sandy Spot



Severely Eroded Spot



Sinkhole



Slide or Slip



Sodic Spot



Spoil Area



Stony Spot



Very Stony Spot



Wet Spot



Other



Special Line Features

### Water Features



Streams and Canals

### Transportation



Rails



Interstate Highways



US Routes



Major Roads



Local Roads

### Background



Aerial Photography

## MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:20,000.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service

Web Soil Survey URL:

Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: Summit County, Ohio

Survey Area Data: Version 21, Aug 29, 2024

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Sep 12, 2020—Sep 21, 2020

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

## Map Unit Legend

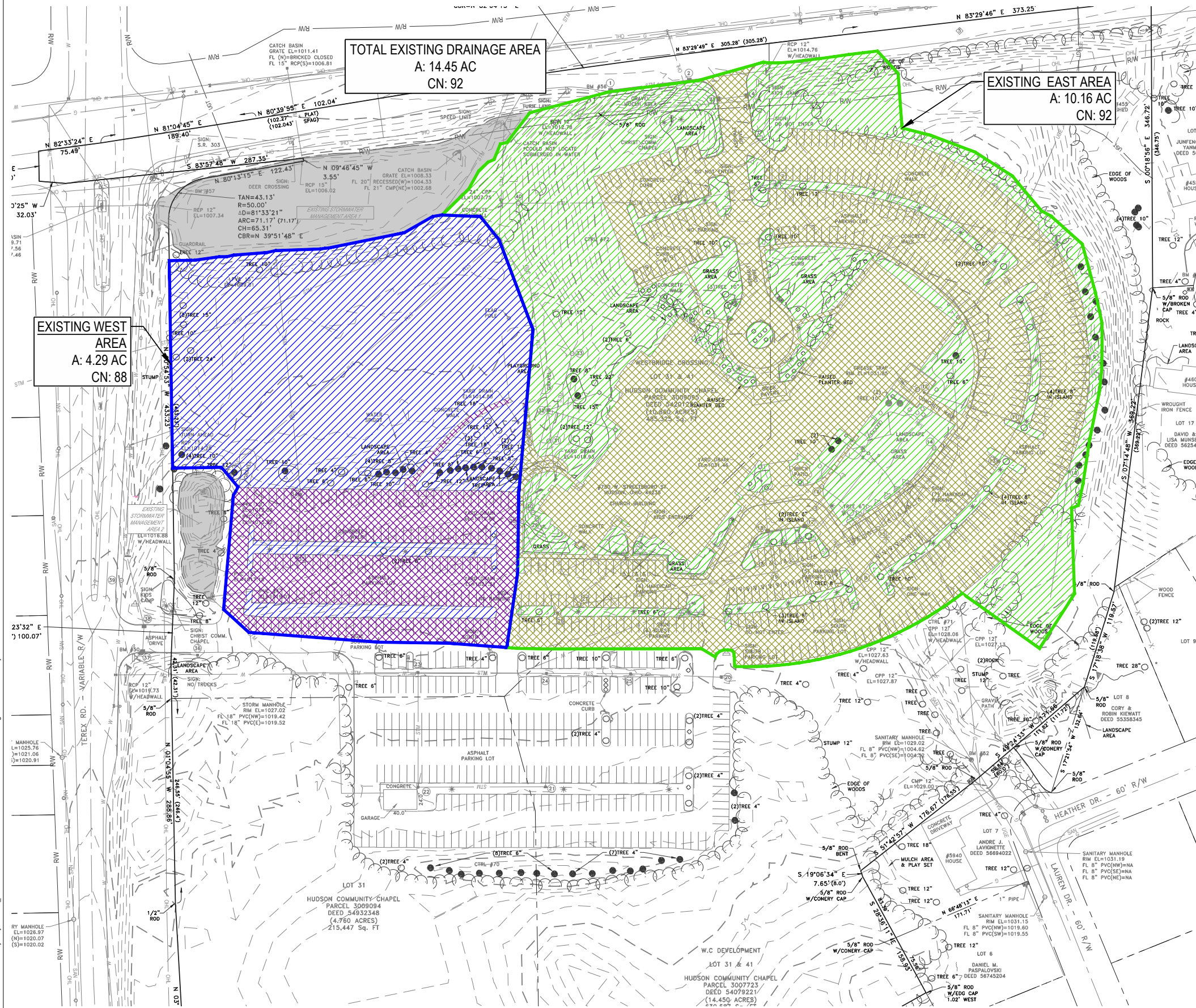
Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
Ca	Canadice silty clay loam	0.2	0.5%
CcB	Caneadea silt loam, 2 to 6 percent slopes	15.1	46.9%
CoC2	Chili gravelly loam, 6 to 12 percent slopes, moderately eroded	2.2	6.9%
EuC	Ellsworth-Urban land complex, 6 to 18 percent slopes	2.3	7.2%
GbC2	Geeburg silt loam, 6 to 12 percent slopes, moderately eroded	6.2	19.3%
GbD2	Geeburg silt loam, 12 to 18 percent slopes, moderately eroded	0.5	1.6%
Mn	Mahoning-Urban land complex, 0 to 2 percent slopes	2.9	9.0%
Sb	Sebring silt loam, 0 to 2 percent slopes	0.6	1.9%
WrB	Wheeling silt loam, 2 to 6 percent slopes	2.2	6.7%
<b>Totals for Area of Interest</b>		<b>32.2</b>	<b>100.0%</b>



