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31913 Cook Road | N Ridgeville | OH | 44039

Storm Water Management Report

Dr. Caruso

1465 Norton Ave
Hudson, Ohio

Summit County



Prepared by:

Greg Alber

Ohio PE 62323

Date 05-03-26



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NARRATIVE:

Project description:

The work of this project consists of proposed office building and associated parking. The project will include:

1. Proposed building and parking.
2. Storm water management system including detention using a dry extended detention pond.

The site is 1.80 acres. Total area disturbed by construction is 0.98 acres.

Existing site conditions:

The area to be disturbed by the proposed construction currently consists of a residential lot. Currently, storm water runoff from area flows overland by sheet flow and shallow concentrated flow from the southeast to the northwest.

Existing cover:

Site soils are;

Mahoning Silt Loam	Group D
Wadsworth Silt Loam	Group D

Storm Water Management:

-During construction:

See SWP3 drawings for the Storm Water Pollution Prevention Plan to be put in place during construction.

-Storm water runoff control:

A new dry detention basin will provide stormwater detention in accordance with City of Hudson standards. The basin will include extended detention of Water Quality Volume for post construction water quality treatment.

Maintenance:

Yearly maintenance to the detention pond will be necessary and will be the owner's responsibility.

Calculations:

Storm water detention calculations and routing were prepared using the computer programs Intelisolve Hydraflow Storm Sewers 2005 in addition to HydroCAD 10.00.

**Storm Drainage Design Criteria:**

Soil Types within work areas are Chili Loam which is group 'D'

-Use soil group 'D' for run-off values for design

CN values for design of run-off control (TR-55)

>75% Grass Cover Good, HSG D CN=80

Impervious CN=98

C values for design of storm sewers

Paved parking lots, roofs, driveways, etc. C=.96

Open Spaces (good condition: grass cover on 75% or more of the area) C=.36

Run-Off Control Narrative

-Capture all disturbed areas of the site and route through the dry detention pond. The pond will provide the additional storage needed for the proposed increase in impervious area. Post-construction storm water quality will be addressed in the dry extended detention pond per the requirements of the Rainwater Land and Development Manual.

Critical Storm Calculation:**Pre-Developed:**

Paved HSG D	0.178 ac	x	98	=17.444	
Gravel HSG D	0.197 ac	x	96	=18.912	
Pervious HSG D	<u>1.425 ac</u>	x	80	<u>=114.00</u>	
	1.800 ac			150.356	CN=84

Post-Developed:

Paved HSG D	0.538 ac	x	98	=52.724	
Gravel HSG D	0.197 ac	x	96	=18.912	
Pervious HSG D	<u>1.065 ac</u>	x	80	<u>=85.200</u>	
	1.800 ac			156.836	CN=87

1 year volume: (see pages D-1 and D-2)

Post – Pre / Pre = $0.134 - 0.109 / 0.109 \times 100 = 23\%$

So: 23% increase in runoff volume indicates 5 year storm



Drainage Area Calculations:

Post Construction Study Area = 0.952 acres

Post CN Calculation

Impervious HSG D	$0.490 \times 98 = 48.02$	
Pervious HSG D	$\frac{0.462}{0.952} \times 80 = \frac{36.96}{84.98}$	CN=89

Pre CN Calculation

Impervious HSG D	$0.169 \times 98 = 16.562$	
Pervious HSG D	$\frac{0.783}{0.952} \times 80 = \frac{62.64}{79.202}$	CN=83

Water Quality Volume Calculation

$$WQ_v = R_v * P * A / 12$$

$$R_v = 0.05 + 0.9(.485) = .487$$

$$WQ_v = 0.487 * 0.9 * 0.952 / 12 = 0.035 = 1515 \text{ cf}$$

$$20\% \text{ sediment storage (Micropool \& Forebay)} = 303 \text{ cf}$$



Detention Storm Calculation Summary Table:

	<u>1 yr</u>	<u>2 yr</u>	<u>5 yr</u>	<u>10 yr</u>	<u>25 yr</u>	<u>50 yr</u>	<u>100yr</u>
<u>Drainage Study Area:</u>							
Pre-Dev Q (cfs)	0.60	0.86	1.29	1.66	2.22	2.69	3.21
Allowable Q (cfs)	0.60	0.60	0.60	1.66	2.22	2.69	3.21
Q into Detention (cfs)	1.97	2.58	3.53	4.30	5.43	6.37	7.38
Q out of Detention (cfs)	0.32	0.45	0.59	0.76	1.56	2.31	3.16
Basin Peak Water Elev.	1060.99	1061.15	1061.38	1061.56	1061.75	1061.88	1062.01



Summit County, Ohio (OH153)				
Summit County, Ohio (OH153)				
Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI	
CnB	Chili loam, 2 to 6 percent slopes	2.3	61.8%	
Pg	Pits, gravel	1.4	38.2%	
Totals for Area of Interest		3.8	100.0%	

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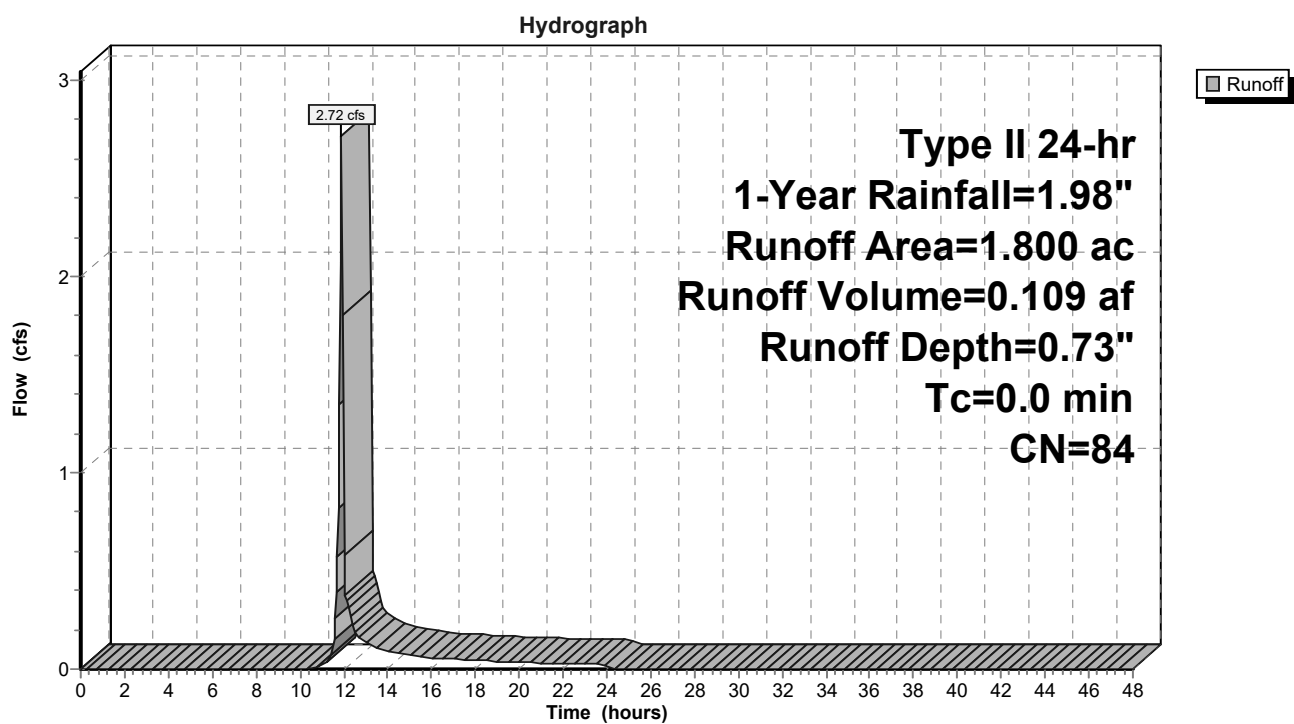
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Subcatchment 1S: Critical Storm Existing



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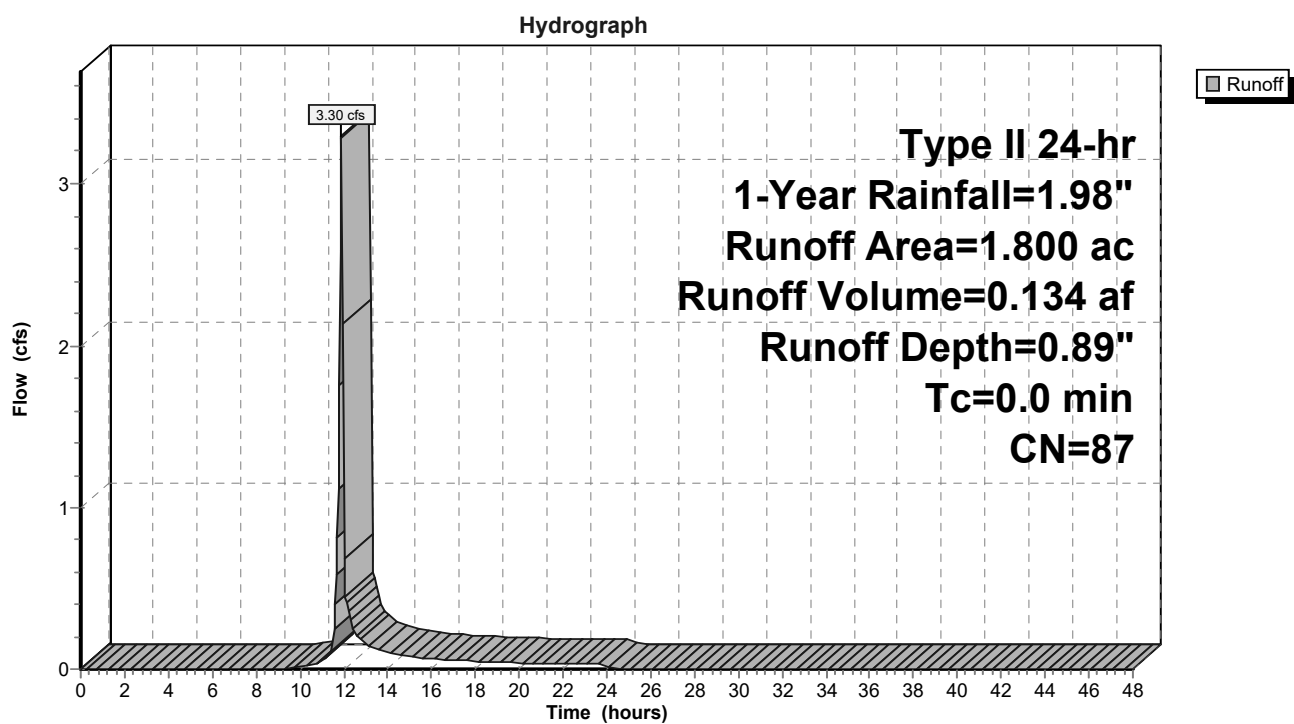
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Subcatchment 2S: Critical Storm Proposed



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Stage-Area-Storage for Pond 8P: Detention Basin work

Elevation (feet)	Surface (sq-ft)	Storage (cubic-feet)	Elevation (feet)	Surface (sq-ft)	Storage (cubic-feet)
1,060.00	2,423	0	1,062.60	5,121	9,717
1,060.05	2,472	122	1,062.65	5,177	9,974
1,060.10	2,521	247	1,062.70	5,232	10,234
1,060.15	2,570	374	1,062.75	5,288	10,497
1,060.20	2,619	504	1,062.80	5,343	10,763
1,060.25	2,668	636	1,062.85	5,399	11,032
1,060.30	2,717	771	1,062.90	5,454	11,303
1,060.35	2,766	908	1,062.95	5,510	11,577
1,060.40	2,815	1,048	1,063.00	5,565	11,854
1,060.45	2,864	1,190			
1,060.50	2,914	1,334			
1,060.55	2,963	1,481			
1,060.60	3,012	1,630			
1,060.65	3,061	1,782			
1,060.70	3,110	1,936			
1,060.75	3,159	2,093			
1,060.80	3,208	2,252			
1,060.85	3,257	2,414			
1,060.90	3,306	2,578			
1,060.95	3,355	2,745			
1,061.00	3,404	2,914			
1,061.05	3,457	3,085			
1,061.10	3,509	3,259			
1,061.15	3,562	3,436			
1,061.20	3,614	3,615			
1,061.25	3,667	3,797			
1,061.30	3,720	3,982			
1,061.35	3,772	4,169			
1,061.40	3,825	4,359			
1,061.45	3,877	4,552			
1,061.50	3,930	4,747			
1,061.55	3,983	4,945			
1,061.60	4,035	5,145			
1,061.65	4,088	5,348			
1,061.70	4,140	5,554			
1,061.75	4,193	5,762			
1,061.80	4,246	5,973			
1,061.85	4,298	6,187			
1,061.90	4,351	6,403			
1,061.95	4,403	6,622			
1,062.00	4,456	6,844			
1,062.05	4,511	7,068			
1,062.10	4,567	7,295			
1,062.15	4,622	7,524			
1,062.20	4,678	7,757			
1,062.25	4,733	7,992			
1,062.30	4,789	8,230			
1,062.35	4,844	8,471			
1,062.40	4,900	8,715			
1,062.45	4,955	8,961			
1,062.50	5,011	9,210			
1,062.55	5,066	9,462			

Required WQv=1515 cf



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Water Quality Calculator

Project Name	Dr Caruso
Project Date	4.30.26
Calculated by:	CLH

Parameters		
Drainage Area	0.95	Acres
Precipitation (P)	0.90	Inches
Runoff Coefficient (Rv)	0.487	none
Drawdown Time (T)	48.00	Hours
WQv depth (H)	2.26	Feet

Runoff Coefficient	
Rv = 0.05 + 0.9i	
i = impervious ratio =	0.485
Rv =	0.49

Calculated Water Quality Volume	
WQv = Rv * P * A / 12	
WQv =	0.0348 acre-feet 1515 cubic feet

Provided Water Quality Volume	
(captured under first window/rim)	
WQv =	1515 cubic feet

Calculated Sediment Storage Volume	
SSV = 20% * WQv	
SSV =	303 cubic feet

Provided Minimum Permanent Pool Volume	
(volume below the WQv orifice)	
PPV =	cubic feet

Calculated Minimum Permanent Pool Volume	
PPV = WQv + SSV	
PPV =	1818 cubic feet

Calculated Water Quality Velocity	
Vo = Cv * (2gH)^0.5	Cv = 0.61
Vo = 0.61 * (2 * 32.2 * H)^0.5	
Vo =	7.36 ft/s

Calculated Water Quality Flow	
Q = WQv / time	
Q =	0.009 cubic feet/sec

Required Orifice Size	
Q = Cd * Ao * (2gH)^0.5	Cd = 0.61

Provided Drawdown Time	
Q = Vo * Aa = Provided WQv / Time	
Vo =	7.36 ft/s
Aa = pi * Ra^2 =	0.13 square inch
	0.0009 square feet

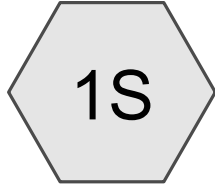
Ao = Q / (Cd * ((2gH)^0.5))	
Ao =	0.0012 square feet 0.17 square inch

Time = Provided WQv / (Vo * Aa)	
Time =	235906 seconds 65.53 hours

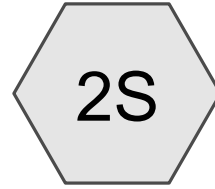
Calculated Orifice Diameter	
Ao = pi * R^2	
R = (Ao * pi)^0.5	
R =	0.2337 inch
D =	0.467 inch

Provided WQ Volume Discharged in 1/3 drawdown Time	
Volume = Q * 1/3 drawdown time	
Volume =	505 cubic feet
which is greater than 1/2 total provided WQv:	
1/2 WQv =	758 cubic feet

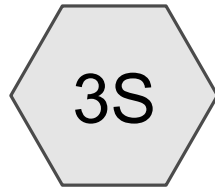
Provided Orifice Diameter	
Da =	0.40 inch
Ra =	0.20 inch



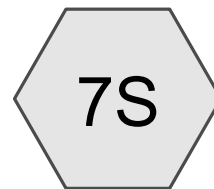
Critical Storm Existing



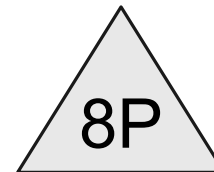
Critical Storm Proposed



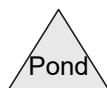
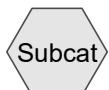
Existing



Developed



Detention Basin



Routing Diagram for Caruso 4.30.26

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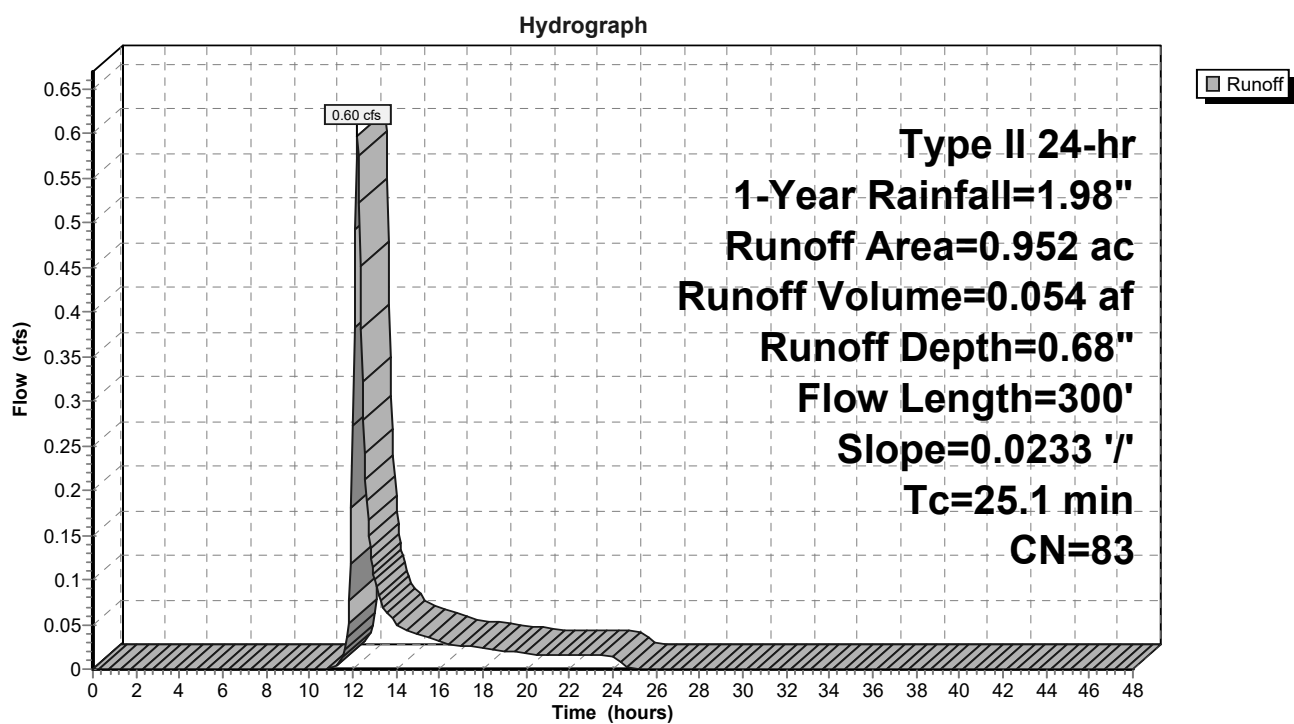
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Subcatchment 3S: Existing



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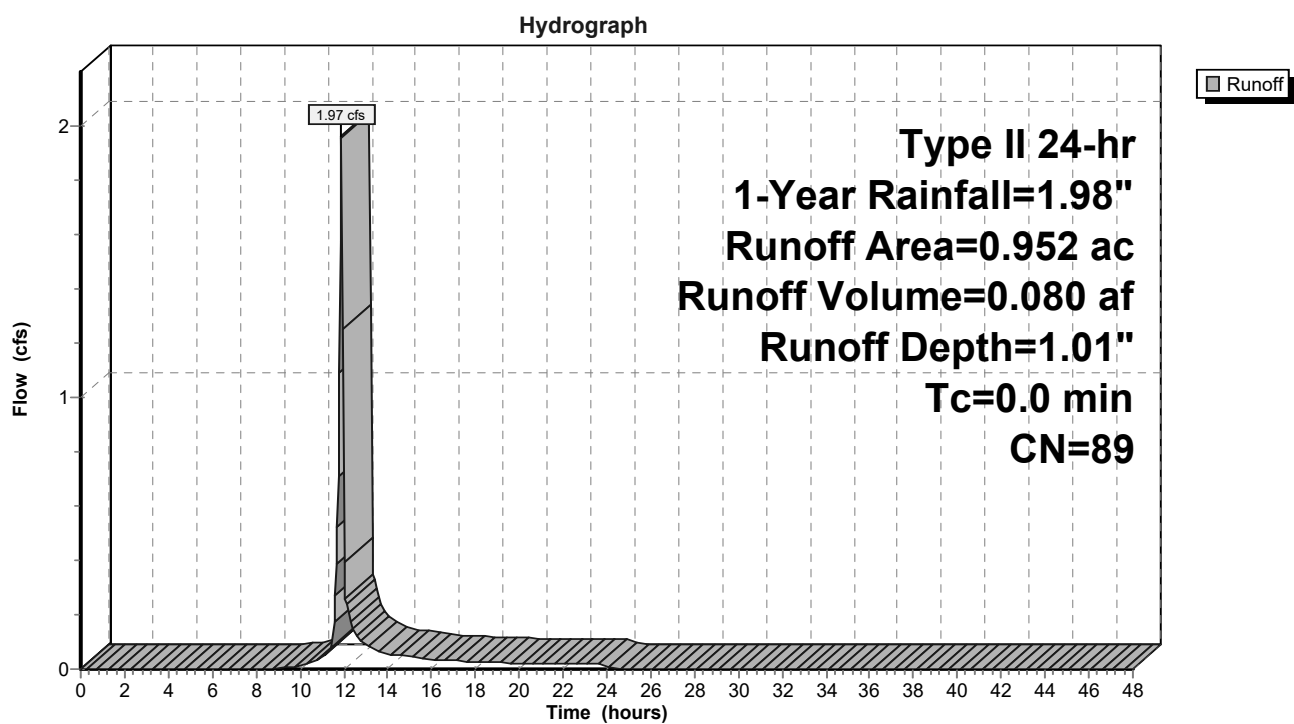
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Subcatchment 7S: Developed