**APPENDIX E:  
DRAINAGE AREA MAPS**



**APPENDIX E1:  
EXISTING CONDITIONS DRAINAGE AREA MAP**



C:\D:\CADD\Drawings\CESO\Hudson - Civil Master Plan Study\Project Files\CESO\03-CIVIL\DATA\ASTM\76295 - EXISTING DA MAP.dwg - 5/9/2025 - Tommy Pillow



EXISTING WEST AREAS (AC)		
PERVIOUS GRASS AREA	IMPERVIOUS	TOTAL AREA
		
HSG D / CN: 84	HSG D / CN: 98	
3.08	1.22	4.29

EXISTING EAST AREAS (AC)		
PERVIOUS GRASS AREA	IMPERVIOUS	TOTAL AREA
		
HSG D / CN: 84	HSG D / CN: 98	
4.10	6.06	10.16

NOTE: ALL SOILS WERE ASSUMED TO BE GROUP "D" FOR PRELIMINARY CALCULATION PURPOSES.



CESO  
WWW.CESONC.COM

175 Monroeville West Ave., Suite 400  
Altoona, OH 44201  
Phone: 330.665.0960 Fax: 888.208.4826



2025-03-14

SOL HARRIS/DAY ARCHITECTURE

# CHRIST COMMUNITY CHAPEL

750 W. STREETSBORO STREET  
HUDSON, OH 44236

Revisions / Submissions

ID	Description	Date
----	-------------	------

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Project Number: 765295

Scale: AS SHOWN

Drawn By: JWH

Checked By: JTK

Date: 5/19/2025

Issue: PERMIT SET

Drawing Title:

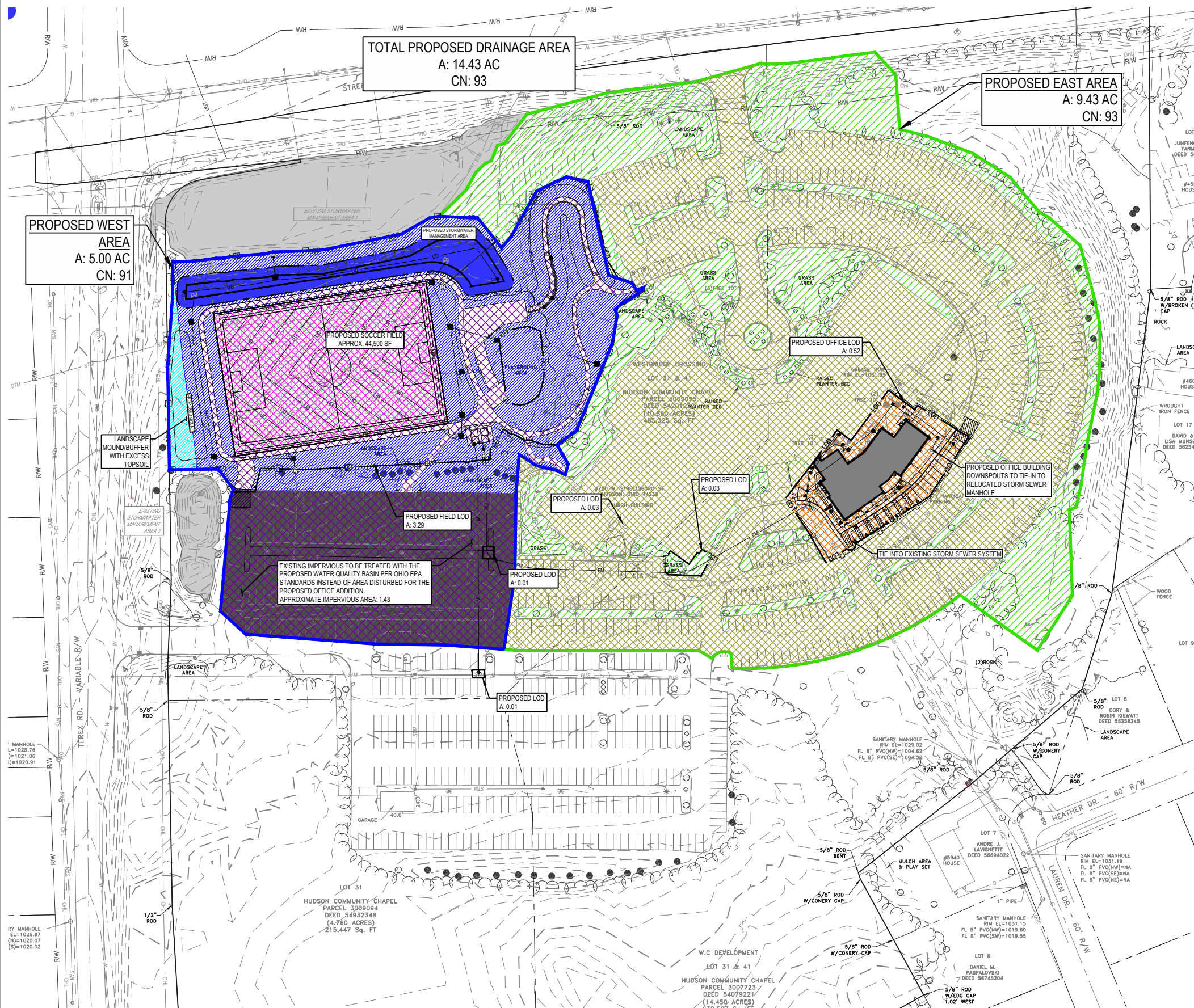
EXISTING DRAINAGE PLAN

EDP

**APPENDIX E2:  
PROPOSED CONDITIONS DRAINAGE AREA MAP**



C:\D:\CC\Draws\CESO\CCC - Hudson - Civil Master Plan Study\Project Files\CESO\103-CIVIL\DATA\ASTM\766295 - PROPOSED DA MAP.dwg - 07/07/25 - Tommy Pilow



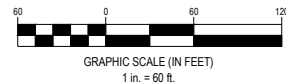
PROPOSED WEST AREAS (AC)					
PERVIOUS AREA (UNDETAINED)	PERVIOUS GRASS AREA	IMPERVIOUS	TURF SOCCER FIELD	POND	TOTAL AREA
HSG D / CN: 84	HSG D / CN: 84	HSG D / CN: 98	HSG D / CN: 95	HSG D / CN: 98	
0.10	1.72	1.80	1.02	0.36	5.00

PROPOSED EAST AREAS (AC)			
PERVIOUS GRASS AREA	IMPERVIOUS	ASSUMED IMPERVIOUS (BLDG ADDITION)	TOTAL AREA
HSG D / CN: 84	HSG D / CN: 98	HSG D / CN: 98	
2.87	5.97	0.52	9.36

FIELD LOD	3.29 AC
OFFICE LOD	0.52 AC
TOTAL LOD	3.81 AC

IMPERVIOUS AREA REQUIRED TO BE DETAINED  
OFFICE ADDITION = 0.52 A C  
TURF FIELD = 1.02 ACRES  
IMPERVIOUS WITHIN LOD FOR FIELD = 0.37 ACRES  
TOTAL REQUIRED = 1.91 ACRES

NOTE: ALL SOILS WERE ASSUMED TO BE GROUP "D" FOR PRELIMINARY CALCULATION PURPOSES.



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SOL HARRIS/DAY ARCHITECTURE

**CHRIST COMMUNITY CHAPEL  
FIELD, WALKING PATH &  
RESTROOM BUILDING**

750 W. STREETSBORO STREET  
HUDSON, OH 44236

Revisions / Submissions

ID	Description	Date
----	-------------	------

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Project Number: 765295

Scale: AS SHOWN

Drawn By: JWH

Checked By: JTK

Date: 06/27/2025

Issue: FOR CONSTRUCTION

Drawing Title:

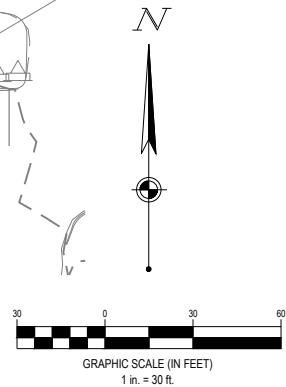
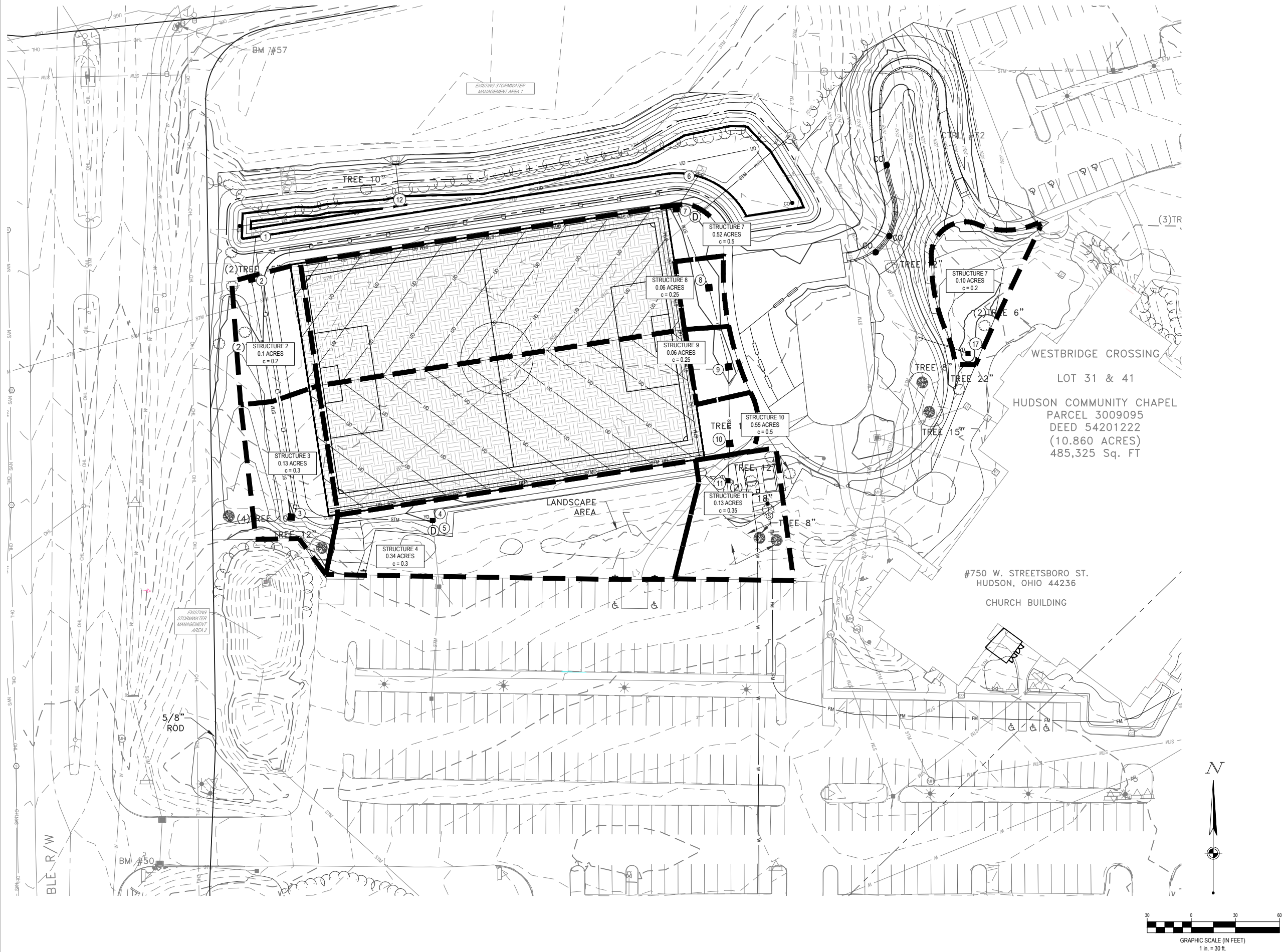
**PROPOSED DRAINAGE  
PLAN**

**PDP**

**APPENDIX E3:  
TRIBUTARY DRAINAGE AREA MAP**



C:\D:\ACCD\04\CESO\CCC - Hudson - Civil Master Plan Study\Project Files\CESO\03-CIVIL\DATA\STM\765295 - TRIBUTARY MAP.dwg - 5/14/2025 - Tommy Pilow



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2025-03-14

SOL HARRIS/DAY ARCHITECTURE

**CHRIST COMMUNITY  
CHAPEL**

750 W. STREETSBORO STREET  
HUDSON, OH 44236

Revisions / Submissions		
ID	Description	Date

© 2025 CESO, INC.	
Project Number:	765295
Scale:	AS SHOWN
Drawn By:	KAN
Checked By:	JMS
Date:	5/19/2025
Issue:	PERMIT SET

Drawing Title:  
**TRIBUTARY MAP**

**TRIB**