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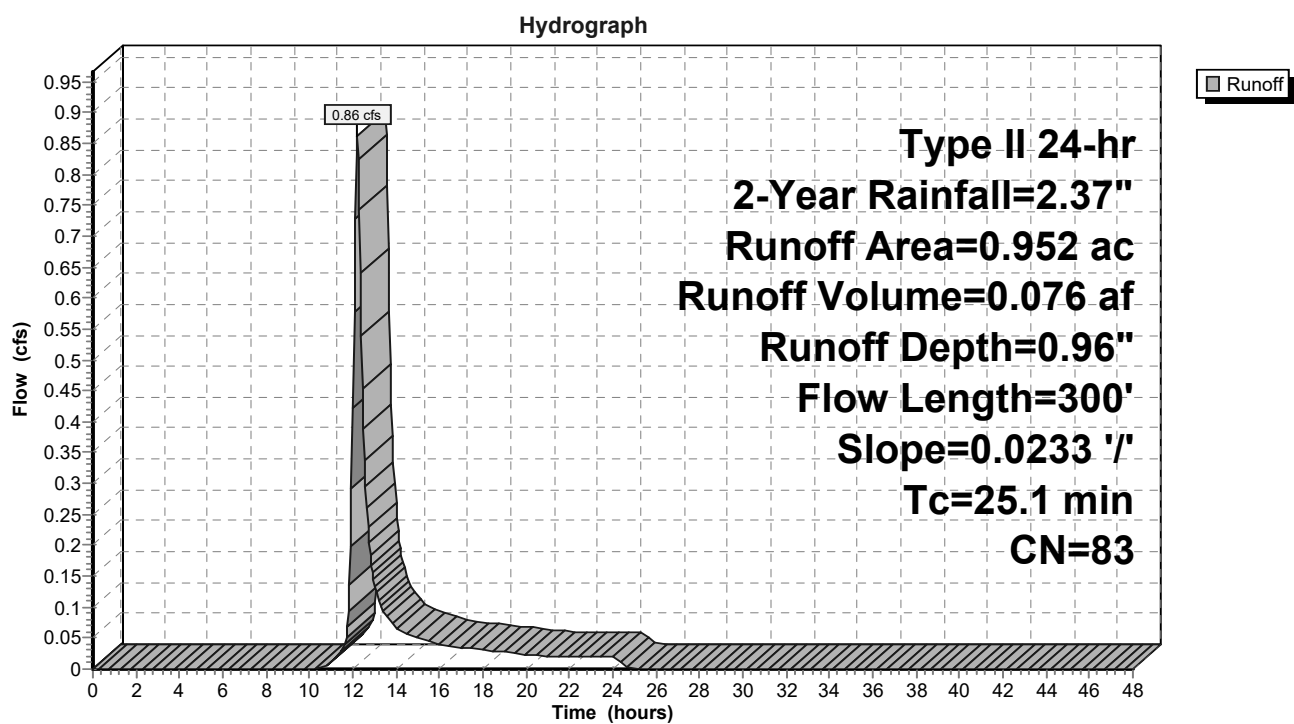
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Subcatchment 3S: Existing



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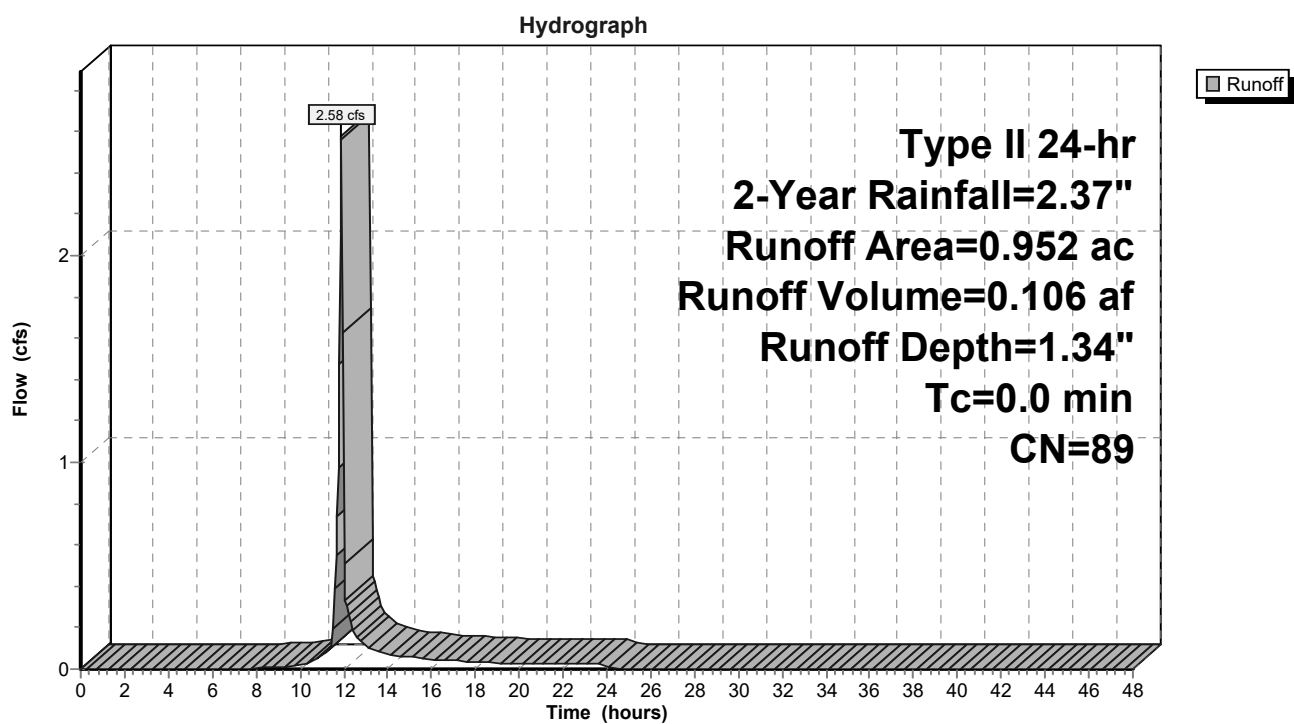
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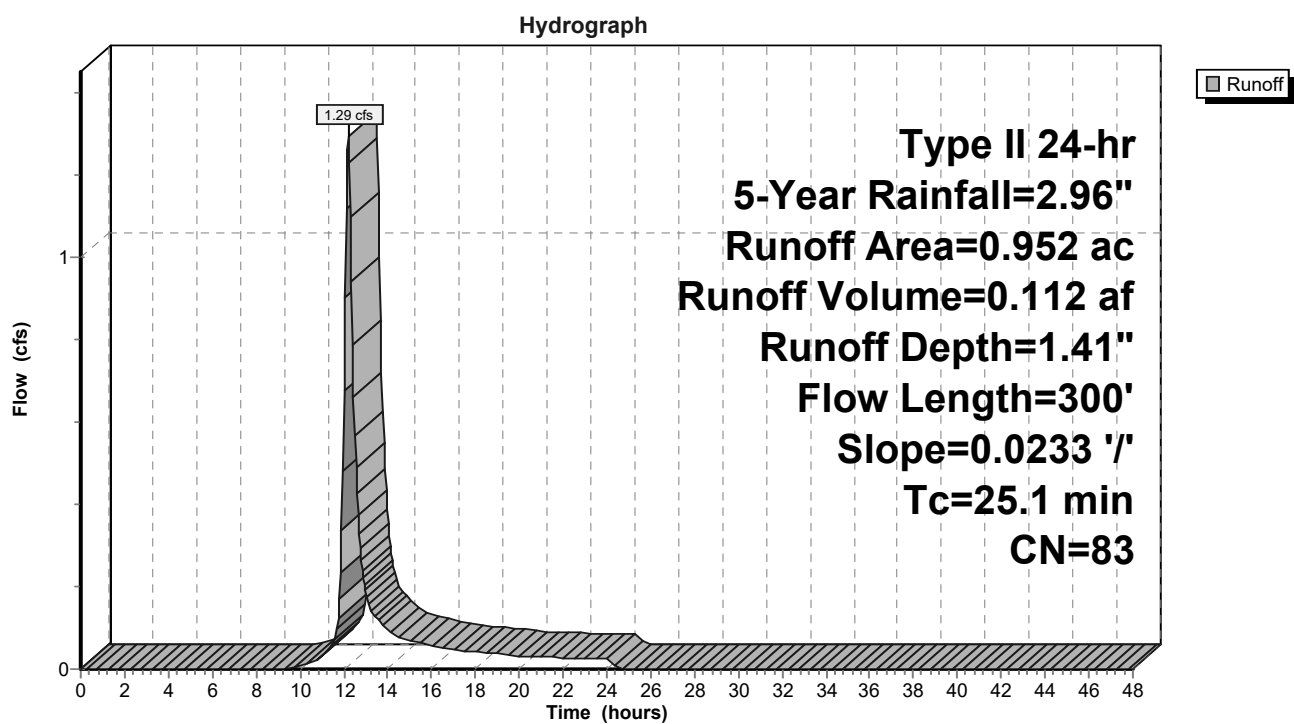
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Subcatchment 3S: Existing



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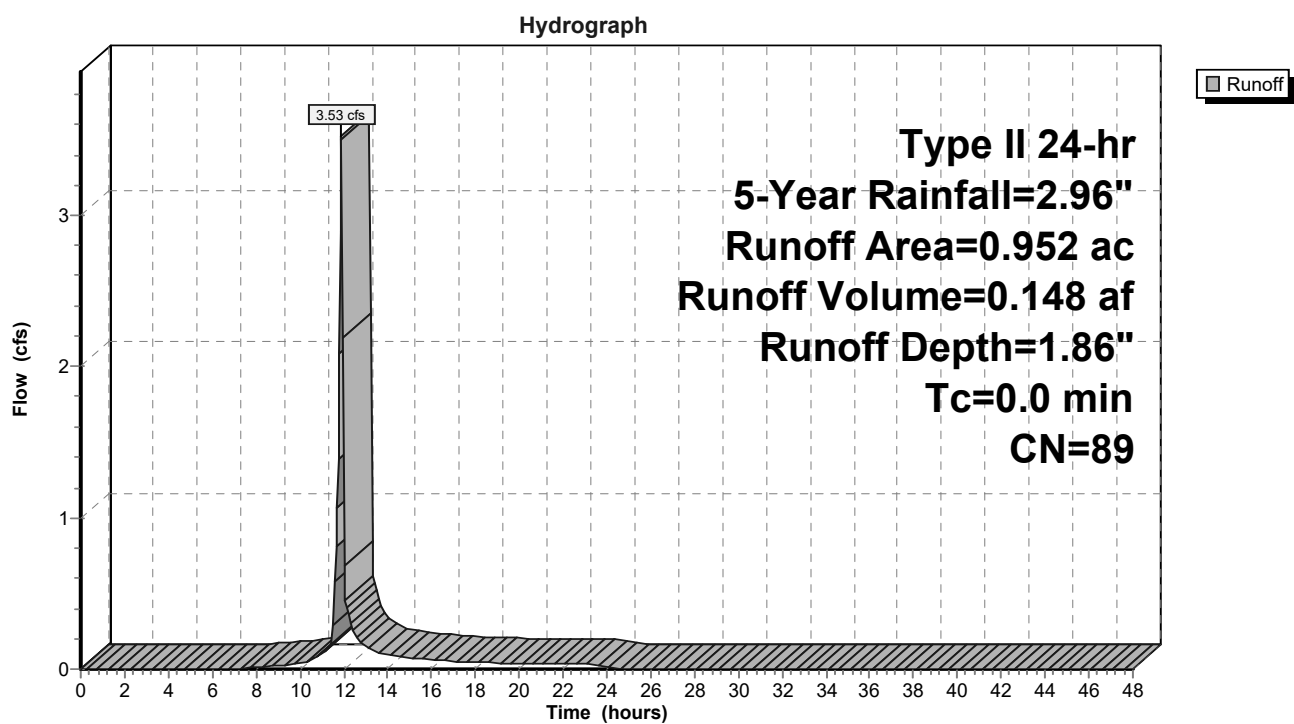
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Subcatchment 7S: Developed



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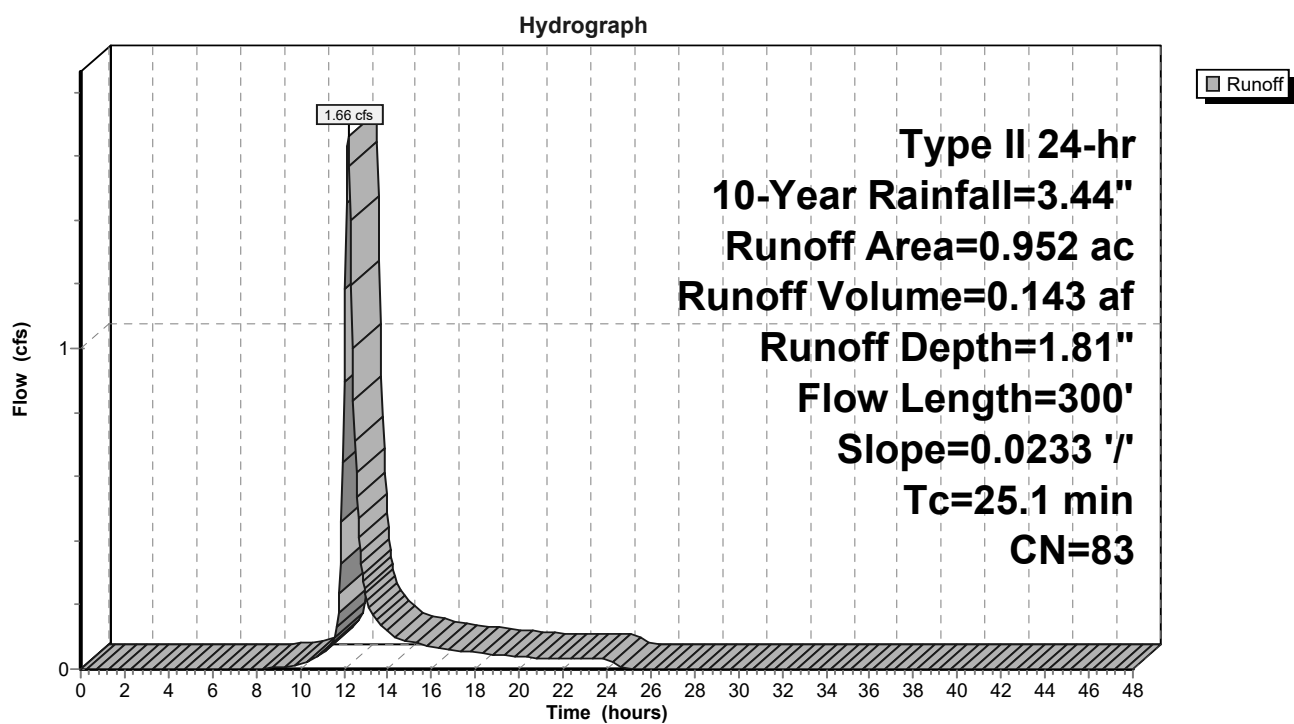
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Subcatchment 3S: Existing



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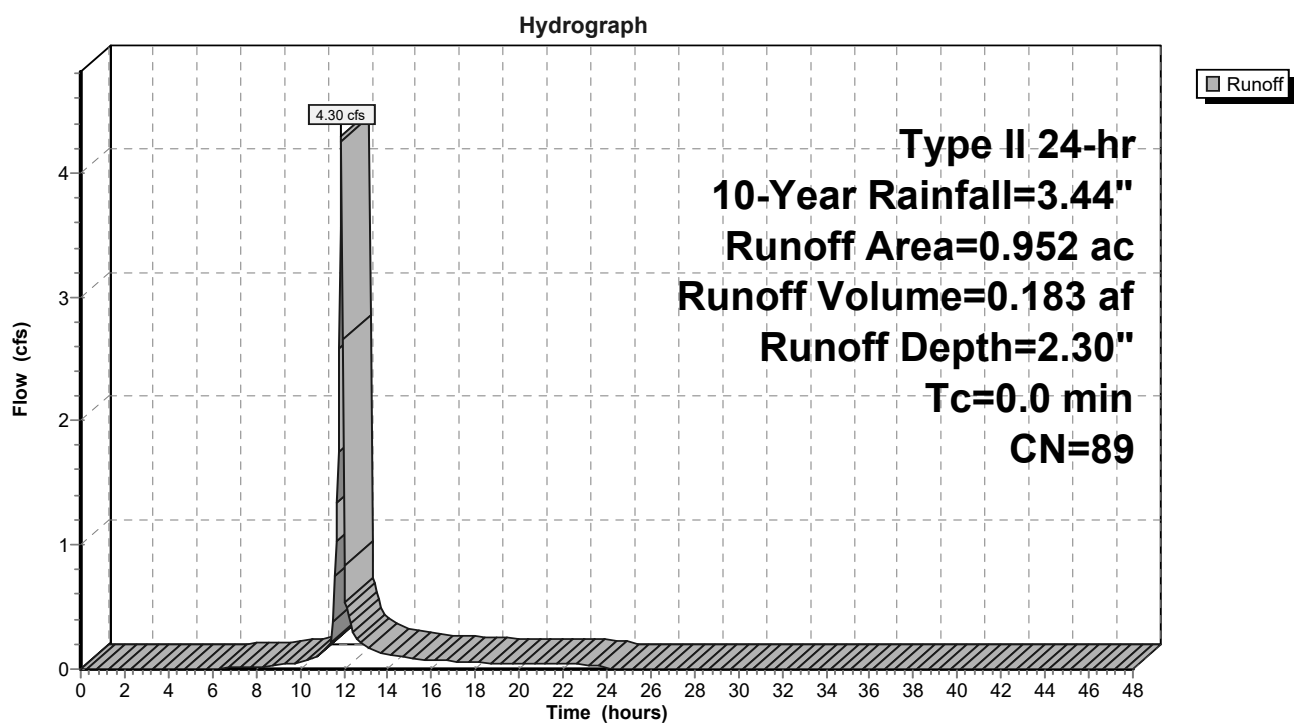
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Type II 24-hr 10-Year Rainfall=3.44"

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Subcatchment 7S: Developed



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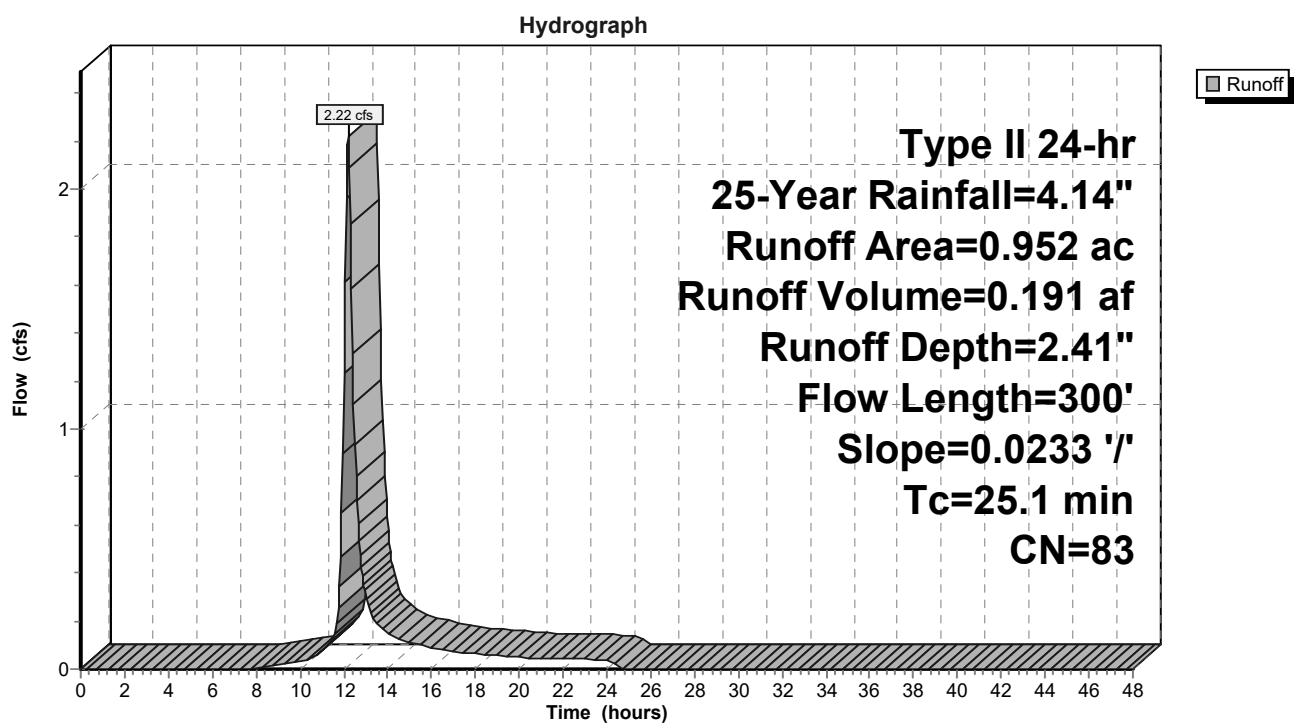
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Subcatchment 3S: Existing



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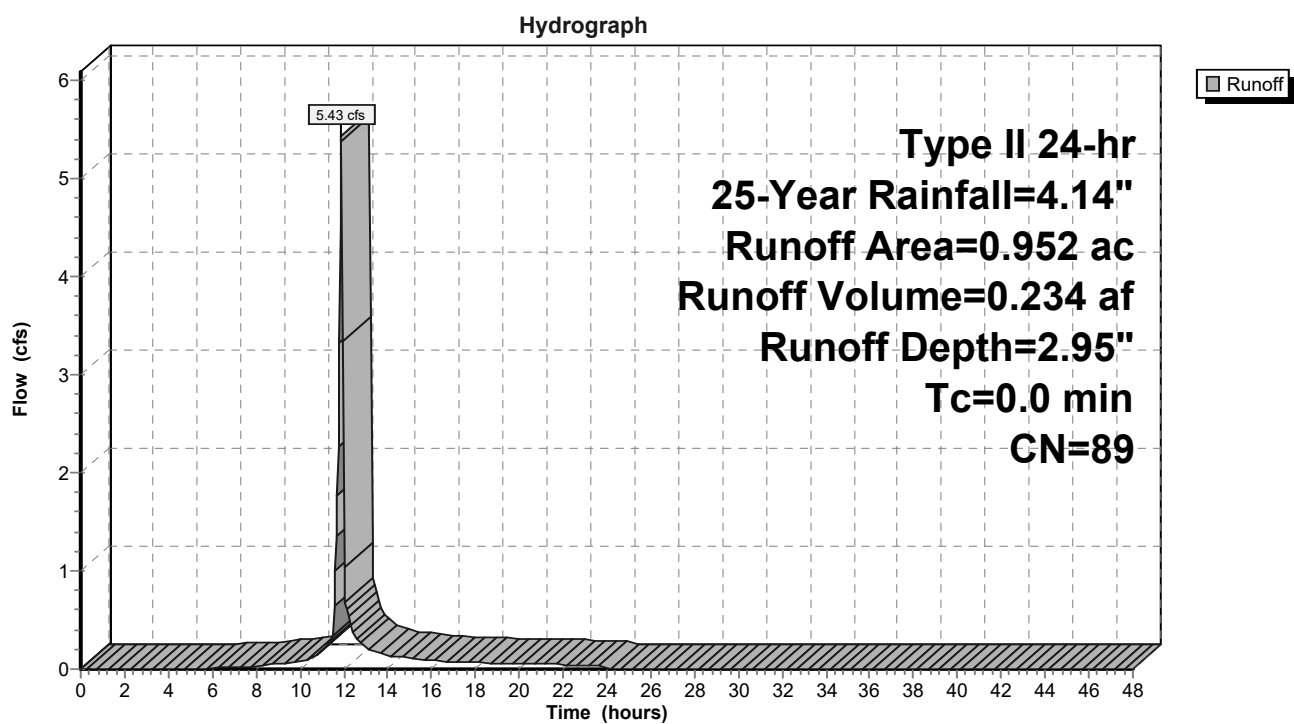
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Type II 24-hr 25-Year Rainfall=4.14"

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Subcatchment 7S: Developed



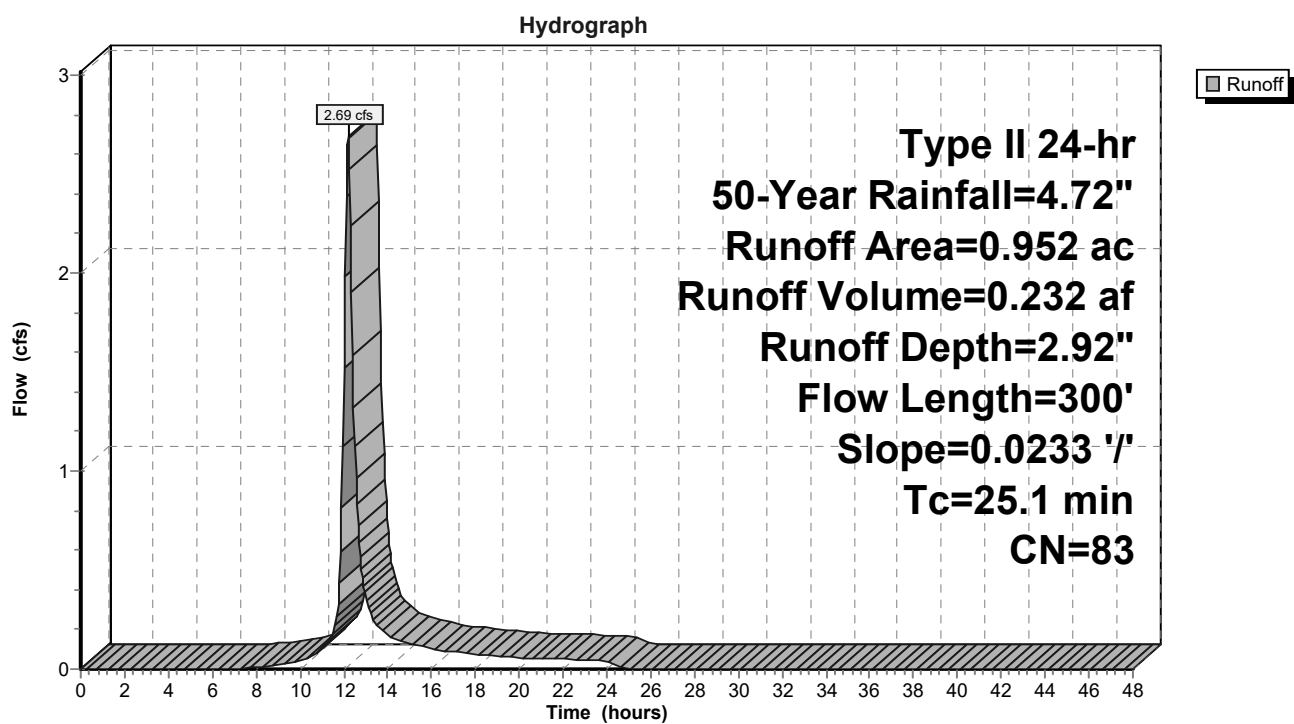
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Type II 24-hr 50-Year Rainfall=4.72"

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Subcatchment 3S: Existing

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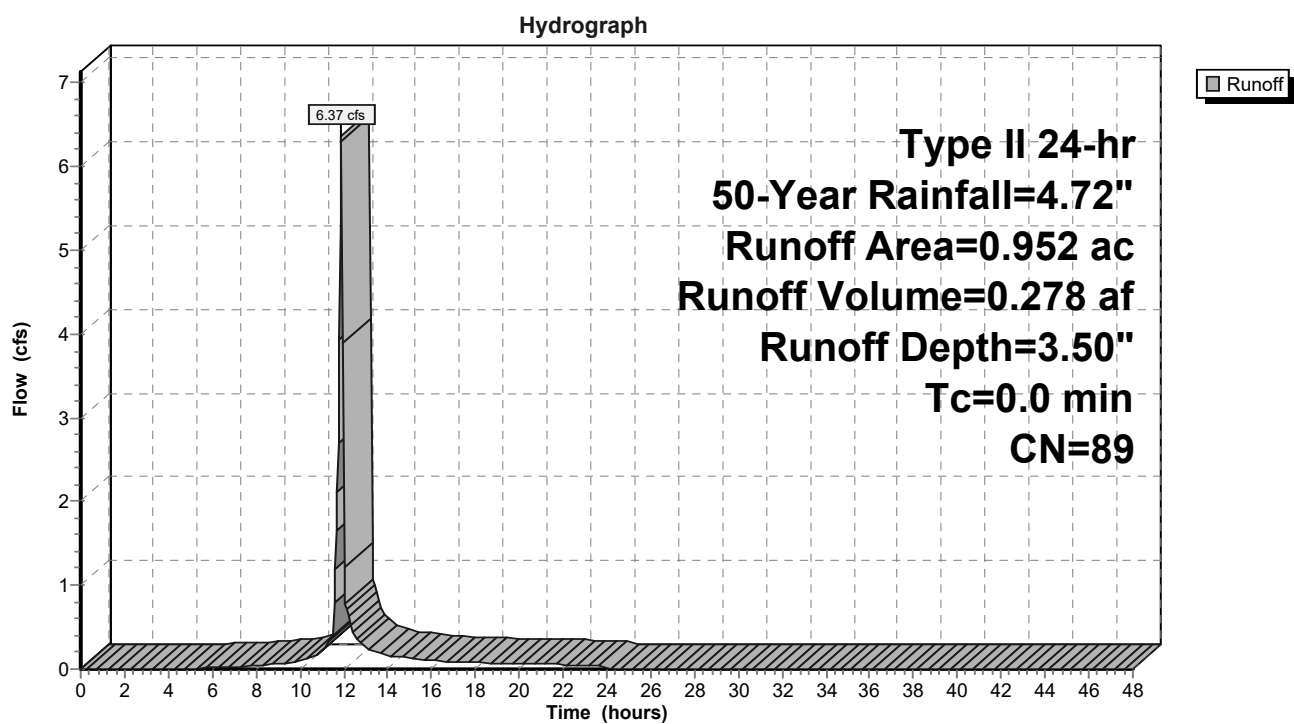
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Type II 24-hr 50-Year Rainfall=4.72"

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Subcatchment 7S: Developed



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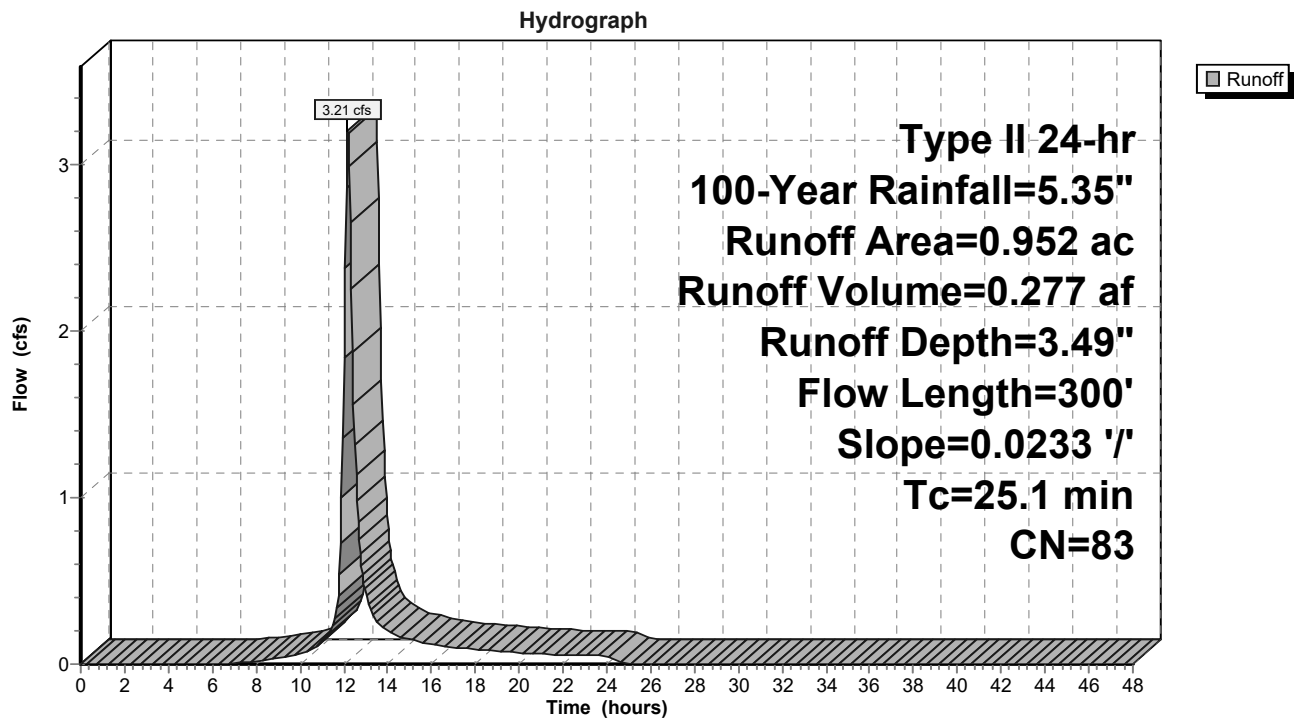
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Type II 24-hr 100-Year Rainfall=5.35"

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Subcatchment 3S: Existing



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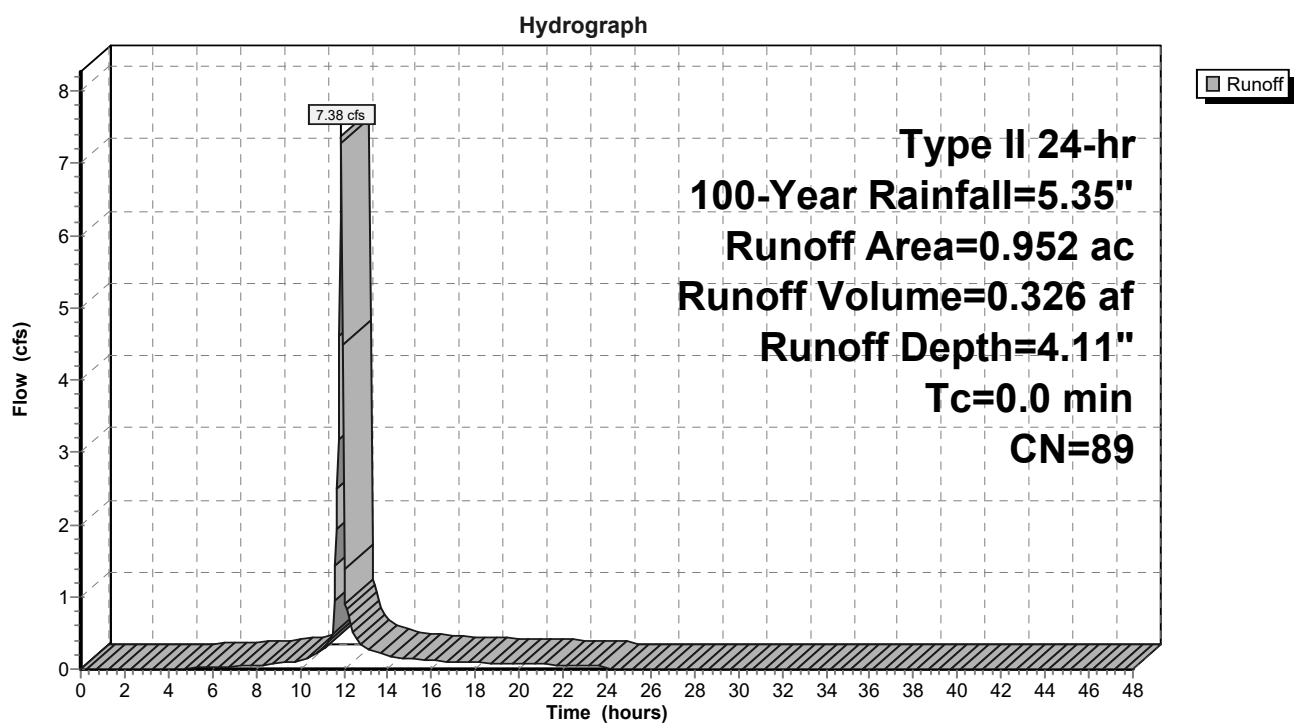
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Type II 24-hr 100-Year Rainfall=5.35"

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Subcatchment 7S: Developed



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Type II 24-hr 1-Year Rainfall=1.98"

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Summary for Pond 8P: Detention Basin

Inflow Area = 0.952 ac, 51.47% Impervious, Inflow Depth = 1.01" for 1-Year event
 Inflow = 1.97 cfs @ 11.89 hrs, Volume= 0.080 af
 Outflow = 0.32 cfs @ 12.03 hrs, Volume= 0.073 af, Atten= 84%, Lag= 8.3 min
 Primary = 0.32 cfs @ 12.03 hrs, Volume= 0.073 af

Routing by Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs
 Starting Elev= 1,060.50' Surf.Area= 2,914 sf Storage= 1,334 cf
 Peak Elev= 1,060.99' @ 12.03 hrs Surf.Area= 3,395 sf Storage= 2,882 cf (1,548 cf above start)

Plug-Flow detention time= 409.9 min calculated for 0.043 af (53% of inflow)
 Center-of-Mass det. time= 138.9 min (962.0 - 823.0)

Volume	Invert	Avail.Storage	Storage Description
#1	1,060.00'	11,854 cf	Custom Stage Data (Prismatic) Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
1,060.00	2,423	0	0
1,061.00	3,404	2,914	2,914
1,062.00	4,456	3,930	6,844
1,063.00	5,565	5,011	11,854

Device	Routing	Invert	Outlet Devices
#1	Primary	1,057.94'	12.0" Round Culvert L= 80.0' CPP, mitered to conform to fill, Ke= 0.700 Inlet / Outlet Invert= 1,057.94' / 1,057.14' S= 0.0100 ' / ' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf
#2	Device 1	1,060.60'	5.5" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#3	Device 1	1,061.50'	24.0" W x 5.5" H Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads

Primary OutFlow Max=0.32 cfs @ 12.03 hrs HW=1,060.99' (Free Discharge)

1=Culvert (Passes 0.32 cfs of 5.18 cfs potential flow)
 2=Orifice/Grate (Orifice Controls 0.32 cfs @ 2.13 fps)
 3=Orifice/Grate (Controls 0.00 cfs)

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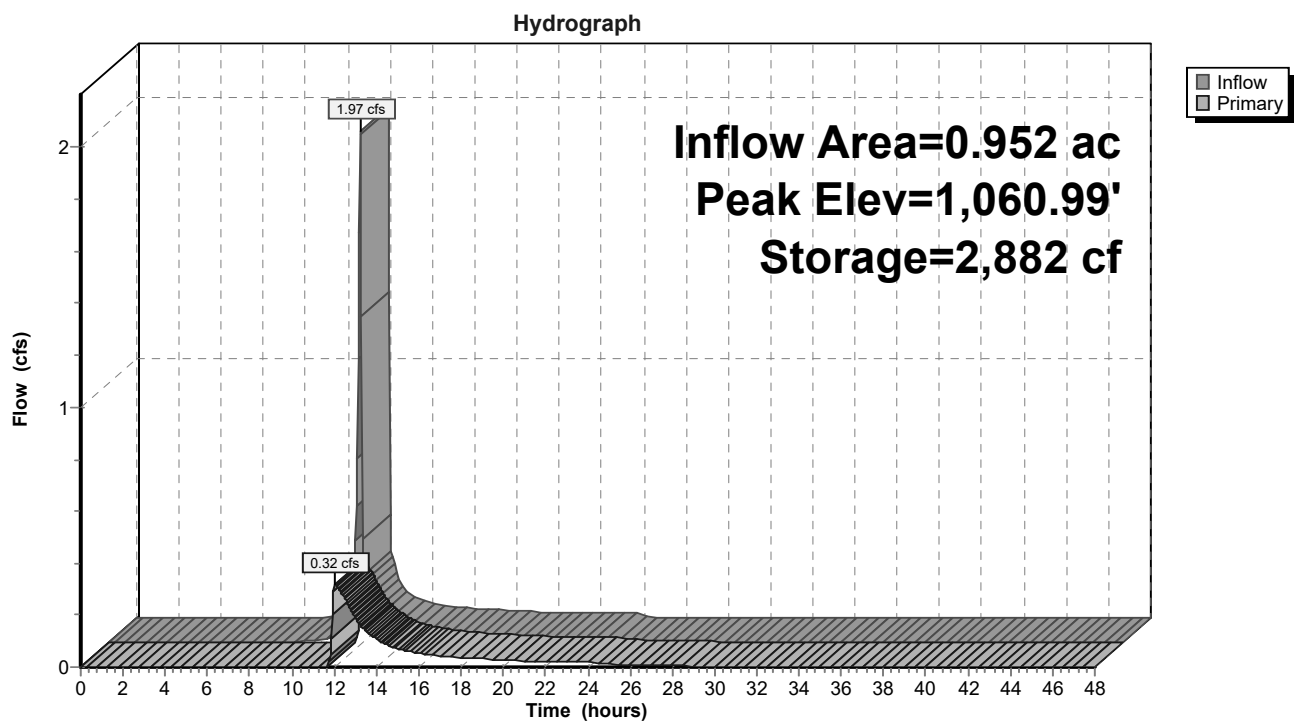
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Printed 5/3/2026

Pond 8P: Detention Basin



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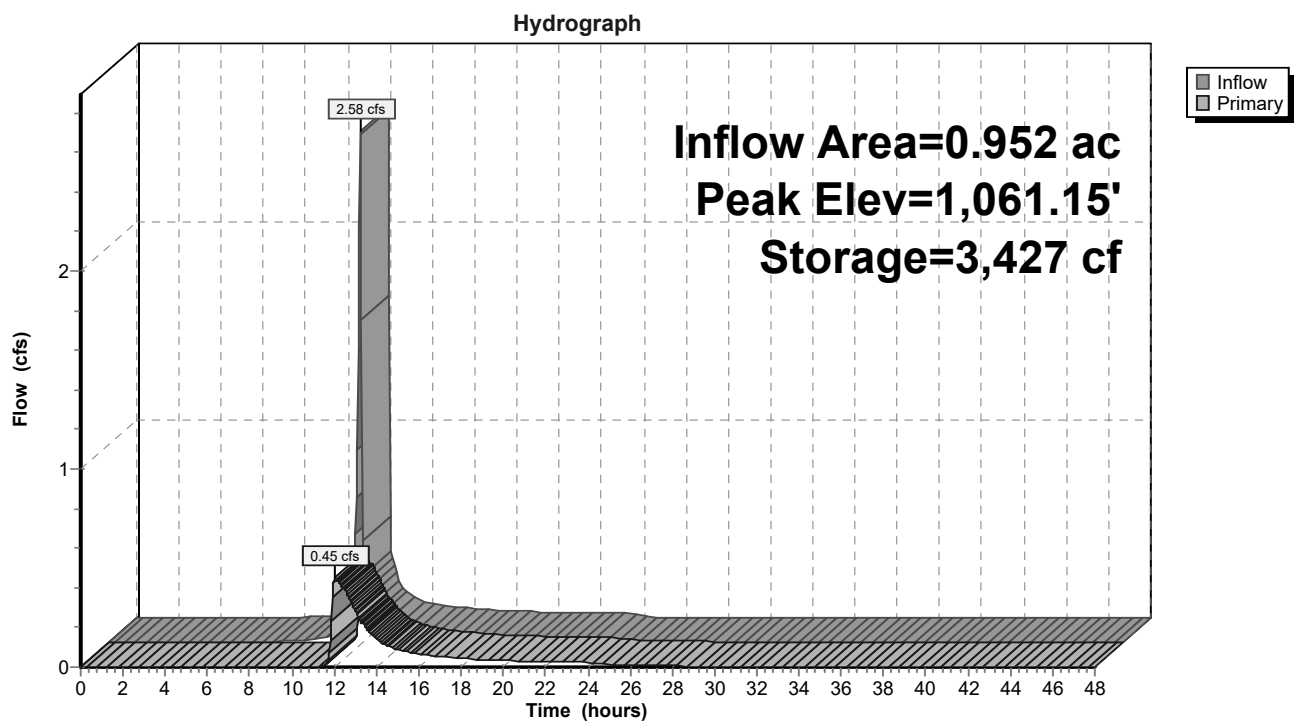
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Type II 24-hr 2-Year Rainfall=2.37"

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Pond 8P: Detention Basin



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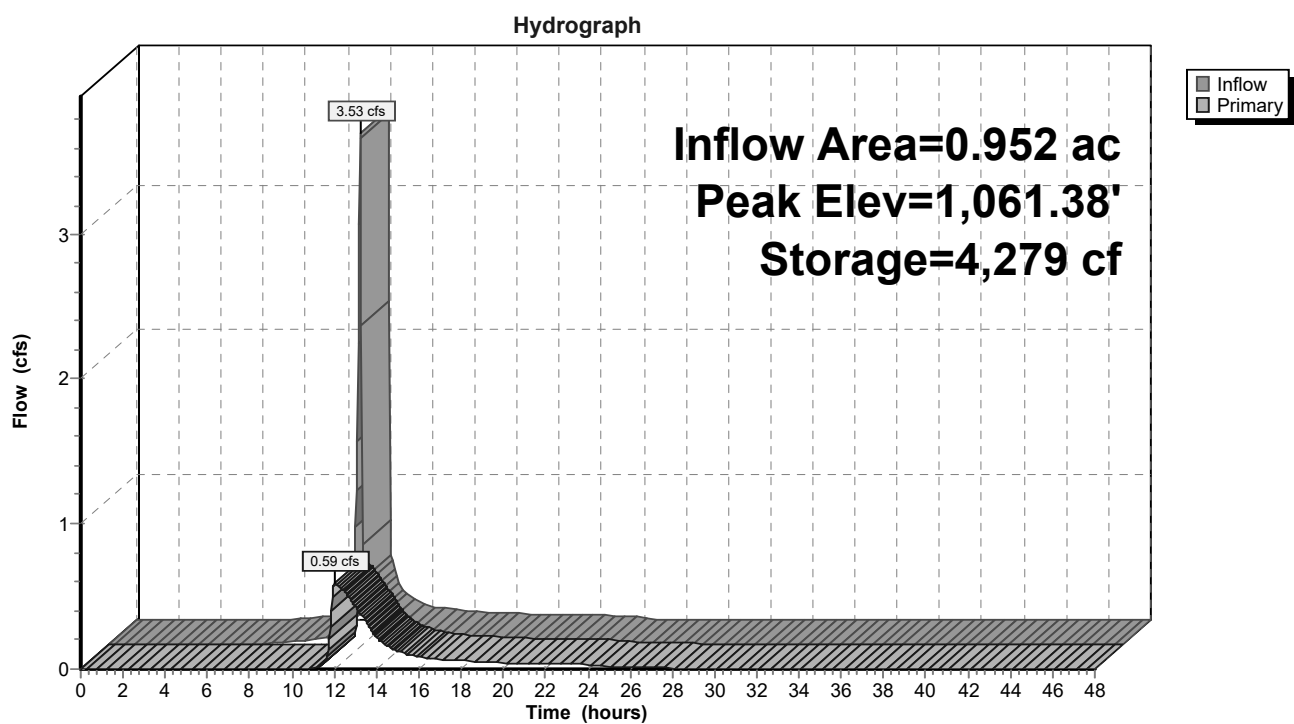
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Type II 24-hr 5-Year Rainfall=2.96"

Printed 5/3/2026

Pond 8P: Detention Basin



Caruso 4.30.26

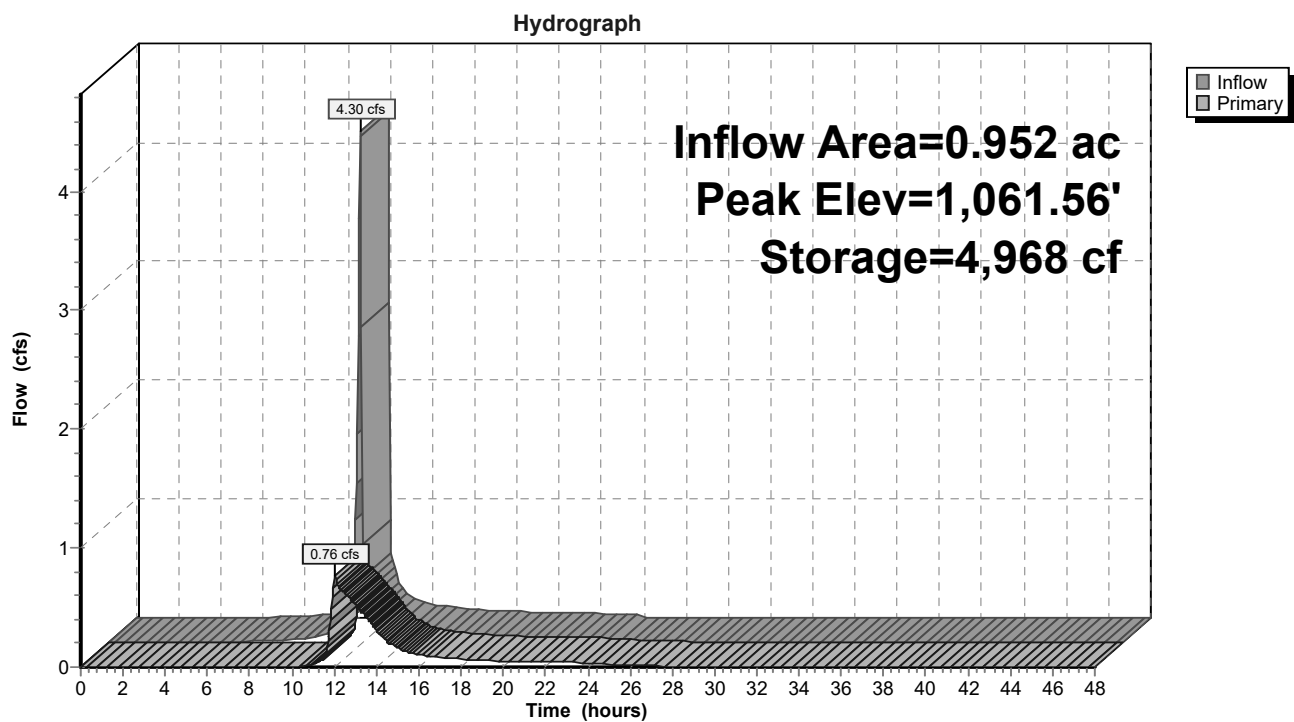
Prepared by Alber & Rice, Inc

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Type II 24-hr 10-Year Rainfall=3.44"

Printed 5/3/2026

Pond 8P: Detention Basin



Caruso 4.30.26

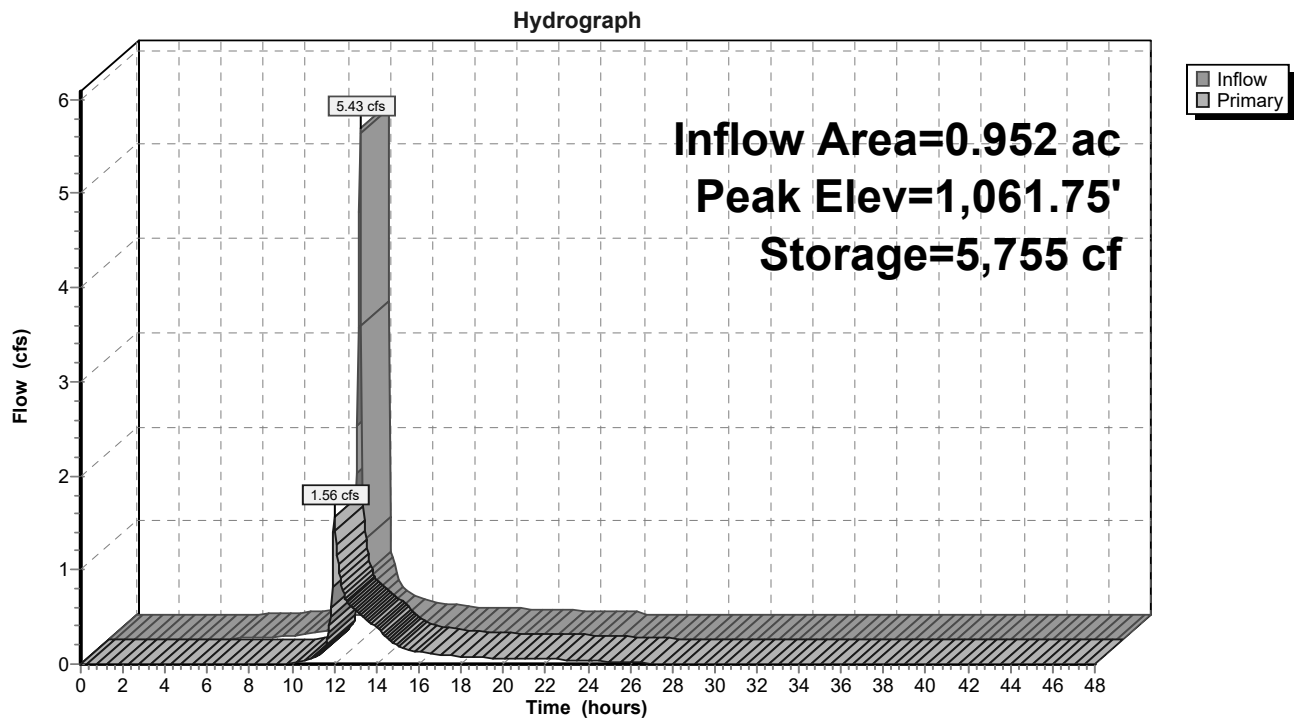
Prepared by Alber & Rice, Inc

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Type II 24-hr 25-Year Rainfall=4.14"

Printed 5/3/2026

Pond 8P: Detention Basin



Caruso 4.30.26

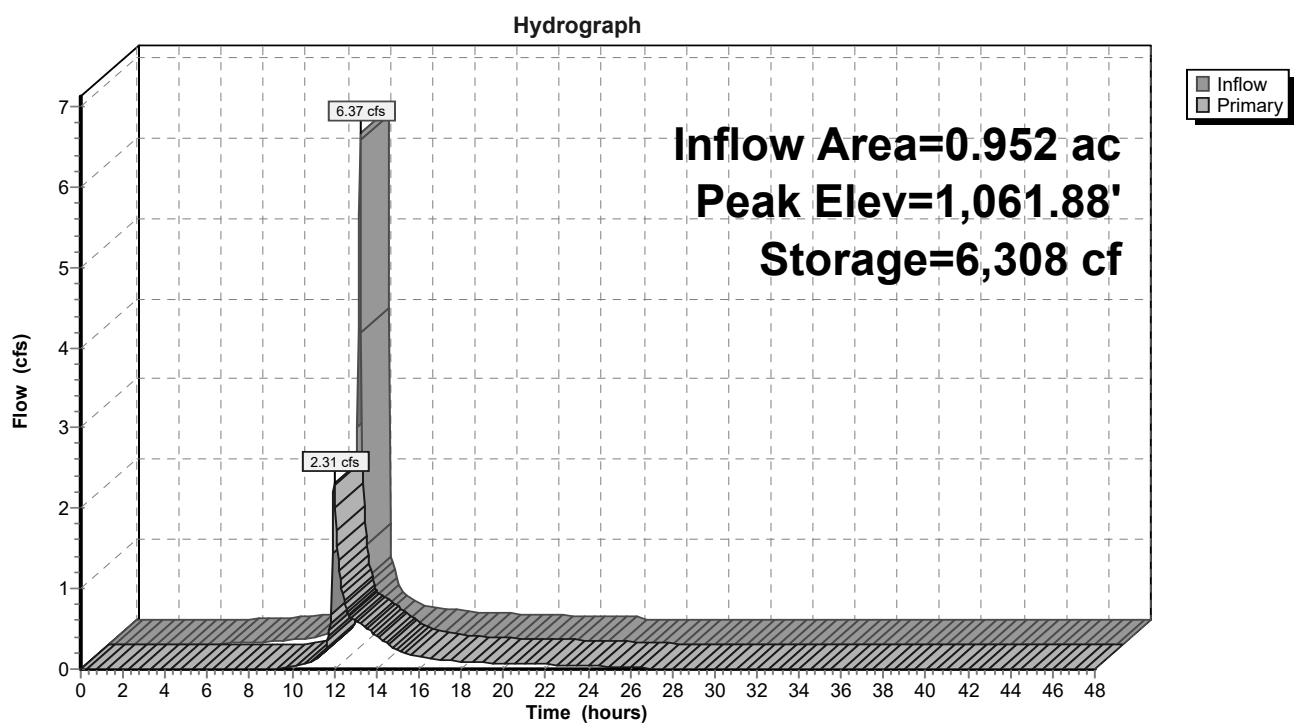
Prepared by Alber & Rice, Inc

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Type II 24-hr 50-Year Rainfall=4.72"

Printed 5/3/2026

Pond 8P: Detention Basin



Caruso 4.30.26

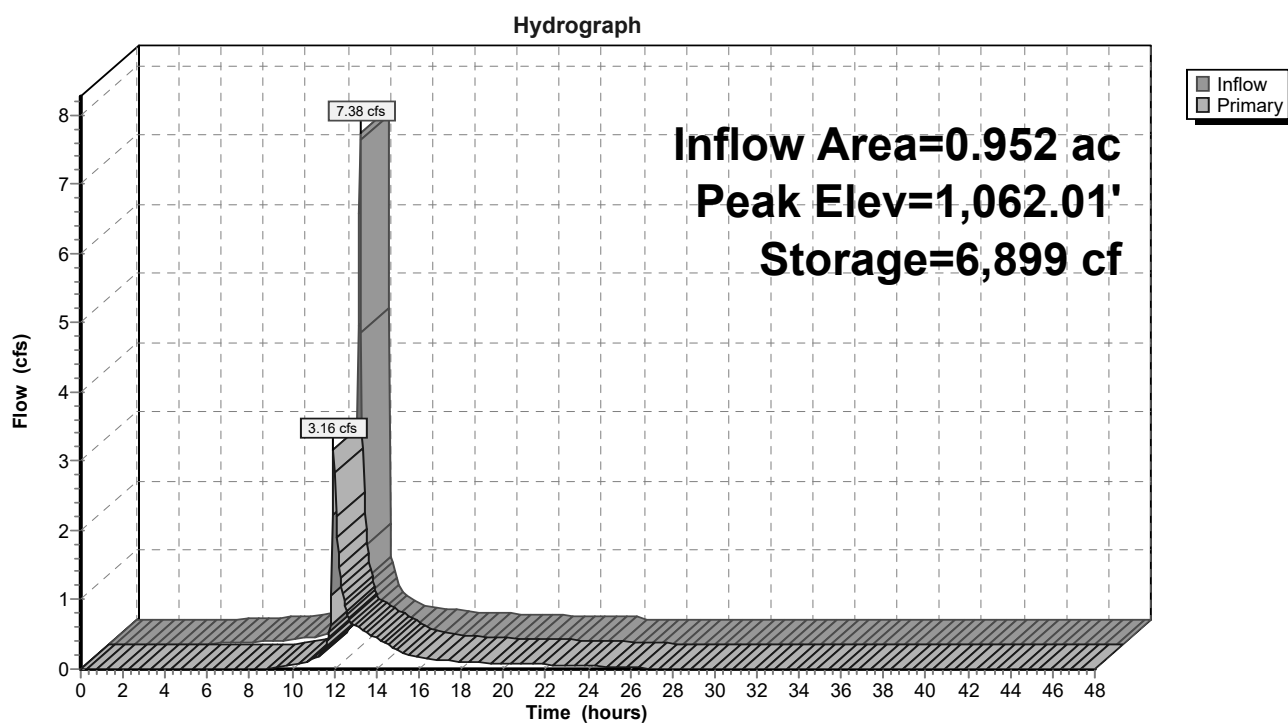
Prepared by Alber & Rice, Inc

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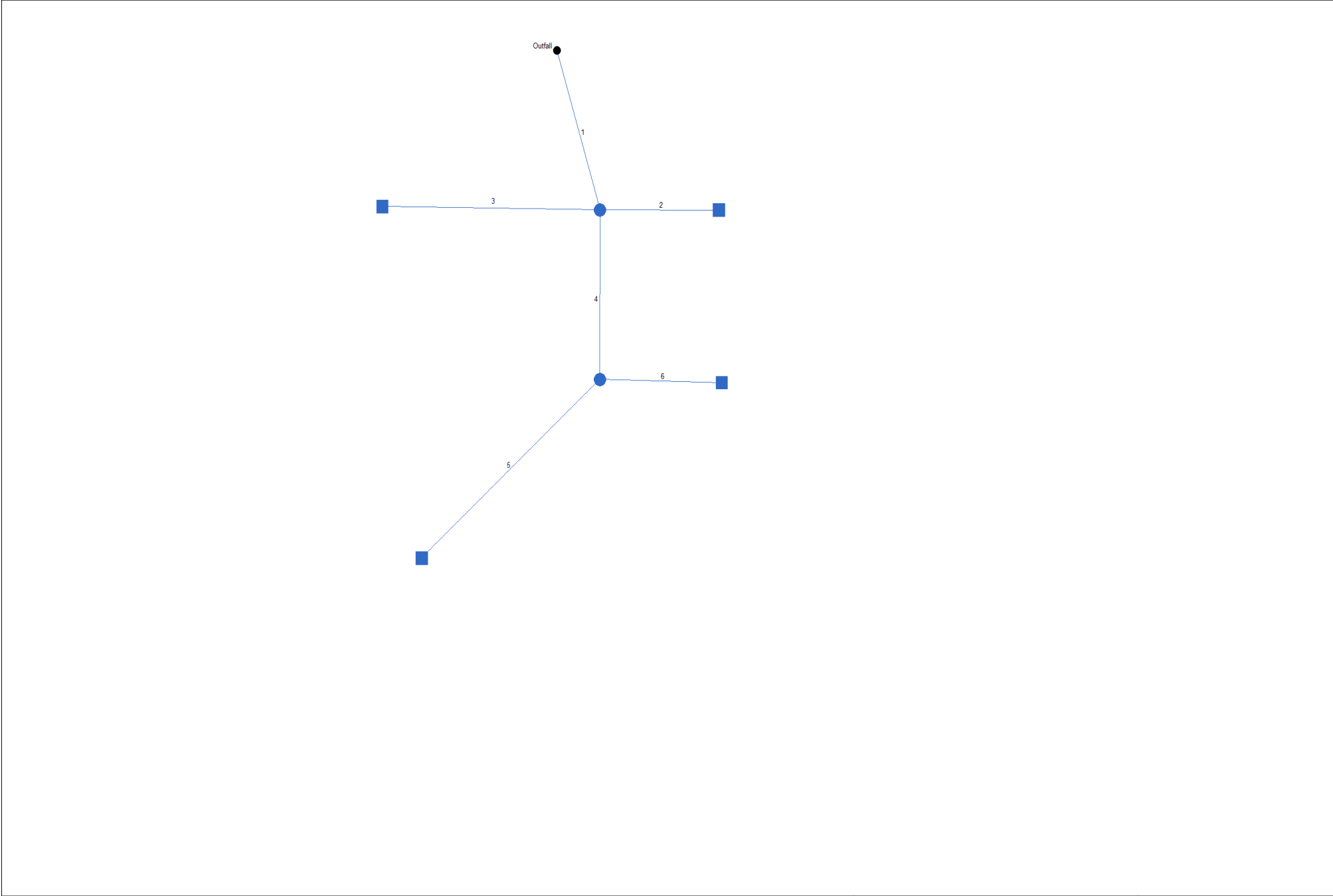
Type II 24-hr 100-Year Rainfall=5.35"

Printed 5/3/2026

Pond 8P: Detention Basin



Hydraflow Plan View



Storm Sewer Tabulation

Station		Len	Drng Area		Rnoff coeff	Area x C		Tc		Rain (l)	Total flow	Cap full	Vel	Pipe		Invert Elev		HGL Elev		Grnd / Rim Elev		Line ID
Line	To Line		Incr	Total		Incr	Total	Inlet	Syst					Size	Slope	Up	Dn	Up	Dn	Up	Dn	
		(ft)	(ac)	(ac)	(C)			(min)	(min)	(in/hr)	(cfs)	(cfs)	(ft/s)	(in)	(%)	(ft)	(ft)	(ft)	(ft)	(ft)	(ft)	
1	End	52.0	0.07	0.61	0.94	0.07	0.48	10.0	21.3	3.5	1.65	2.52	3.42	12	0.50	1061.26	1061.00	1061.85	1061.59	1064.30	1062.00	
2	1	41.0	0.03	0.03	0.36	0.01	0.01	10.0	10.0	4.9	0.05	2.55	0.09	12	0.51	1061.47	1061.26	1062.03	1062.03	1065.00	1064.30	
3	1	75.0	0.16	0.16	0.93	0.15	0.15	10.0	10.0	4.9	0.73	2.53	1.64	12	0.51	1061.64	1061.26	1062.09	1062.03	1064.00	1064.30	
4	1	53.0	0.16	0.35	0.90	0.14	0.25	10.0	14.5	4.2	1.05	2.54	2.00	12	0.51	1061.53	1061.26	1062.08	1062.03	1064.00	1064.30	
5	4	83.0	0.12	0.12	0.69	0.08	0.08	10.0	10.0	4.9	0.42	2.53	1.19	12	0.51	1061.95	1061.53	1062.30	1062.25	1064.50	1064.00	
6	4	42.0	0.07	0.07	0.36	0.02	0.02	10.0	10.0	4.9	0.12	2.52	0.26	12	0.50	1061.74	1061.53	1062.24	1062.24	1065.00	1064.00	
Caruso Dental																Number of lines: 6				Run Date: 05-03-2026		
NOTES: Intensity = 139.19 / (Inlet time + 17.50) ^ 1.01; Return period = 10 Yrs.																						

Storm Sewer Tabulation

Station		Len	Drng Area		Rnoff coeff	Area x C		Tc		Rain (l)	Total flow	Cap full	Vel	Pipe		Invert Elev		HGL Elev		Grnd / Rim Elev		Line ID
Line	To Line		Incr	Total		Incr	Total	Inlet	Syst					Size	Slope	Up	Dn	Up	Dn	Up	Dn	
		(ft)	(ac)	(ac)	(C)			(min)	(min)	(in/hr)	(cfs)	(cfs)	(ft/s)	(in)	(%)	(ft)	(ft)	(ft)	(ft)	(ft)	(ft)	
1	End	52.0	0.07	0.61	0.94	0.07	0.48	10.0	19.0	4.5	2.16	2.52	3.61	12	0.50	1061.26	1061.00	1061.97	1061.71	1064.30	1062.00	
2	1	41.0	0.03	0.03	0.36	0.01	0.01	10.0	10.0	6.1	0.06	2.55	0.09	12	0.51	1061.47	1061.26	1062.14	1062.14	1065.00	1064.30	
3	1	75.0	0.16	0.16	0.93	0.15	0.15	10.0	10.0	6.1	0.92	2.53	1.66	12	0.51	1061.64	1061.26	1062.19	1062.14	1064.00	1064.30	
4	1	53.0	0.16	0.35	0.90	0.14	0.25	10.0	13.6	5.4	1.34	2.54	2.11	12	0.51	1061.53	1061.26	1062.20	1062.14	1064.00	1064.30	
5	4	83.0	0.12	0.12	0.69	0.08	0.08	10.0	10.0	6.1	0.52	2.53	1.30	12	0.51	1061.95	1061.53	1062.35	1062.30	1064.50	1064.00	
6	4	42.0	0.07	0.07	0.36	0.02	0.02	10.0	10.0	6.1	0.15	2.52	0.26	12	0.50	1061.74	1061.53	1062.37	1062.37	1065.00	1064.00	
Caruso Dental																Number of lines: 6				Run Date: 05-03-2026		
NOTES: Intensity = 146.14 / (Inlet time + 15.00) ^ 0.98; Return period = 25 Yrs.																						